DLR Group
1111 Lincoln Mall #201
Lincoln, NE 68508
Tel: 402.742.4200

ADDENDUM NO. CC-3

TELEGRAPH DISTRICT
401 BUILDING RENOVATION
LINCOLN, NEBRASKA
DLR GROUP PROJECT NO. 10-16202-00

COMBINED CONTRACT March 21, 2016

NOTICE TO BIDDERS: Amend the Project Manual and Drawings to the above referenced project as follows:

PROJECT MANUAL

ITEM NO. 1 SECTION 004323 ALTERNATES FORM

A. Replace Alternate Form with Revised 004323 Alternate Form as shown on Attachment No. 1.

ITEM NO. 2 SECTION 230713 DUCT INSULATION & 233113 METAL DUCTS

Under base bid:

A. All interior ductwork shall be single wall construction.
B. All fan coil supply and return ductwork, new ERV supply ductwork to fan coils, and transfer ducts shall have a 1 inch thick interior duct liner. Liner shall be flexible fibrous-glass type, complying with ASTM C 1071, NFPA 90A, and with NAIMA AH124 “Fibrous Glass Duct Liner Standard.” Liner shall have a maximum thermal conductivity of 0.27 Btu x in./h x sq. ft. x deg F at 75 deg F mean temperature. Liner adhesive shall be water-based, and comply with NFPA 90A and ASTM C916.
C. All existing ERV supply ductwork in the chase shall be wrap insulated with 1-1/2” of mineral-fiber blanket insulation. Blanket insulation shall be mineral or glass fibers bonded with a thermosetting resin. Comply with ASTM C 612. Provide products by CertainTeed, Johns Manville, Knauf Insulation, Manson Insulation, or Owens Corning.
D. Interior exhaust ductwork to the ERV shall be left uninsulated.

Under Alternate #004, revise fan coil supply and return ductwork and new ERV supply ductwork to be double-wall rectangular and round ductwork, per the original project specifications. All other duct insulation not mentioned to remain as defined in base bid write-up as defined above.

ITEM NO. 3 SECTION 263213.14 DIESEL ENGINE GENERATORS

A. Paragraph 2.6.H Control and Monitoring Panel, Added “located at generator.”
B. Paragraph 2.6.I: Common Remote Panel with Common Audible Alarm: Added “located in building”

ITEM NO. 4 SECTION 315001 FOUNDATION SUPPORT

A. Add Section 315001 Foundation Support as shown in Attachment No. 2.

DRAWINGS

ITEM NO. 1 DRAWING S1.2 UPPER FOUNDATION PLAN

A. Modify drawings as shown on Attachment S1.2-1

ITEM NO. 2 DRAWING S2.1 FRAMING PLAN – LEVEL 1

A. Modify drawings as shown on Attachment S2.1-1

ITEM NO. 3 DRAWING S2.2 FRAMING PLAN – LEVEL 2

A. Modify drawings as shown on Attachment S2.2-1

ITEM NO. 4 DRAWING S2.3 FRAMING PLAN – LEVEL 3

A. Modify drawing as shown on Attachment S2.3-1

ITEM NO. 5 DRAWING S2.4 ROOF FRAMING PLAN

A. Modify drawing as shown on Attachment S2.4-1

ITEM NO. 6 DRAWING S2.5 ROOF FRAMING PLAN - PENTHOUSE

A. Modify drawings as shown on Attachment S2.5-1.

ITEM NO. 7 DRAWING S3.1 STRUCTURAL DETAILS

A. Modify drawings as shown on Attachment S3.1-1
B. Modify detail 35/S3.1 TYP COLUMN COVER DETAIL
C. Add detail 36/S3.1 BASE PLATES
D. Add detail 45/S3.1 TYP CONCRETE WALL FOOTING DETAIL
E. Add detail 46/S3.1 TYP FOOTING AT EXISTING FOOTING
F. Add detail 47/S3.1 TYP STEP FOOTING DETAIL

ITEM NO. 8 DRAWING S5.1 STRUCTURAL DETAILS

A. Modify drawings as shown on Attachment S5.1-1
B. Add detail 17/S5.1 TYP MECHANICAL OPENING DETAIL
C. Add detail 37/S5.1 CONNECTION DETAIL
D. Modify detail 45/S5.1 CONNECTION DETAIL

ITEM NO. 9 DRAWING M5.2 MECHANICAL DETAILS AND GAS RISER

A. Add the Penthouse Heat Recovery Unit Detail as shown on Attachment M5.2.

ITEM NO. 10 DRAWING E6.2 ELECTRICAL ONE-LINE DIAGRAM

A. Add Keynote 5 to the Utility Transformer in the top left corner of the sheet. Keynote 5 shall read “THE EXISTING SERVICE ENTRANCE DIP POLE WILL BE RELOCATED BY LES. LES WILL PROVIDE NEW 4” PRIMARY CONDUIT AND CONDUCTORS FROM THE DIP POLE TO THE EXISTING LES

TRANSFORMER. THE ELECTRICAL CONTRACTOR SHALL COORDINATE THE OUTAGE AND POLE RELOCATION WITH THE OWNER, GENERAL CONTRACTOR, LES AND OTHER SUBCONTRACTORS."

END OF ADDENDUM

DOCUMENT 004323 - ALTERNATES FORM

Attachment No. 1 for CC-3, Dated March 21, 2016

1.1 BID INFORMATION

A. Bidder: _____.

B. Prime Contract: _____.

C. Project Name: Telegraph District 401 Building Renovation.

D. Project Location: 401 S. 21st Street, Lincoln, Nebraska.

E. Owner: 401 Building, LLC.

F. Owner Project Number: NA.

G. Architect: DLR Group.

H. Architect Project Number: 10-16202-00.

1.2 BID FORM SUPPLEMENT

A. This form is required to be attached to the Bid Form.

1.3 DESCRIPTION

A. The undersigned Bidder proposes the amount below be added to or deducted from the Base Bid if particular alternates are accepted by Owner. Amounts listed for each alternate include costs of related coordination, modification, or adjustment.

1. Cost-Plus-Fee Contract: Alternate price given below includes adjustment to Contractor's Fee.

B. If the alternate does not affect the Contract Sum, the Bidder shall indicate "NO CHANGE."

C. If the alternate does not affect the Work of this Contract, the Bidder shall indicate "NOT APPLICABLE."

D. The Bidder shall be responsible for determining from the Contract Documents the affects of each alternate on the Contract Time and the Contract Sum.

E. Owner reserves the right to accept or reject any alternate, in any order, and to award or amend the Contract accordingly within [60] days of the Notice of Award unless otherwise indicated in the Contract Documents.

F. Acceptance or non-acceptance of any alternates by the Owner shall have no affect on the Contract Time unless the "Schedule of Alternates" Article below provides a formatted space for the adjustment of the Contract Time.

1.4 SCHEDULE OF ALTERNATES

A. Alternate No. 001: Provide Baked Enamel Steel Toilet Compartments in lieu of Plastic Toilet Compartments:

1. ADD____ DEDUCT____ NO CHANGE____ NOT APPLICABLE____.
2. _____ Dollars ($_____).
3. ADD____ DEDUCT____ calendar days to adjust the Contract Time for this alternate.

B. Alternate No. 002: Replace all instances of APC-2 with APC-1. Perimeter trim to remain:

1. ADD____ DEDUCT____ NO CHANGE____ NOT APPLICABLE____.
2. _____ Dollars ($_____).
3. ADD____ DEDUCT____ calendar days to adjust the Contract Time for this alternate.

C. Alternate No. 003: Provide alternate Carpet for CPT-1 as follows: Mfr: Interface; Style: Detours; Color: Slate.

1. ADD____ DEDUCT____ NO CHANGE____ NOT APPLICABLE____.
2. _____ Dollars ($_____).
3. ADD____ DEDUCT____ calendar days to adjust the Contract Time for this alternate.

D. Alternate No. 004: Duct Insulation: Provide fan coil supply and return ductwork and new ERV supply ductwork as double-wall rectangular and round ductwork, per the original project specifications.

E. ADD____ DEDUCT____ NO CHANGE____ NOT APPLICABLE____.

F. _____ Dollars ($_____).

G. ADD____ DEDUCT____ calendar days to adjust the Contract Time for this alternate.

1.5 SUBMISSION OF BID SUPPLEMENT

A. Respectfully submitted this ____ day of _____, 2016.

B. Submitted By:_____(Insert name of bidding firm or corporation).

C. Authorized Signature:_____(Handwritten signature).

D. Signed By:_____(Type or print name).

E. Title:_____(Owner/Partner/President/Vice President).

END OF DOCUMENT 004323

Attachment No. 2 for CC-3, dated March 21, 2016

SECTION 315001 - FOUNDATION SUPPORT

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

A. Section includes helical piers.

B. Related Requirements:

1. Section 312000 "Earthwork, Subgrade, and Subbase" for excavating and backfilling and for controlling surface-water runoff and ponding.

1.3 ACTION SUBMITTALS

A. Product Data: For each type of product.

1. Include construction details, material descriptions, performance properties, and dimensions of individual components and profiles, and calculations for pile and pier system.

B. Shop Drawings: For helical piers, prepared by or under the supervision of a qualified professional engineer. Show fabrication and installation details.

1. Include plans, elevations, sections, and details.
2. Indicate welds by standard AWS symbols, distinguishing between shop and field welds, and show size, length, and type of each weld.
3. Include arrangement, test and anchor piles, equipment, and instrumentation. Submit plan and structural analysis data signed and sealed by the qualified professional engineer responsible.

1.4 INFORMATIONAL SUBMITTALS

A. Qualification Data: For qualified Installer and professional engineer.

B. Existing Conditions: Using photographs or video recording to show existing conditions of adjacent construction and site improvements that might be misconstrued as damage caused by inadequate performance of excavation support and protection systems. Submit before Work begins.

C. Welding Certificates.

D. Mill Test Reports: For steel shapes and plates, signed by manufacturer.

E. Helical piers should be installed as shown on plan. The installer and engineer shall submit a written plan of repair to architect-engineer.

1. The plan shall include, but limited to the following:

a. Total number of piers required.
b. Locations of the individual piers.
c. Minimum installed depth.
d. Minimum final installation torque of the helical piers.

1.5 QUALITY ASSURANCE

A. Installer Qualifications: Manufacturer's authorized representative who is trained and approved for installation of units required for this Project. The Manufacturer shall provide certification that the Installation Contractor is an approved installer of their product. Installation Contractor shall have more than 5 years minimum experience on projects of similar scope.

1. Installer's responsibility includes engaging a qualified professional engineer to prepare design documents.

B. Testing Agency Qualifications: Qualified according to ASTM E 329 for testing indicated.

C. Welding Qualifications: Qualify procedures and personnel according to AWS D1.1.

D. Preinstallation Conference: Conduct conference at Project site.

1.6 DELIVERY, STORAGE, AND HANDLING

A. Deliver to Project site in such quantities and at such times to ensure continuity of installation. Handle and store piles/piers at Project site to prevent buckling of physical damage.

1.7 PROJECT CONDITIONS

A. Protect structures, underground utilities, and other construction from damage cause by pier installation. Do not interrupt any utility serving facilities occupied by the Owner or others unless permitted.

B. Site Information: A geotechnical report has been prepared for this Project and is included for reference in the Project Manual for information only. The opinions expressed in the report are those of a geotechnical engineer and represent interpretations of subsoil conditions, tests, and results of analyses conducted by a geotechnical engineer. Owner is not responsible for interpretations or conclusions drawn from the data.

PART 2 - PRODUCTS

2.1 PERFORMANCE REQUIREMENTS

A. Piers shall be designed to support the nominal compressive loads as shown on the project plans.

1. Helical Piers: The overall length, helix configuration, and minimum torsional resistance of a helical pile shall be such that the required geotechnical capacity is developed by the helix plate(s) in an appropriate bearing stratum.

a. All piers shall be installed to provide a minimum factor of safety against ultimate bearing resistance of two (2.5), unless noted otherwise. Piers must satisfy the deflection criteria stated on the plans or drawings.
b. Each pier shall be designed to meet a corrosion service live of 50 years in accordance with ICC-ES Acceptance Criteria 358, unless noted otherwise.
c. The pier design shall take into account such pier spacing, pier buckling potential, soil stratification, corrosion, and strain compatibility issues as are present for this project.

B. All structural steel components shall be designed within the limits provided by the American Institute of Steel Construction (AISC) Specification for Structural Steel Buildings (AISC 360).

2.2 HELICAL PIERS

A. Helical Bearing Plate:

1. Hot rolled carbon steel sheet, strip, or plate per ASTM A572, or A1018, or A656 with minimum yield strength of 50 ksi. Minimum plate thickness is 3⁄8".
2. Helix bearing plates are formed on matching metal dies to true helical shape.

B. Helical Shaft:

1. Structural steel tube or pipe, welded or seamless, in compliance with ASTM A500. Minimum wall thickness is 0.313 inch. Minimum yield strength is 42 ksi. Minimum ultimate tension/compression strength is 58 ksi.

C. Coupling bolts:

1. 3⁄4" diameter bolt per ASTM A320 Grade L7, Sy (min) = 105 ksi, Su (min) = 125 ksi.
2. 7⁄8" diameter bolt per ASTM A193 Grade B7, Sy (min) = 105 ksi, Su (min) = 125 ksi.
3. 3⁄4" diameter bolt per SAE J429 Grade 5, Sy (min) = 92 ksi, Su (min) = 120 ksi.

D. Foundation Bracket:

1. Brackets are formed from steel that meets or exceeds the requirements of ASTM A36, and have a hot-dipped galvanized coating per ASTM A153.

E. Welding:

1. All welding shall be in accordance with AWS D1.1, latest revision.

F. Finish:

1. All material shall be hot-dipped galvanized in accordance with ASTM A153 after fabrication.

PART 3 - EXECUTION

3.1 PREPARATION AND EXCAVATION

A. Protect structures, utilities, sidewalks, pavements, and other facilities from damage caused by settlement, lateral movement, undermining, washout, and other hazards that could develop during excavation operations.

1. Shore, support, and protect utilities encountered.

B. Hand or machine excavate immediately adjacent to the building foundation at each pier locations to expose the footing for the bottom of the grade beam to an appropriate working width and depth.

1. After installation is complete, replace soil in exposed areas in 6” lifts and mechanically compact into place.

3.2 INSTALLATION OF HELICAL PIERS

A. The helical pier shall be positioned as shown. Proper angular alignment shall be established at the start of installation.

B. The installing contractor shall furnish and install all helical piers per the project plans and approved pier design documentation. In the event of conflict between the project plans and the approved pier design documents, the installing contractor shall not begin construction on any affected items until such conflict has been resolved.

C. The helical pier shall be installed in a smooth, continuous manner. The rate of helical pier rotation shall be in the range of 5 to 20 revolutions per minute. Sufficient down pressure shall be applied to advance the helical pier.

D. Plain extension material may be required to position the helical pier at the depth required. Extensions shall be coupled to the helical pile using the bolts provided with the extension. These bolts shall be installed and tightened to snug tight condition.

E. Installation torque shall be monitored throughout the installation process.

F. If underground obstructions are encountered during installation, the installer shall have the option of removing the obstruction if possible or relocating the helical pier. This latter option may require the relocation of adjacent helical piers.

G. Installation Tolerances: Install piers without exceeding the following tolerances, measured at pier heads:

1. Location: 4 inches (102 mm) from location indicated after initial installation, and 6 inches (152 mm) after pier installation is completed.
2. Plumb: Maintain 1 inch (25 mm) in 4 feet (1.2 m) from vertical, or a maximum of 4 inches (102 mm), measured when pier is aboveground..
3. Batter Angle: Maximum 1 inch (25 mm) in 4 feet (1.2 m) from required angle, measured when pier is aboveground.

H. Termination of Installation:

1. The maximum installation torque shall at no time exceed the torque rating of the helical pier shaft as specified for the project.
2. Helical piers shall be installed to the minimum torque value as provided by the pier analysis.
3. If the minimum torque requirement has not been satisfied at the minimum depth level, the installer shall have the following options:

    a. Install the helical pier deeper using additional plain extension material until the specified torque level is obtained, or
    b. Remove the existing helical pier and install a helical pier with larger and/or more helices. This revised helical pier shall be installed at least three (3) feet beyond the termination depth of the original helical pier.
    c. Add additional helical piers.

4. The minimum depth of installation shall be provided by the pier design. If the installer cannot achieve the depth, the engineer shall be contacted before proceeding further. If the maximum torque rating of the installing unit has been reached but that of the helical pier has not prior to satisfying the minimum depth requirement, the Dealer shall have the option of utilizing a higher torque installing unit meeting the requirement of paragraph IV to drive the helical pier deeper.

    a. If the minimum torque rating of the helical pier and/or installing unit has been reached prior to satisfying the minimum depth level, the installer shall have the following options: (The approval of the Engineer of Record shall be obtained before option 1 or 2 is implemented.)

        1) Terminate the installation at the depth obtained, or
        2) Remove the existing helical pier and install a helical pier with smaller and/or fewer helices. This revised helical pier shall be installed at least three (3) feet beyond the termination depth of the original helical pier.

3.3 CONNECTION BRACKET HAUNCH

A. The helical pier shall be connected to the structure using an approved steel bracket capable of safely transferring the structural loads to the pier.

3.4 INSTALLATION RECORD

A. Written installation records shall be maintained for each helical pier. These records shall include, but are not limited to the following:

1. Project name and/or location.
2. Name of authorized installer.
3. Name of Installer's foreman or representative who witnessed the installation.
4. Date and time of installation.
5. Location and reference number of pier.
6. Descriptions of lead section and extensions installed.
7. Overall depth of installation as referenced from bottom of grade beam or footing.
8. Torque readings for the last three (3) feet of installation if practical for helical piers. In lieu of this requirement, the termination torque shall be recorded as a minimum.
9. Any other applicable information relating to the installation.

3.5 FIELD QUALITY CONTROL

A. Special Inspections: Owner will engage a qualified special inspector to perform the following special inspections:

1. Helical Pier foundations.

B. Testing Agency: Owner will engage a qualified independent testing agency to perform tests and inspections.

C. Tests and Inspections:

1. The test plan shall include, but not be limited to, the following:

   a. The number and locations of tests, based on site and subsurface conditions.
   b. The maximum load to be applied during the test
   c. The acceptance criteria including load versus displacement.

2. The test equipment shall be capable of applying a compression load equal to the maximum test load specified in the test plan.
3. If the compression test requires additional helical piers for reaction, these helical piers shall be installed to the same torque requirements as the test helical pier.
4. The helical pier shall be tested to the greater of the safety factored load or its ultimate capacity, defined as the maximum load the helical pier can resist at continuous creep conditions.

5. Test records shall include the following;

    a. Items as outlined in Section 3.5 Installation Records, of this Specification.
    b. Magnitudes of applied loads and corresponding displacements.

3.6 DISPOSAL

A. Remove withdrawn piers and damaged piers from site and legally dispose of them off Owner's property.

END OF SECTION 315001

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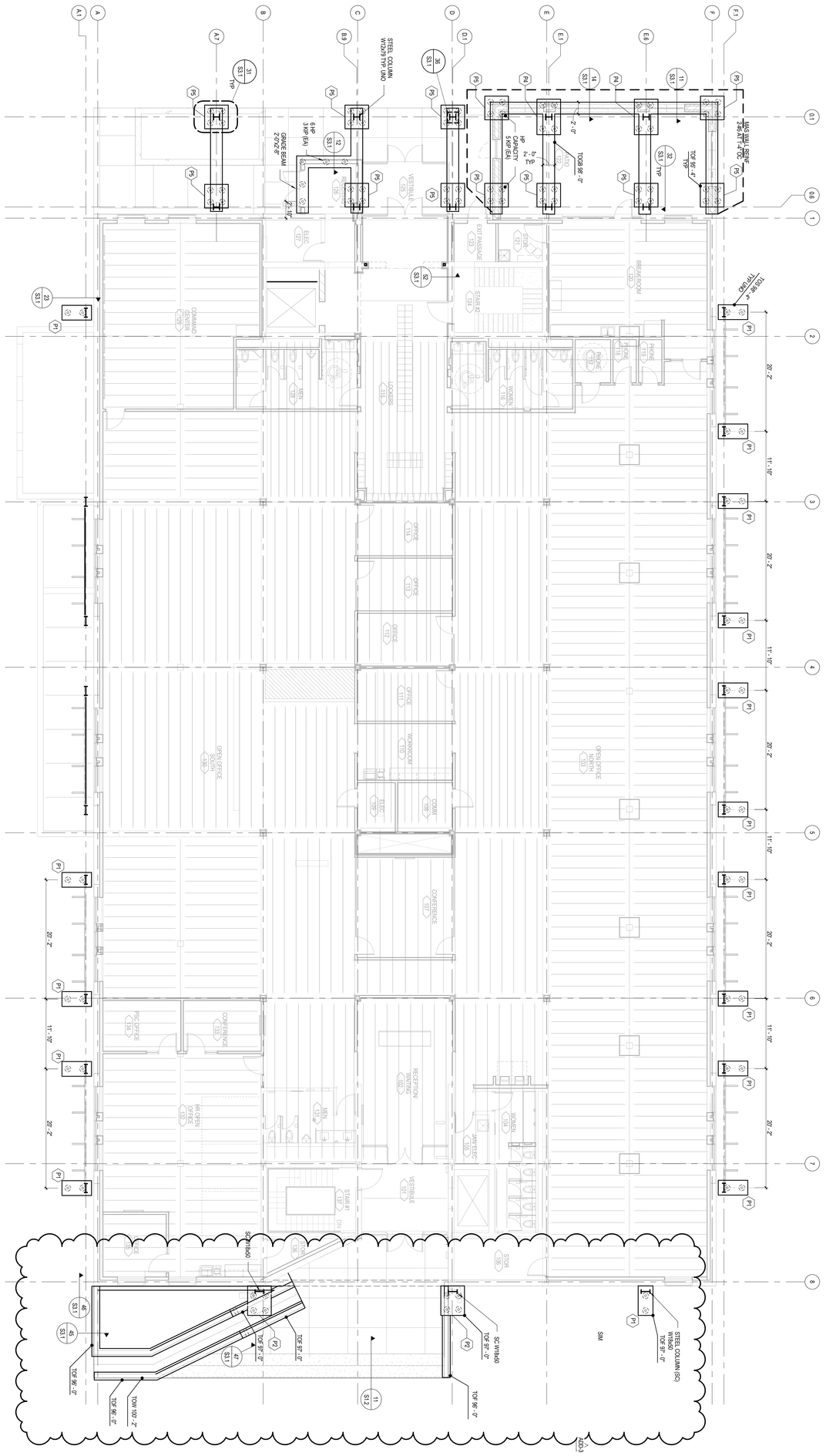
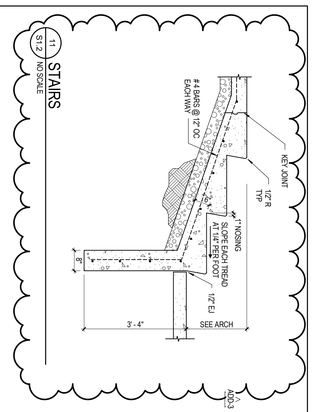
UPPER FOUNDATION PLAN

NORTH

SCALE: 1/8" = 1'-0"





DLR Group

Architecture Engineering Planning Interiors

S1.2

10-16202-00

2/24/2016

Revisions

ADD-3 03/21/16

UPPER FOUNDATION PLAN

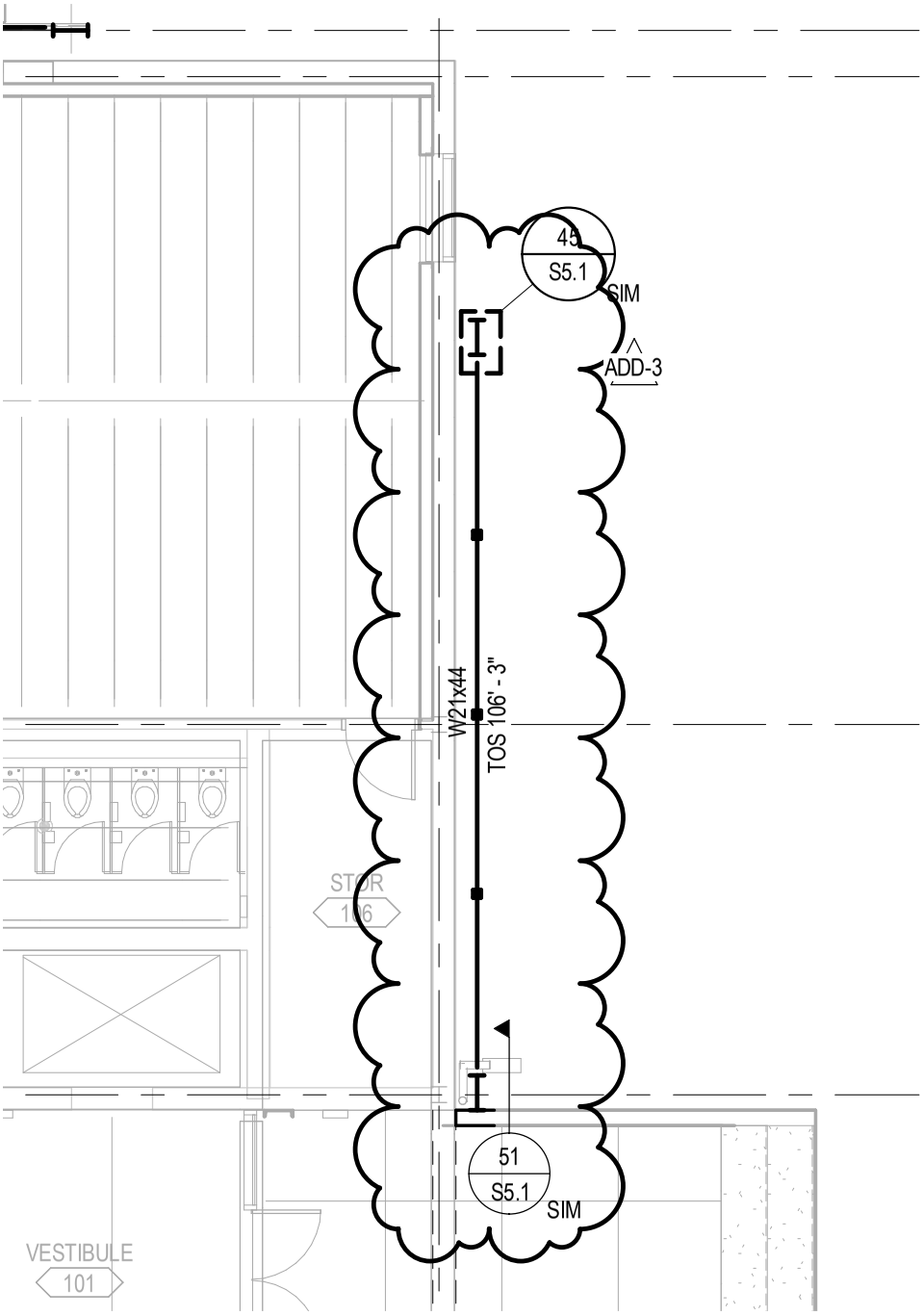
TELEGRAPH DISTRICT

401 BUILDING RENOVATION

Attachment No. S1.2-1
to: Addendum #3
Dated: 03/21/2016

PROFESSIONAL CIVIL ENGINEER
CESAR A. GARCIA
STATE OF NEBRASKA

FEB 26/2016



Attachment No. S2.1-1
to Addendum # 3
Dated: 03/21/03

DLR Group
Architecture Engineering Planning Interiors

S2.1

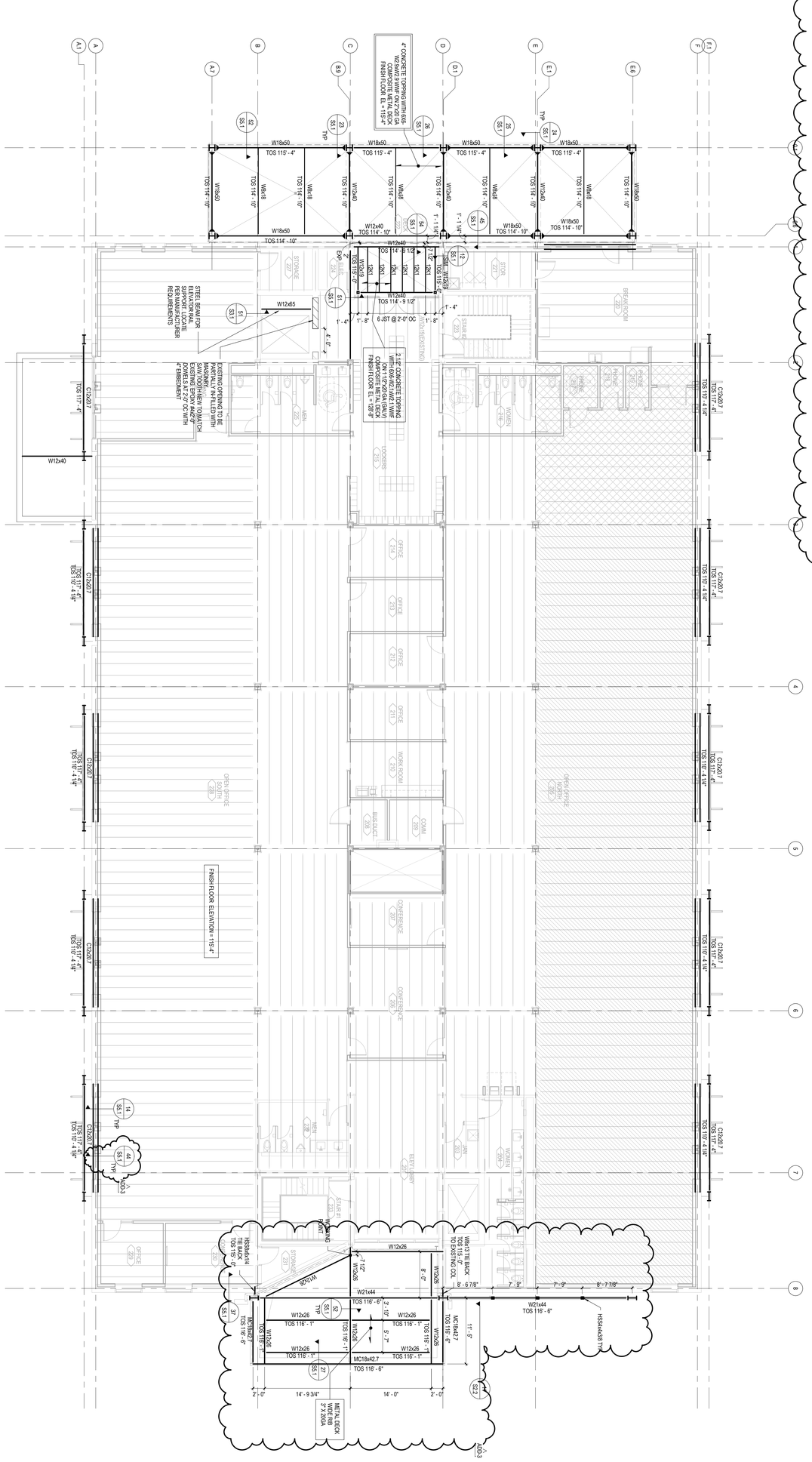
10-16202-00

2/24/2016

FRAMING PLAN - LEVEL 1

TELEGRAPH DISTRICT

401 BUILDING RENOVATION



11 S2.2 **CANOPY**
NO SCALE

12 S2.2 **CONNECTION**
NO SCALE

13 S2.2 **CONNECTION**
NO SCALE

ADD-3

4" CONCRETE TOPPING WITH 6X6-W2.9xW2.9 WWF ON 2"x20 GA COMPOSITE METAL DECK FINISH FLOOR EL = 115'-4"

2 1/2" CONCRETE TOPPING WITH 6X6-W2.1xW2.1 WWF ON 1 1/2"x20 GA (GALV) COMPOSITE METAL DECK FINISH FLOOR EL = 128'-8"

STEEL BEAM FOR ELEVATOR RAIL SUPPORT. LOCATE PER MANUFACTURER REQUIREMENTS

EXISTING OPENING TO BE PARTIALLY IN-FILLED WITH MASONRY. SAW TOOTH NEW TO MATCH EXISTING. EPOXY #4x2'-0" DOWELS AT 2'-0" OC WITH 4" EMBEDMENT

FINISH FLOOR ELEVATION = 115'-4"

METAL DECK WIDE RIB 3" X 20GA

FRAMING PLAN - LEVEL 2
SCALE: 1/8" = 1'-0"
NORTH

Attachment No. S2.2-1
to: Addendum #3
Dated: 03/21/2016

PROFESSIONAL CIVIL ENGINEER
CESAR AUGUSTO E-12603
STATE OF NEBRASKA
FEB 26/2016

FRAMING PLAN - LEVEL 2
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

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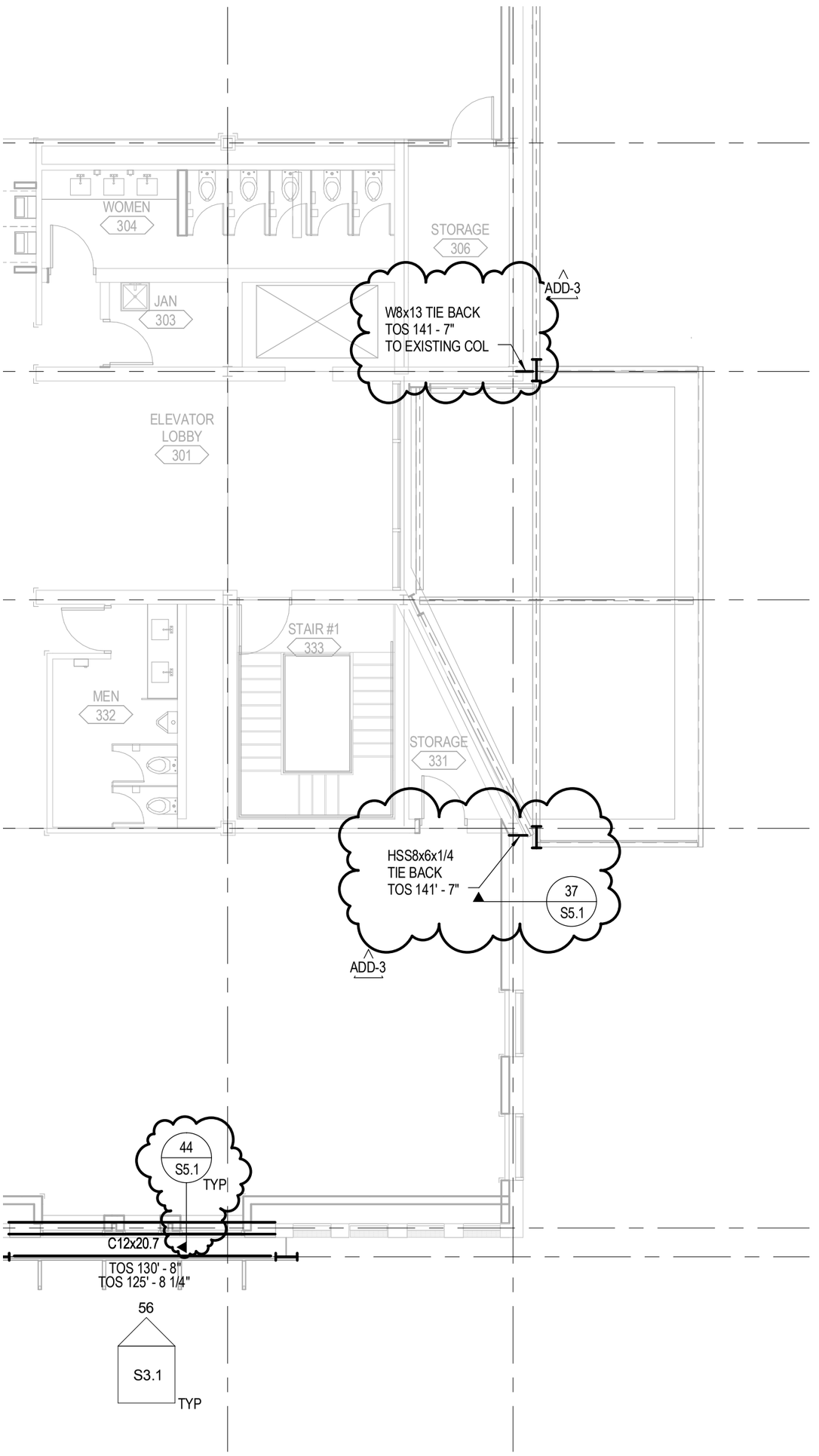

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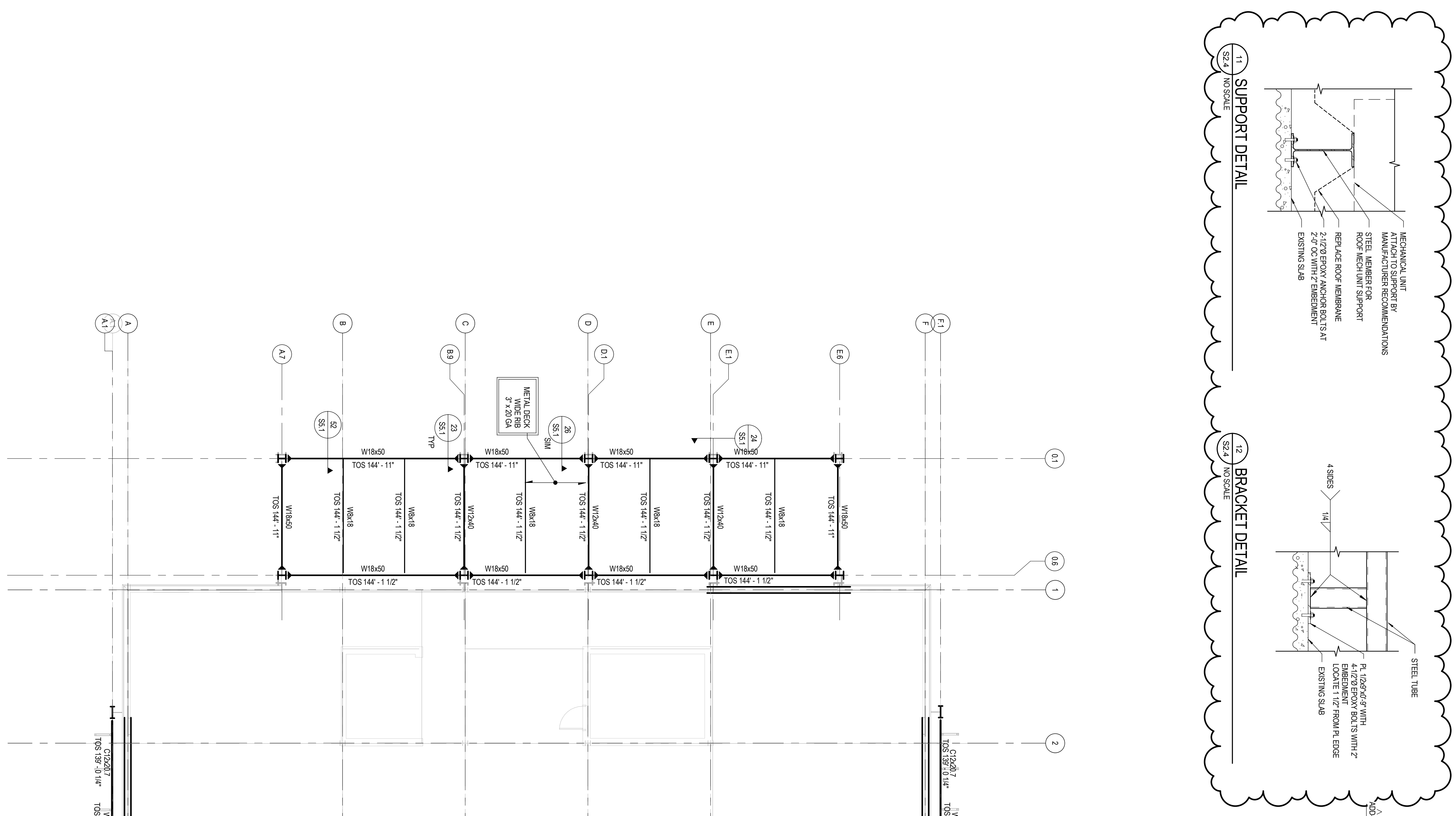
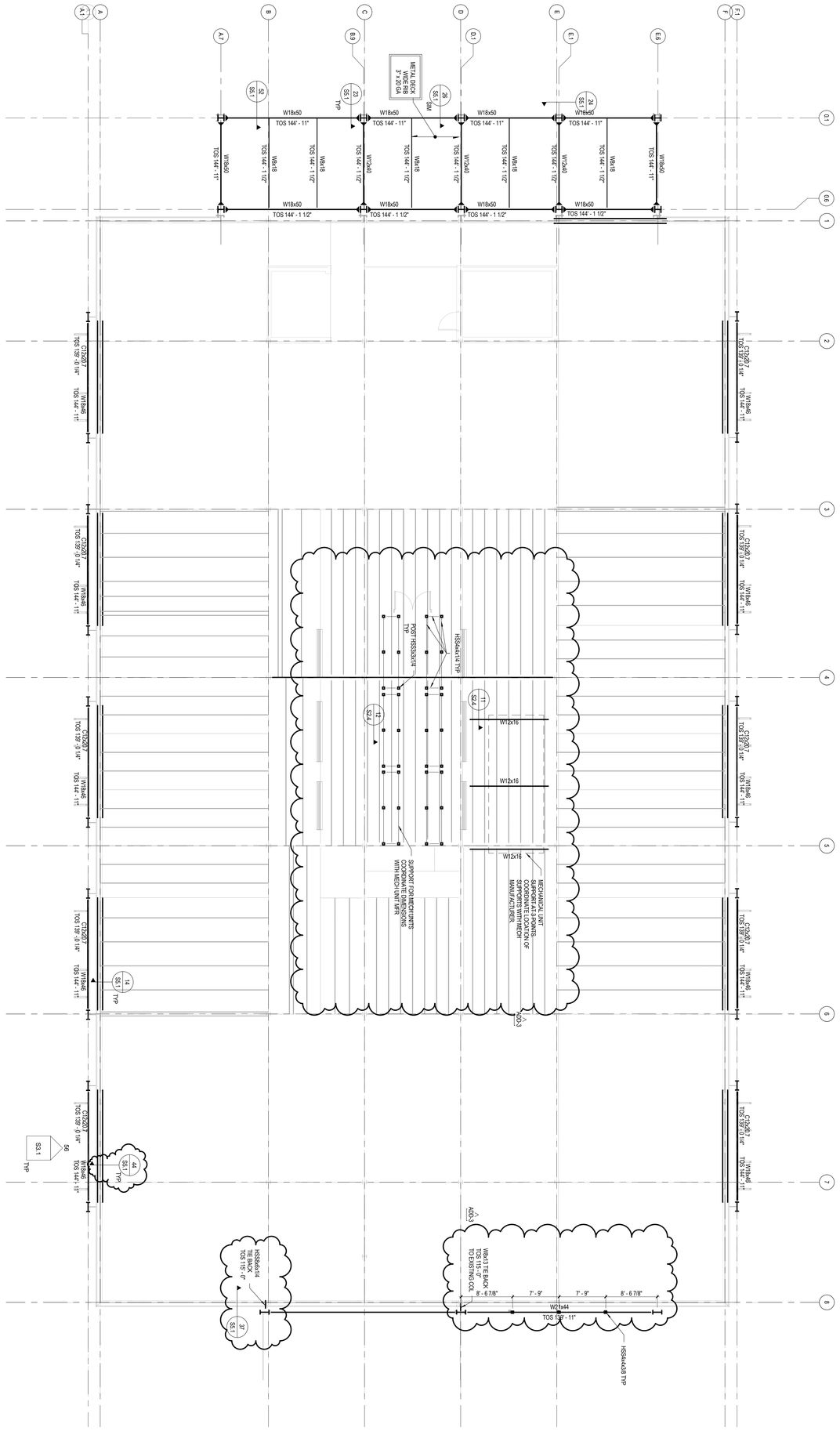
10-16202-00
2/24/2016

FRAMING PLAN - LEVEL 3
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

Attachment No. S2.3-1
to Addendum#3
Dated: 03/21/16





11
S2.4
SUPPORT DETAIL
NO SCALE

12
S2.4
BRACKET DETAIL
NO SCALE

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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E-13603
FEB 26/2016

Attachment No. S2.4-1
to: Addendum #3
Dated: 03/21/2016

ROOF FRAMING PLAN
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

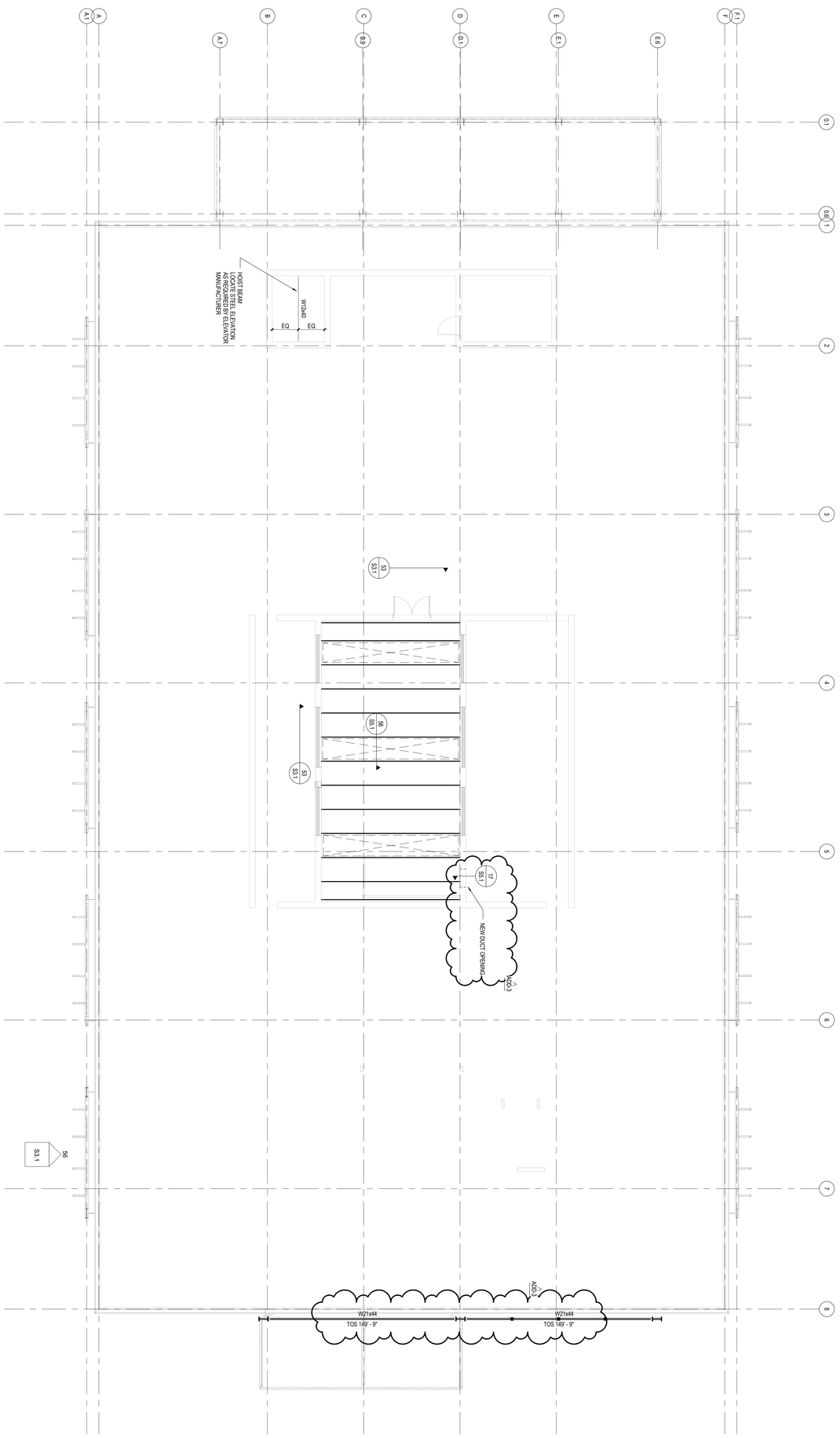
S2.4
10-16202-00
2/24/2016
Revisions
ADD-3 03/21/16

DLR Group
Architecture Engineering Planning Interiors

NORTH

SCALE: 1/8" = 1'-0"

PENTHOUSE FRAMING PLAN



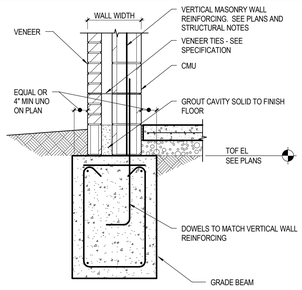
DLR Group
Architecture Engineering Planning Interiors

S2.5
10-16202-00
2/24/2016
Revisions
ADD-3 032116

ROOF FRAMING PLAN - PENTHOUSE
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

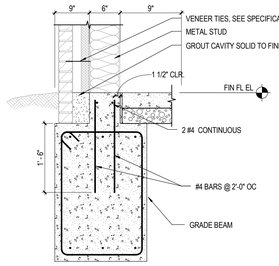
Attachment No. S2.5-1
to: Addendum #3
Dated: 03/21/2016

PROFESSIONAL CIVIL ENGINEER
CESAR A. GARCIA
STATE OF NEBRASKA
FEB 26/2016



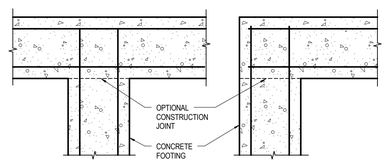
11 S3.1 TYP MASONRY WALL FOOTING DETAIL

NO SCALE (AT EXTERIOR)



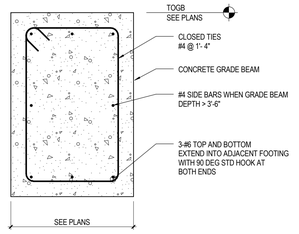
12 S3.1 TYP FOUNDATION AT STUD WALL

NO SCALE



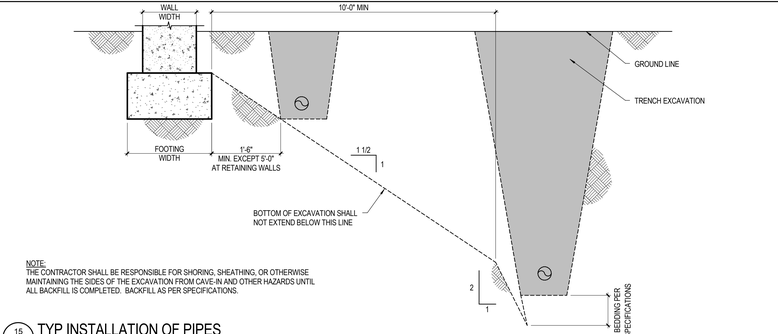
13 S3.1 TYP FOOTING REINFORCING DETAIL

NO SCALE (AT CORNERS AND INTERSECTIONS)



14 S3.1 GRADE BEAM DETAIL

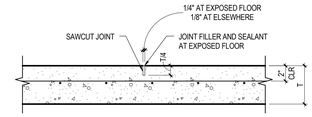
NO SCALE



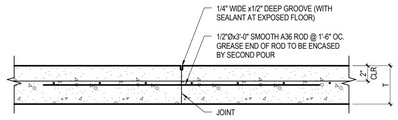
NOTE:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR SHORING, SHEATHING, OR OTHERWISE MAINTAINING THE SIDES OF THE EXCAVATION FROM CAVE-IN AND OTHER HAZARDS UNTIL ALL BACKFILL IS COMPLETED. BACKFILL AS PER SPECIFICATIONS.

15 S3.1 TYP INSTALLATION OF PIPES

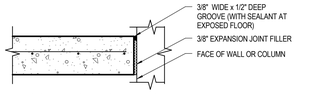
NO SCALE (PARALLEL TO FOOTING)



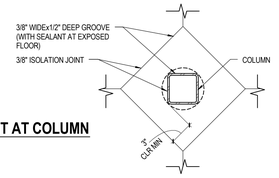
A CONTROL (CONTRACTION) JOINT (CJ)



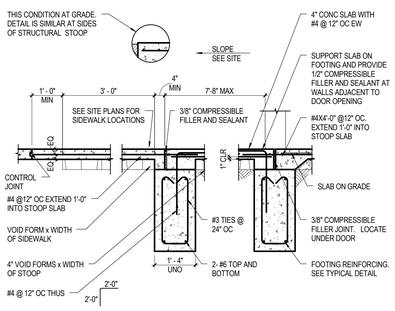
B CONSTRUCTION JOINT (CONST JT)



C ISOLATION JOINT (IJ)

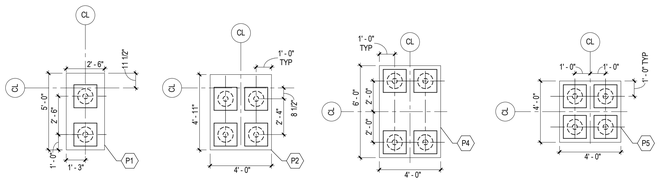


D ISOLATION JOINT AT COLUMN



21 S3.1 TYP SLAB-ON-GRADE JOINTS

NO SCALE



FOOTING 2'-0"x5'-0"x36"
3-#6 LONG WAY TOP&BOTT
6-#6 SHORT WAY TOP&BOTT

HELICAL PIER
12 KIPS COMPRESSION
2 KIPS TENSION

FOOTING 4'-0"x4'-11"x36"
5-#6 LONG WAY TOP&BOTT
6-#6 SHORT WAY TOP&BOTT

HELICAL PIER
25 KIPS COMPRESSION
20 KIPS TENSION

FOOTING 4'-0"x6'-0"x36"
5-#6 LONG WAY TOP&BOTT
7-#6 SHORT WAY TOP&BOTT

HELICAL PIER
28 KIPS COMPRESSION
22 KIPS TENSION

FOOTING 4'-0"x4'-0"x36"
5-#6 EACH WAY TOP&BOTT

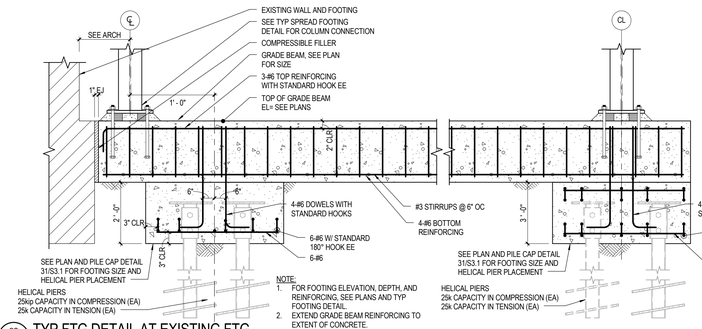
HELICAL PIER
25 KIPS COMPRESSION
20 KIPS TENSION

31 S3.1 TYP PILE CAP LAYOUT

NO SCALE

22 S3.1 TYP STOOP DETAIL

NO SCALE



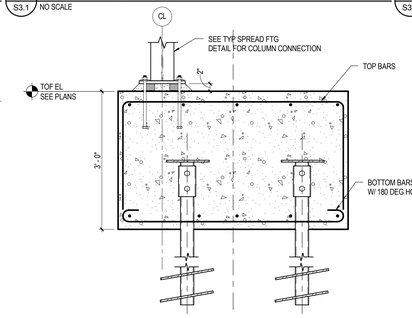
32 S3.1 TYP FTG DETAIL AT EXISTING FTG

NO SCALE

NOTE:
1. FOR FOOTING ELEVATION, DEPTH, AND REINFORCING, SEE PLANS AND TYP FOOTING DETAIL.
2. EXTEND GRADE BEAM REINFORCING TO EXTENT OF CONCRETE.

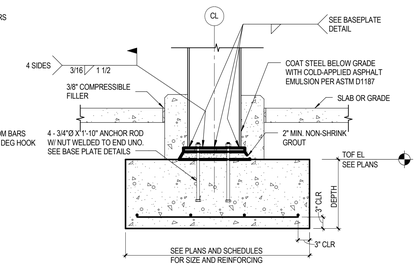
23 S3.1 CONCRETE FOOTING DETAIL

NO SCALE



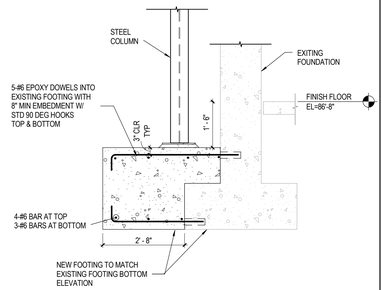
24 S3.1 TYP SPREAD FOOTING DETAIL

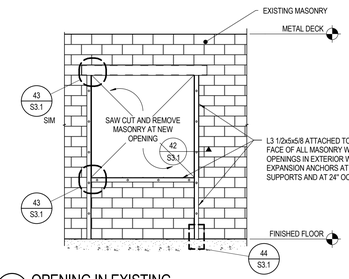
NO SCALE (AT STEEL W COLUMN)



25 S3.1 NEW FOOTING AT EXISTING

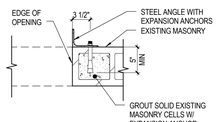
NO SCALE





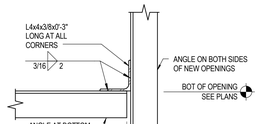
41 S3.1 OPENING IN EXISTING

NO SCALE



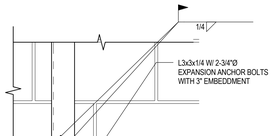
42 S3.1 NEW OPENING DETAIL

NO SCALE (AT EXISTING MASONRY WALL)



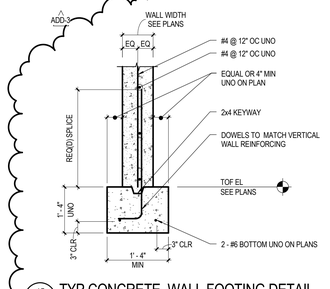
43 S3.1 CONNECTION DETAIL

NO SCALE



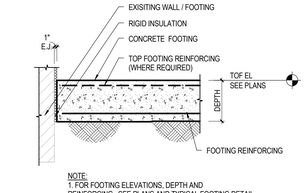
44 S3.1 CONNECTION DETAIL

NO SCALE



45 S3.1 TYP CONCRETE WALL FOOTING DETAIL

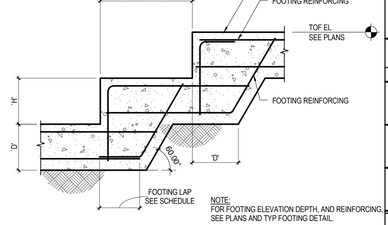
NO SCALE



NOTE:
1. FOR FOOTING ELEVATIONS, DEPTH AND REINFORCING, SEE PLANS AND TYPICAL FOOTING DETAIL.

46 S3.1 TYP FOOTING AT EXISTING FOOTING

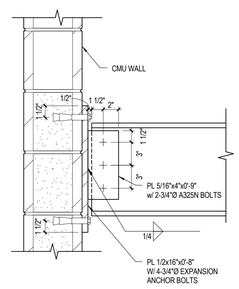
NO SCALE



NOTE:
FOR FOOTING ELEVATION DEPTH, AND REINFORCING, SEE PLANS AND TYP FOOTING DETAIL.

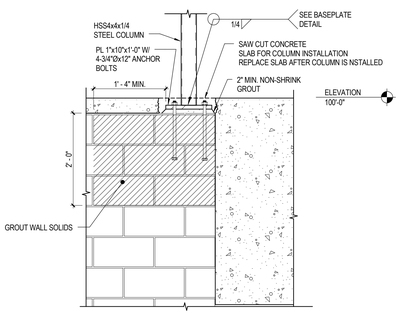
47 S3.1 TYP STEP FOOTING DETAIL

NO SCALE



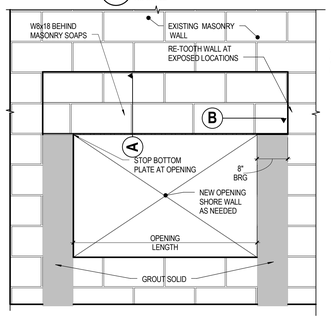
51 S3.1 CONNECTION

NO SCALE



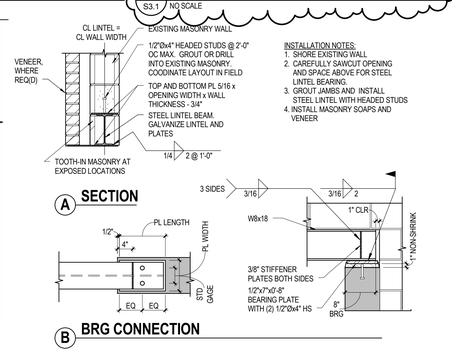
52 S3.1 COLUMN AT EXISTING CMU

NO SCALE



53 S3.1 TYP NEW OPENING IN EXISTING

NO SCALE (AT EXIST MASONRY WALL)



INSTALLATION NOTES:
1. SHORE EXISTING WALL
2. CAREFULLY SAWCUT OPENING AND SPACE ABOVE FOR STEEL LINTEL BEARING.
3. GROUT JAMBS AND INSTALL STEEL LINTEL WITH HEADED STUDS
4. INSTALL MASONRY SOAPS AND VENEER

35 S3.1 TYP COLUMN COVER DETAIL

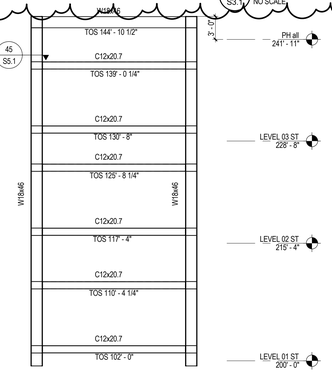
NO SCALE (AT EXTERIOR STEEL COLUMN)

36 S3.1 BASE PLATES

NO SCALE

56 S3.1 WINDOW FRAMING ELEVATION

NO SCALE



Attachment No. S3.1-1
to: Addendum #3
Dated: 03/21/2016

STRUCTURAL DETAILS
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

S3.1
10-162/02-00
2/24/2016
Revisions
#ADD3 03/21/16

DLR Group
Architecture Engineering Planning Interiors

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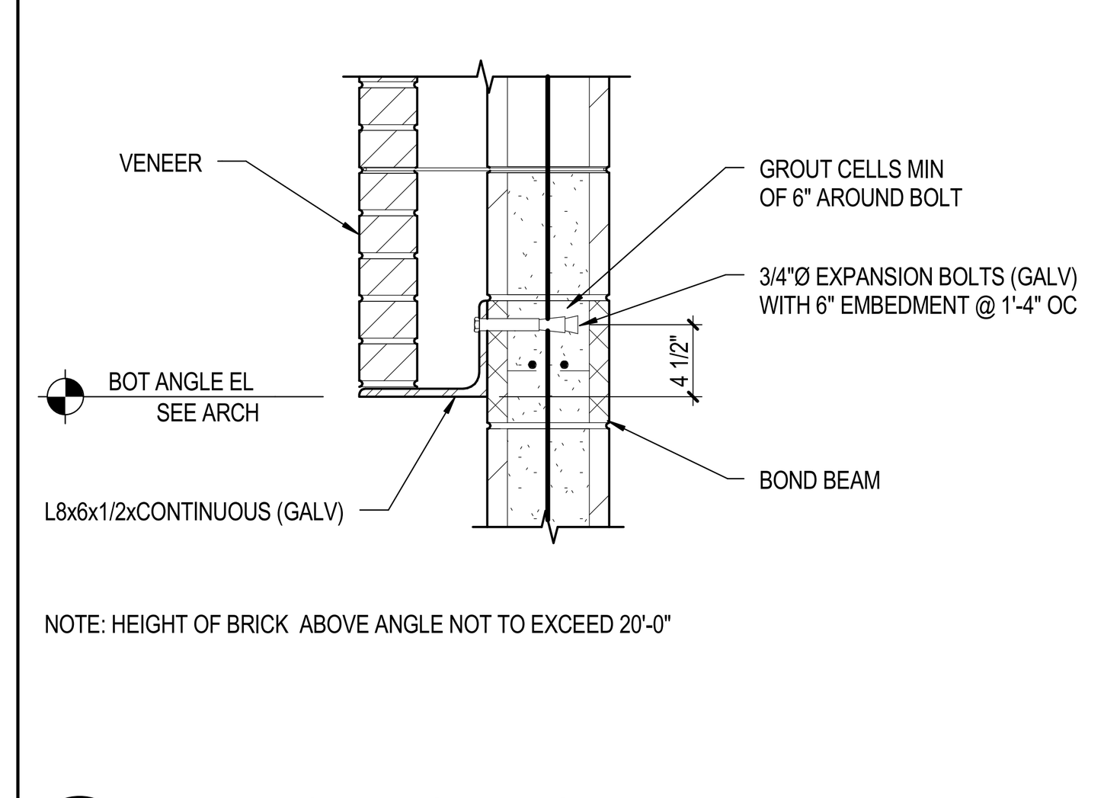
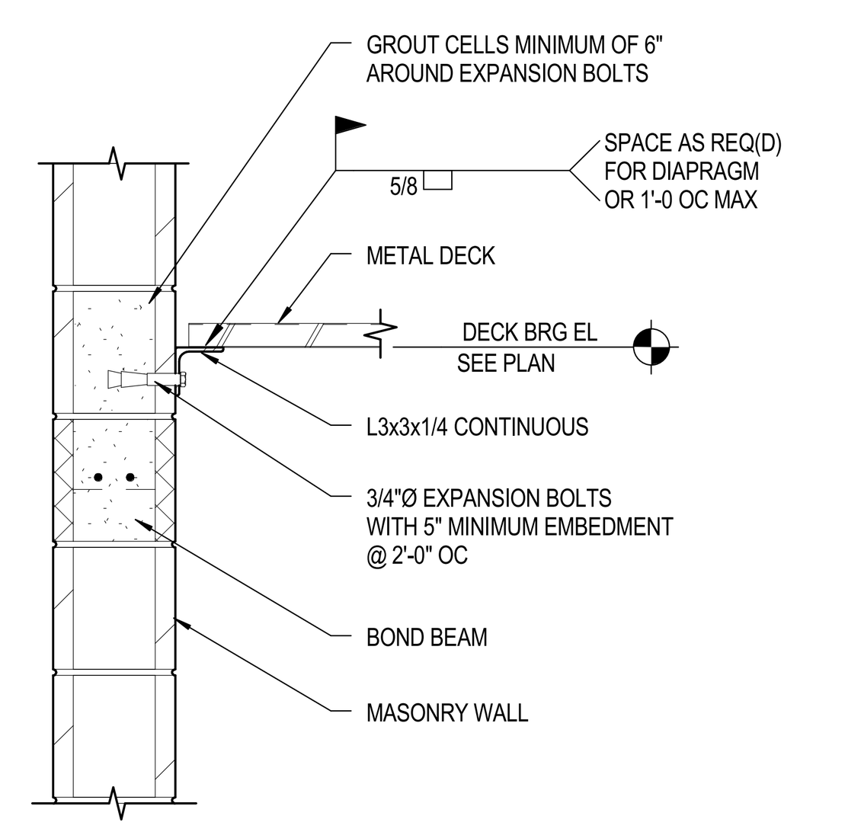
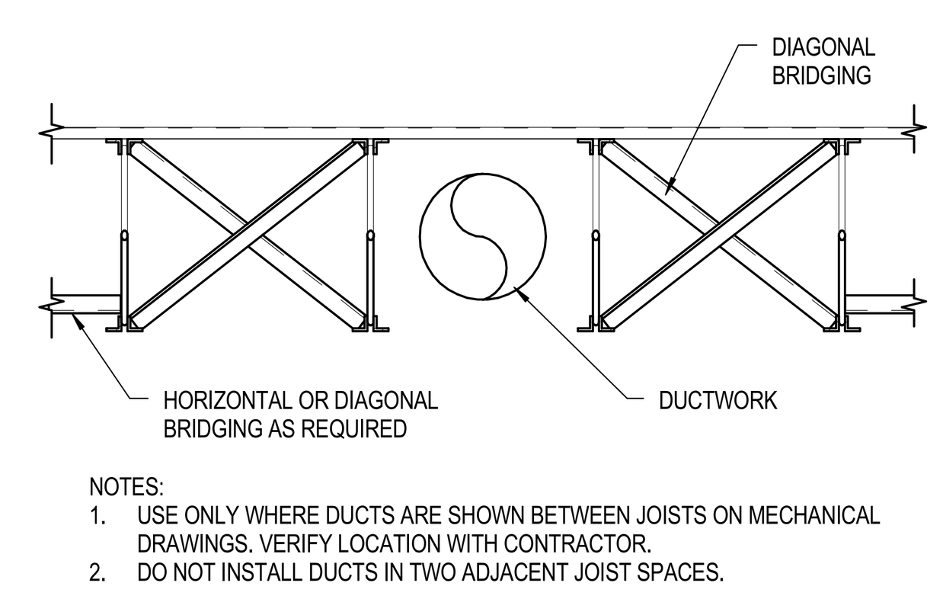
2/21/2016 12:18:26 PM



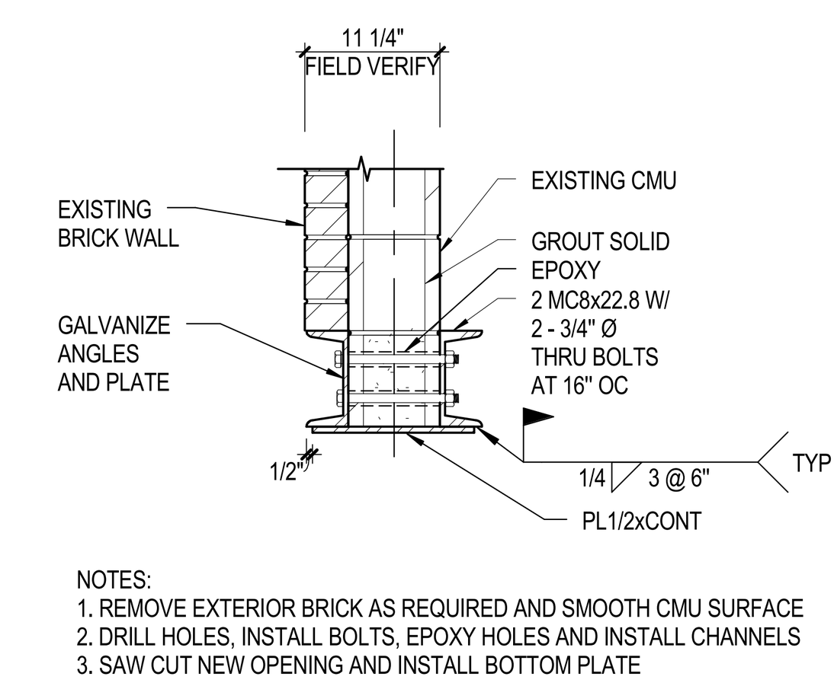
11 S5.1 TYP BRICK EDGE ANGLE DETAIL
NO SCALE (AT HORIZONTAL JOINT)



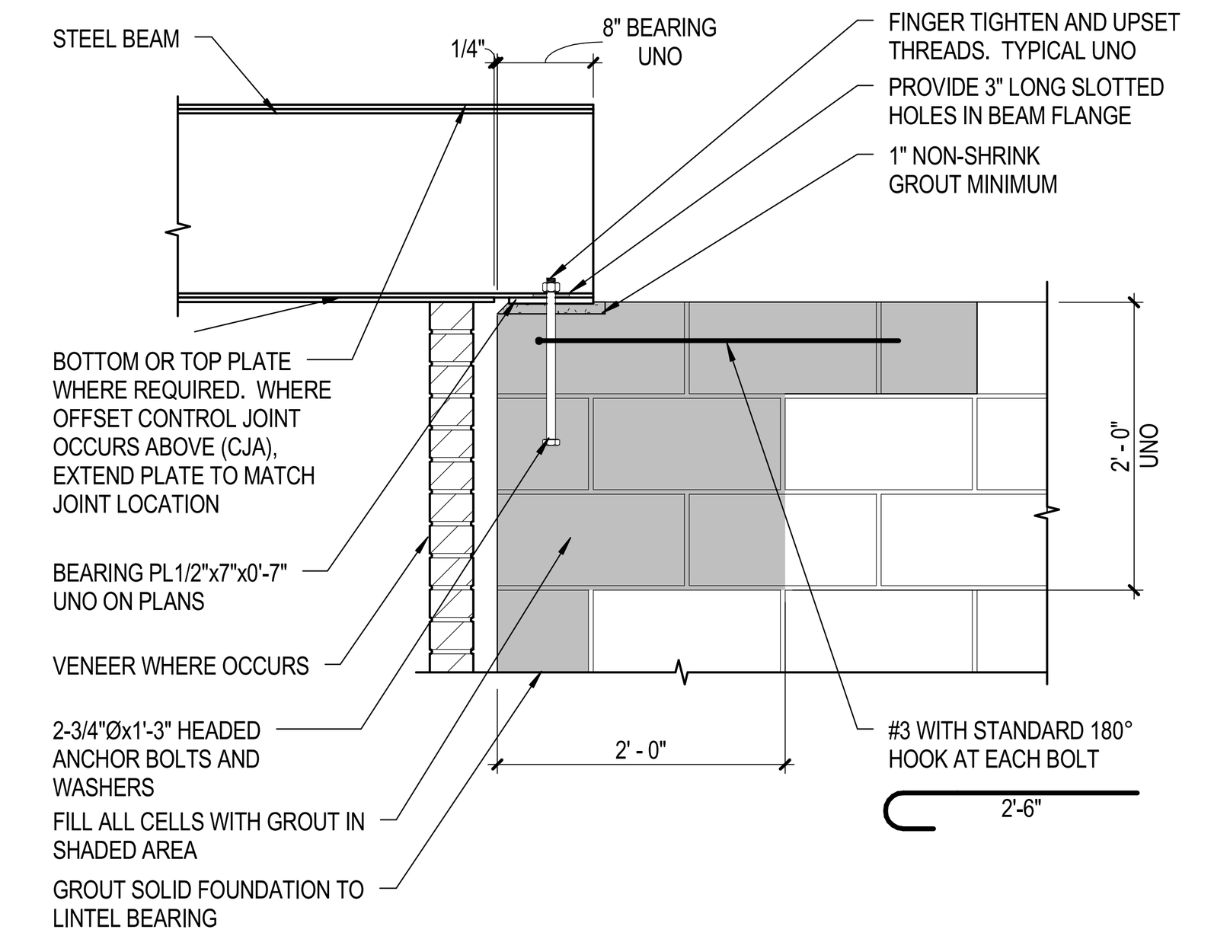
12 S5.1 TYP DECK TO WALL DETAIL
NO SCALE (AT CMU WALL, 1 SIDE)



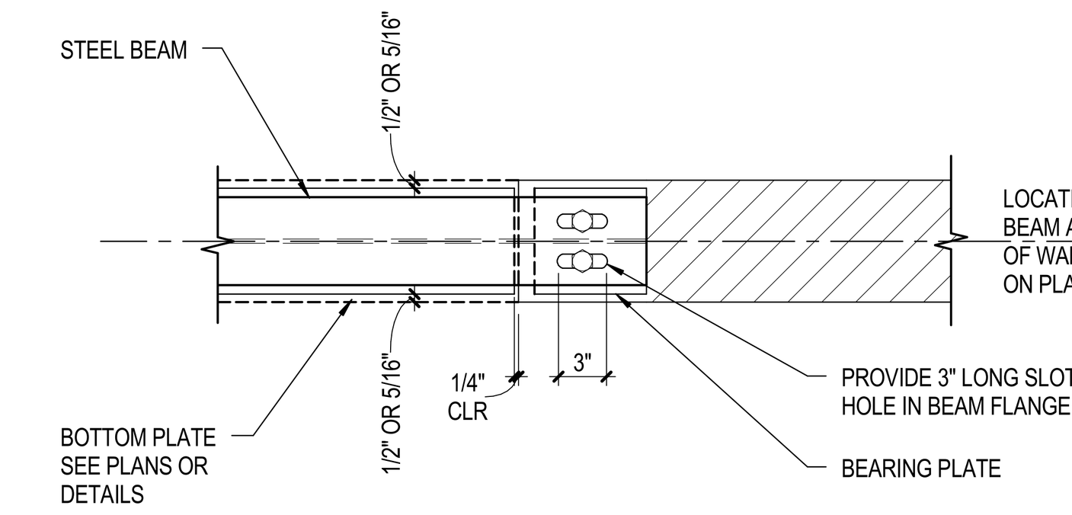
13 S5.1 TYP BRIDGING INTERRUPTION DETAIL
NO SCALE (AT DUCTWORK)



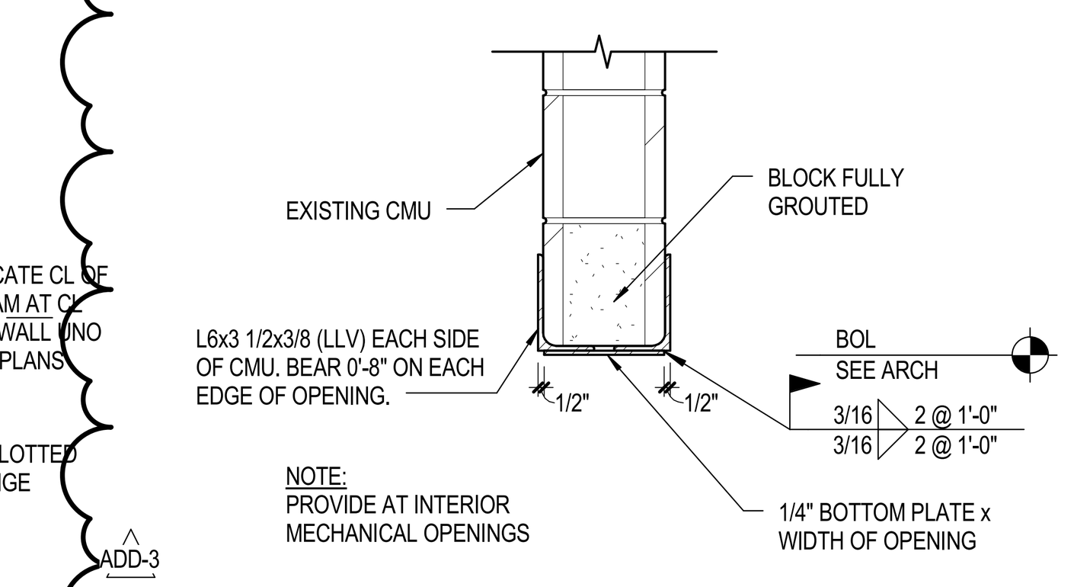
14 S5.1 TYP NEW OPENING IN EXISTING
NO SCALE



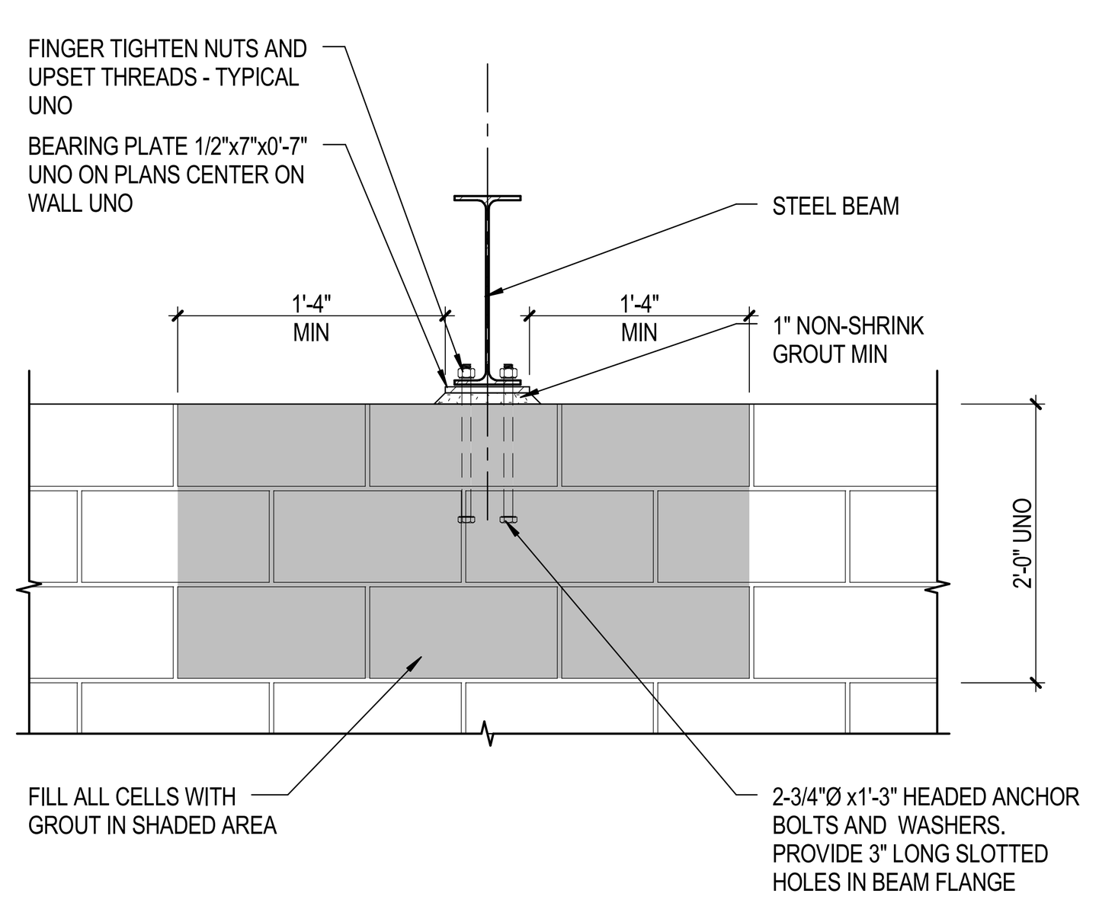
15 S5.1 TYP BEARING STEEL ON MASONRY
NO SCALE (PARALLEL TO WALL)



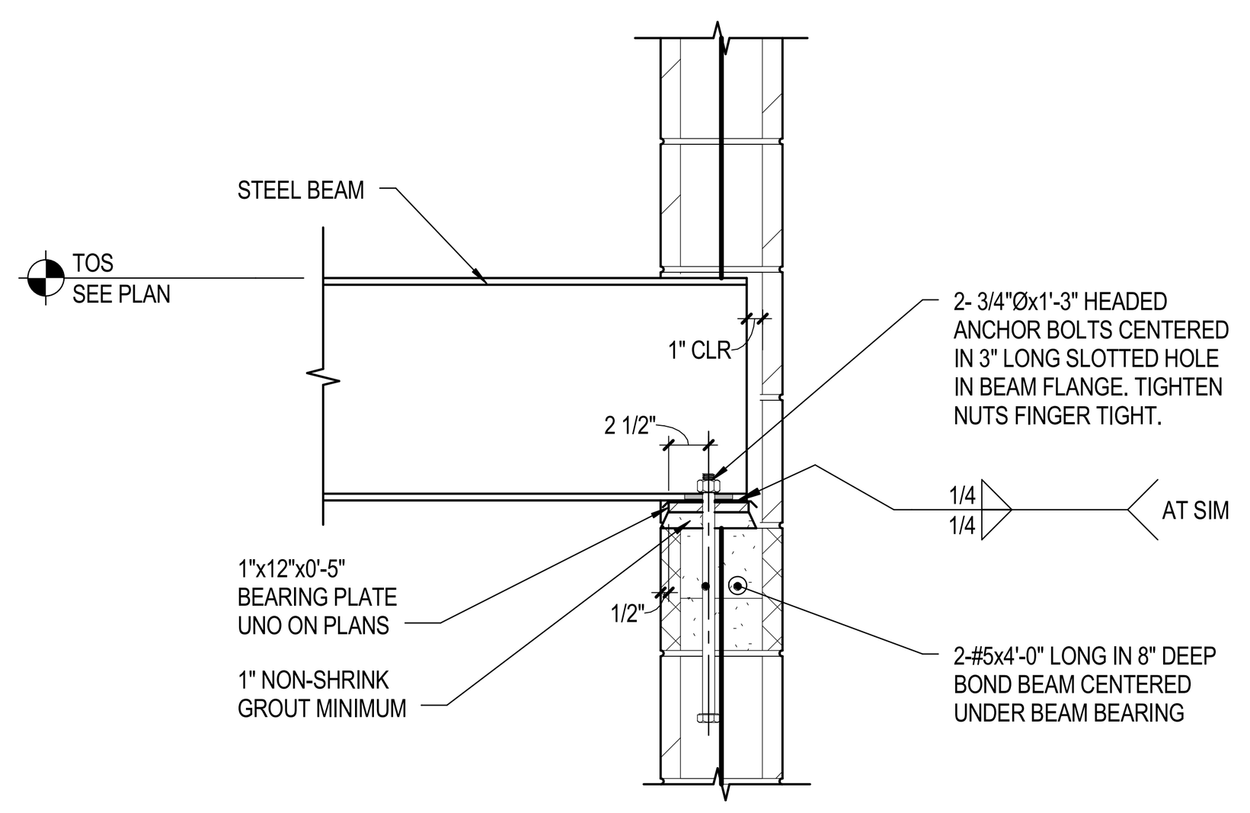
16 S5.1 TYP STEEL BEAM BEARING DETAIL
NO SCALE



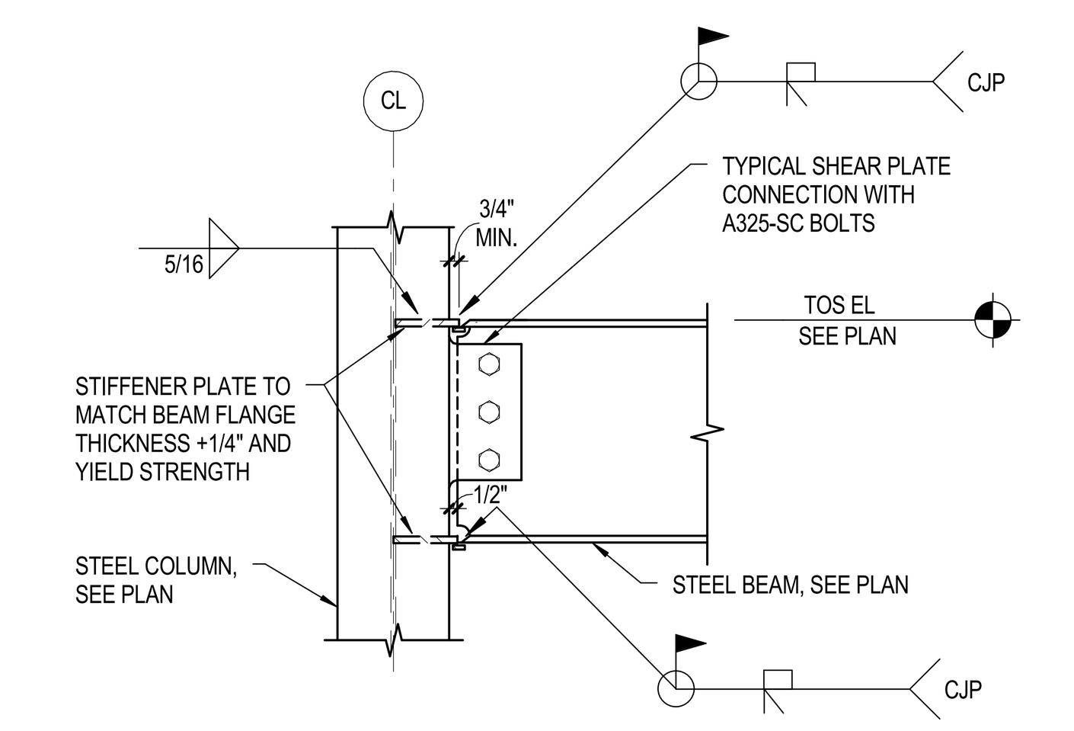
17 S5.1 TYP MECHANICAL OPENING DETAIL
NO SCALE (OPENING WIDTH ≤ 5'-0")



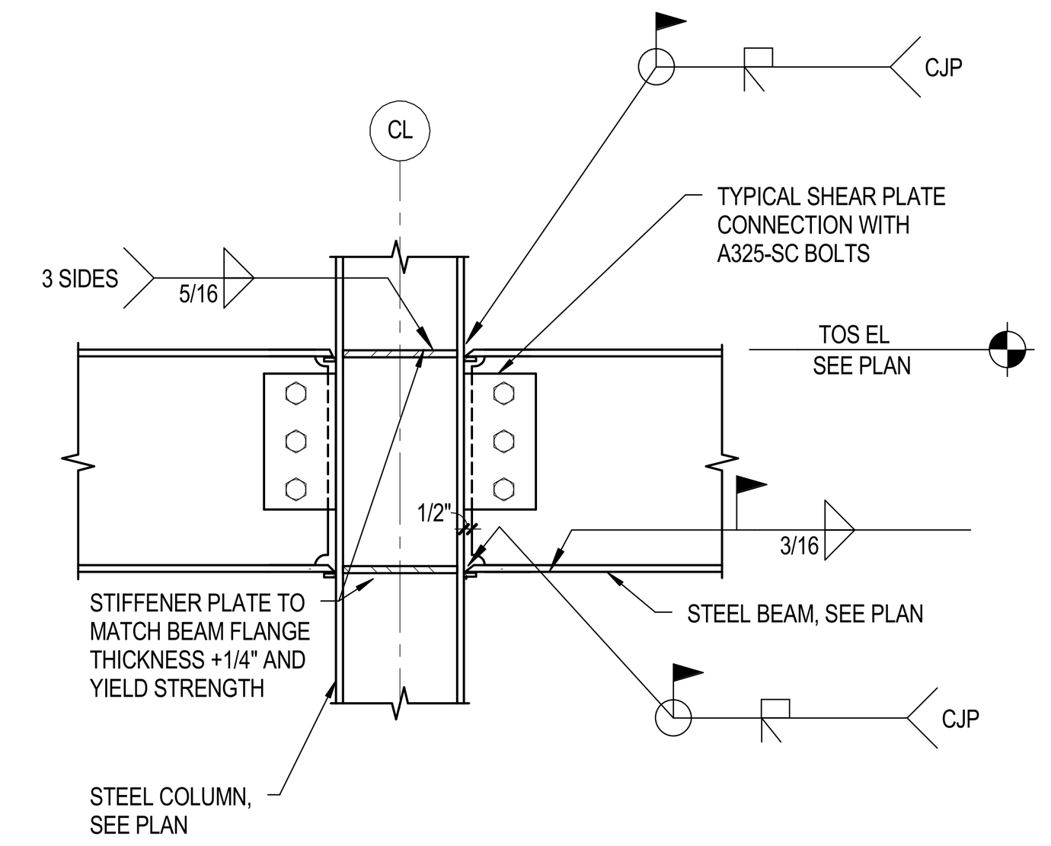
21 S5.1 TYP BEARING STEEL ON MASONRY
NO SCALE (PERPENDICULAR WALL)



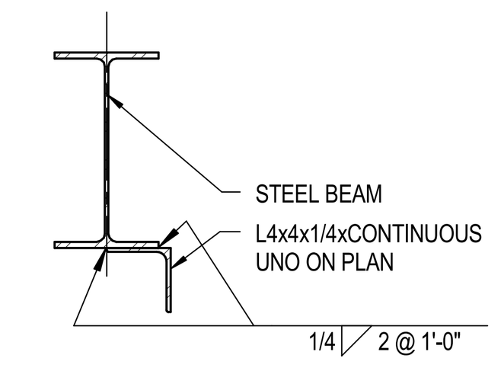
22 S5.1 STEEL BEAM AT MASONRY WALL
NO SCALE



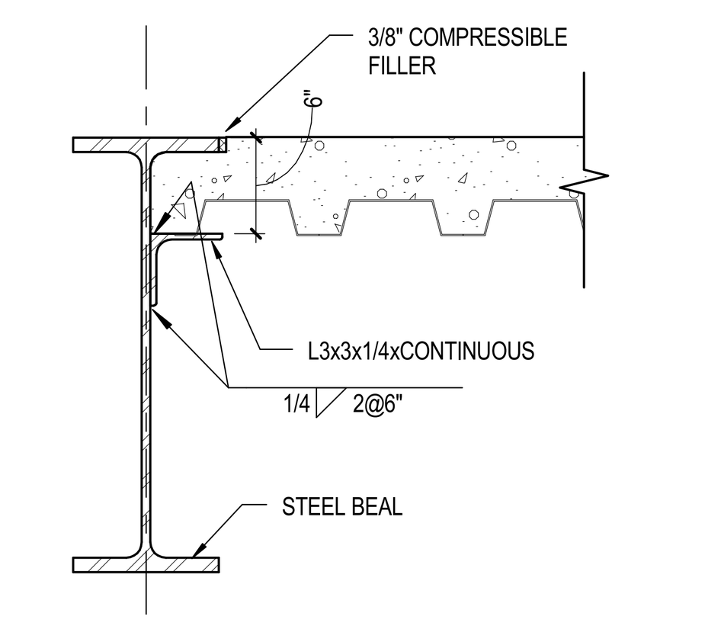
23 S5.1 BEAM / COLUMN CONNECTION DETAIL
NO SCALE (AT RIGID BEAM / COLUMN)



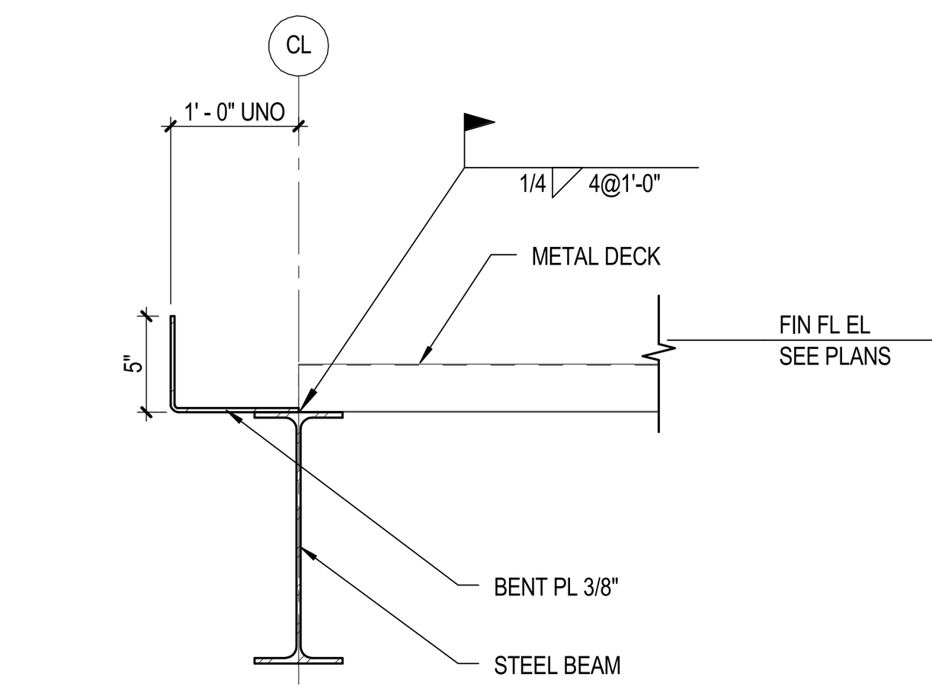
24 S5.1 BEAM / COLUMN CONNECTION DETAIL
NO SCALE (AT RIGID BEAM / COLUMN)



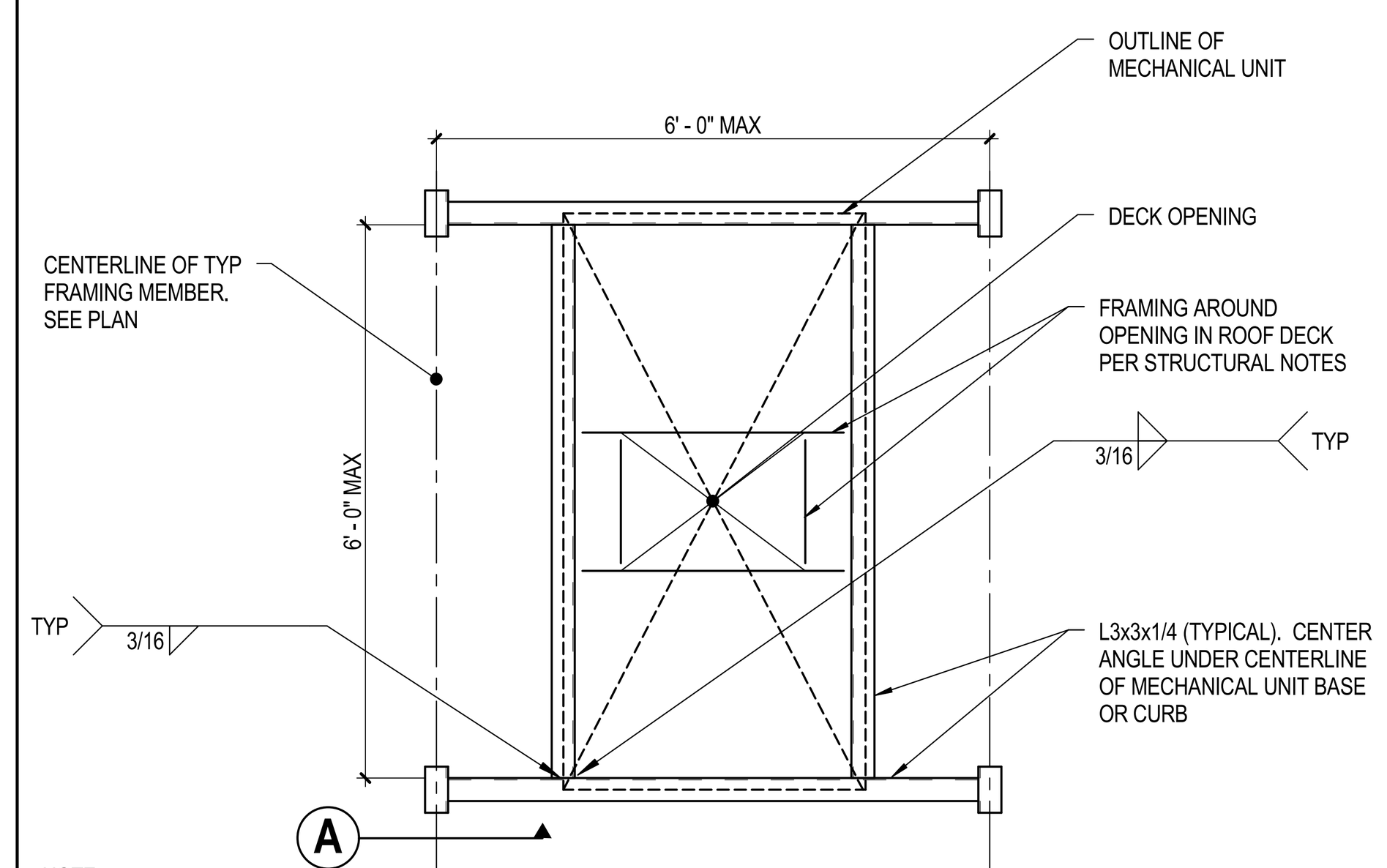
25 S5.1 SECTION
NO SCALE



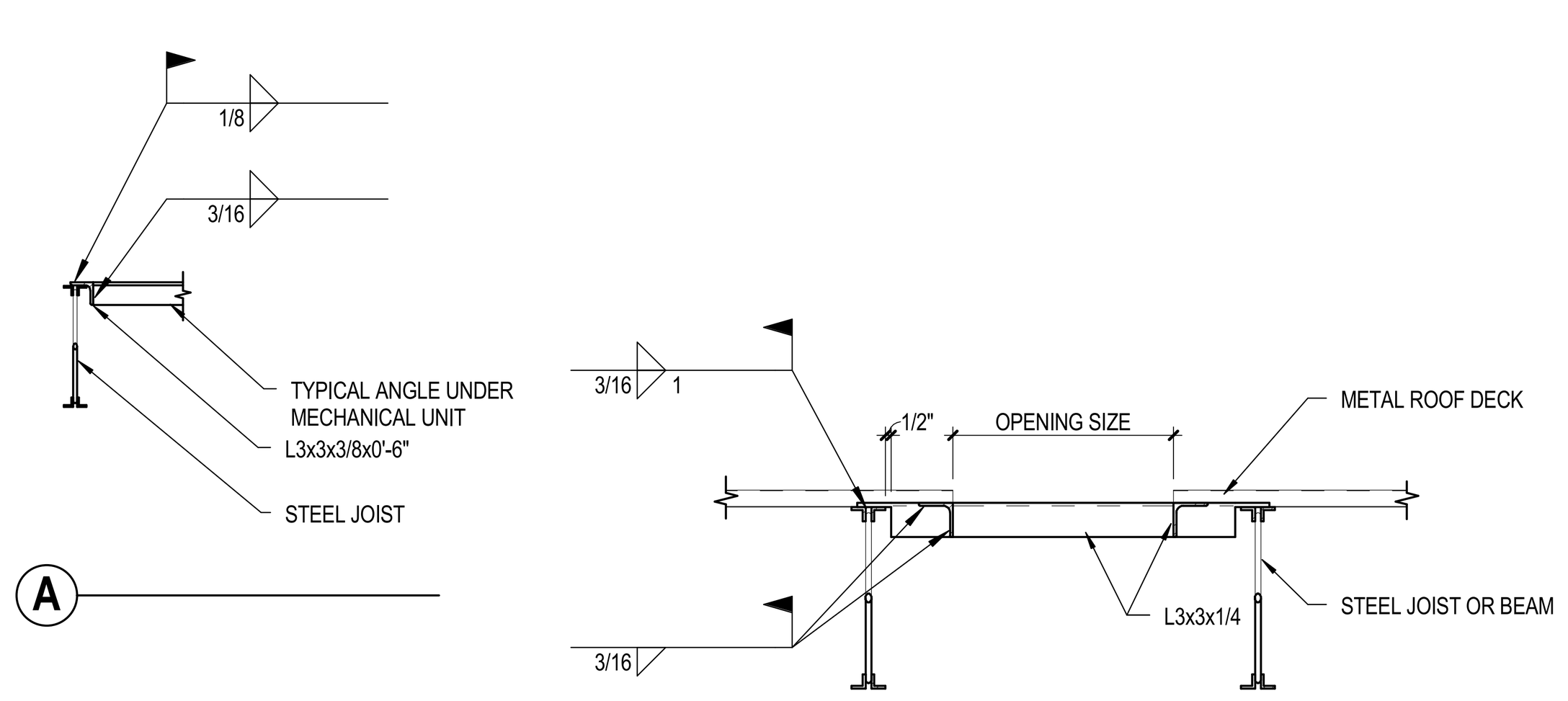
26 S5.1 CONNECTION DETAIL
NO SCALE



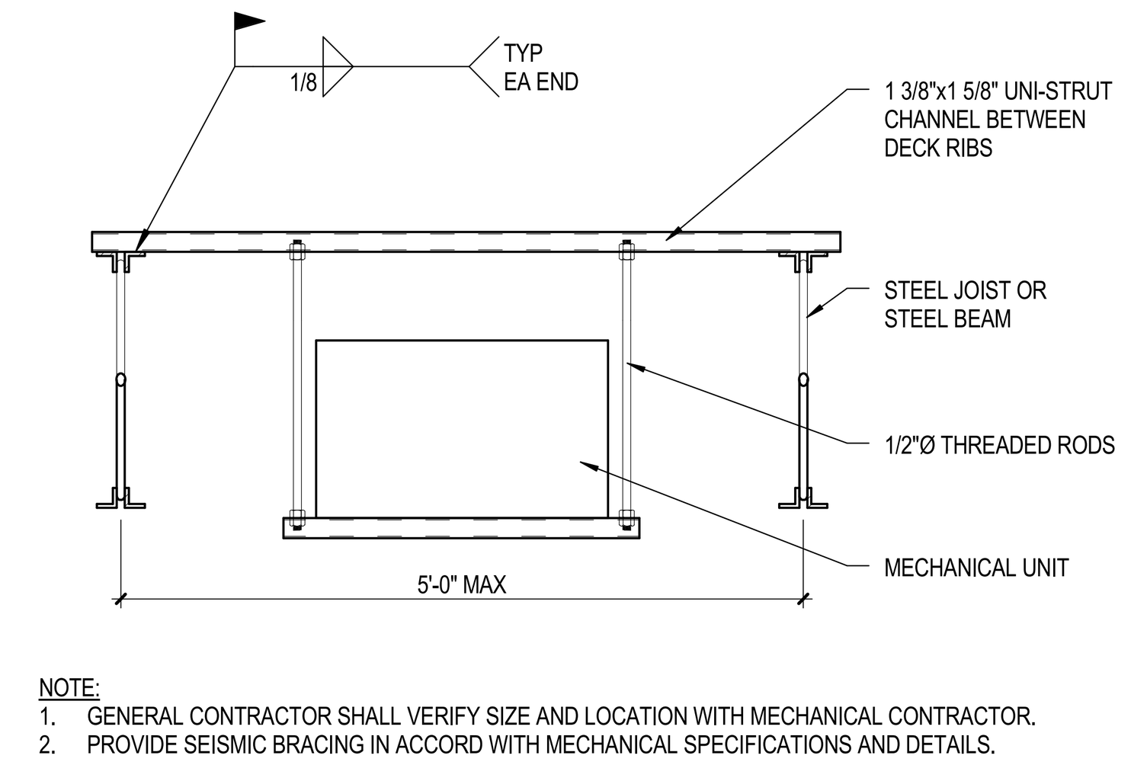
27 S5.1 SLAB EDGE DETAIL
NO SCALE



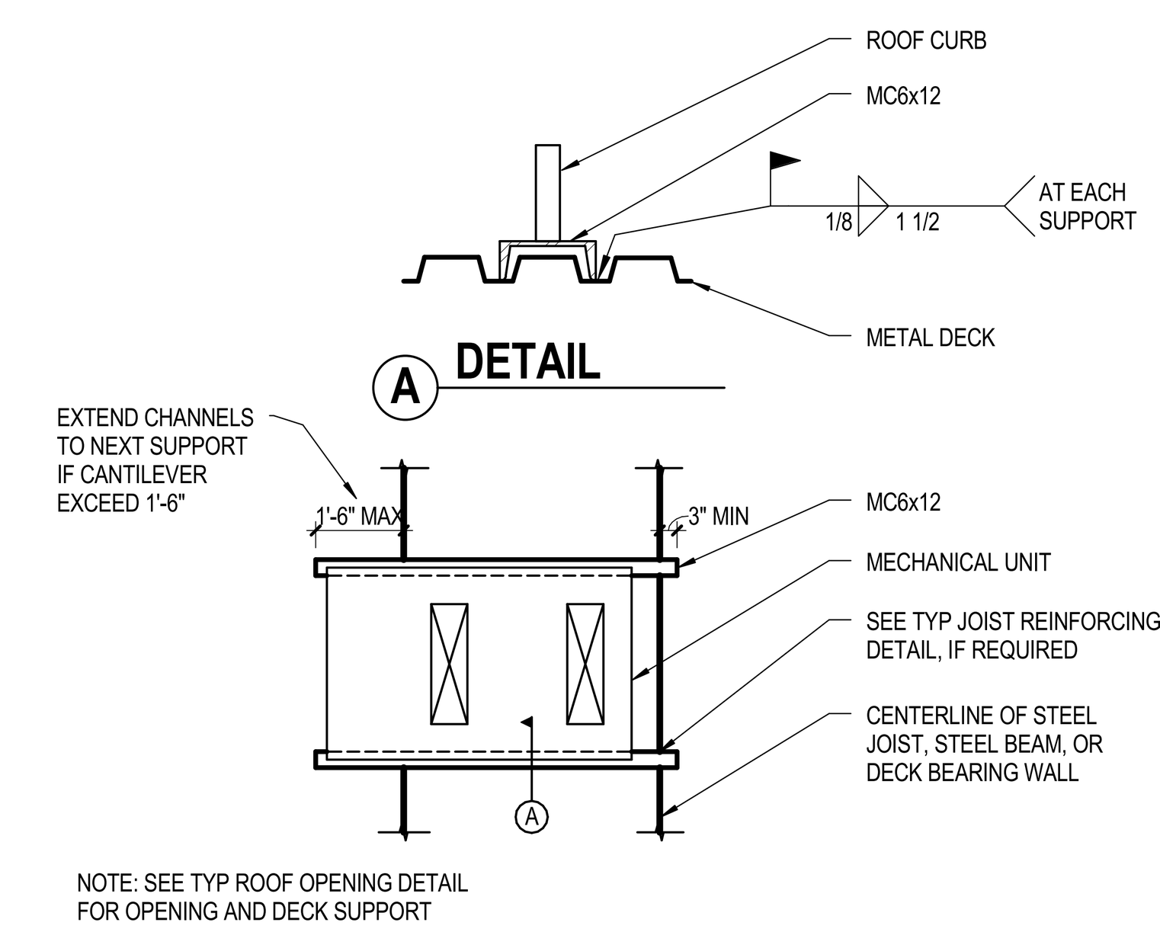
31 S5.1 TYP FRAMING AT SMALL MECHANICAL UNIT
NO SCALE (AT ROOF DECK)



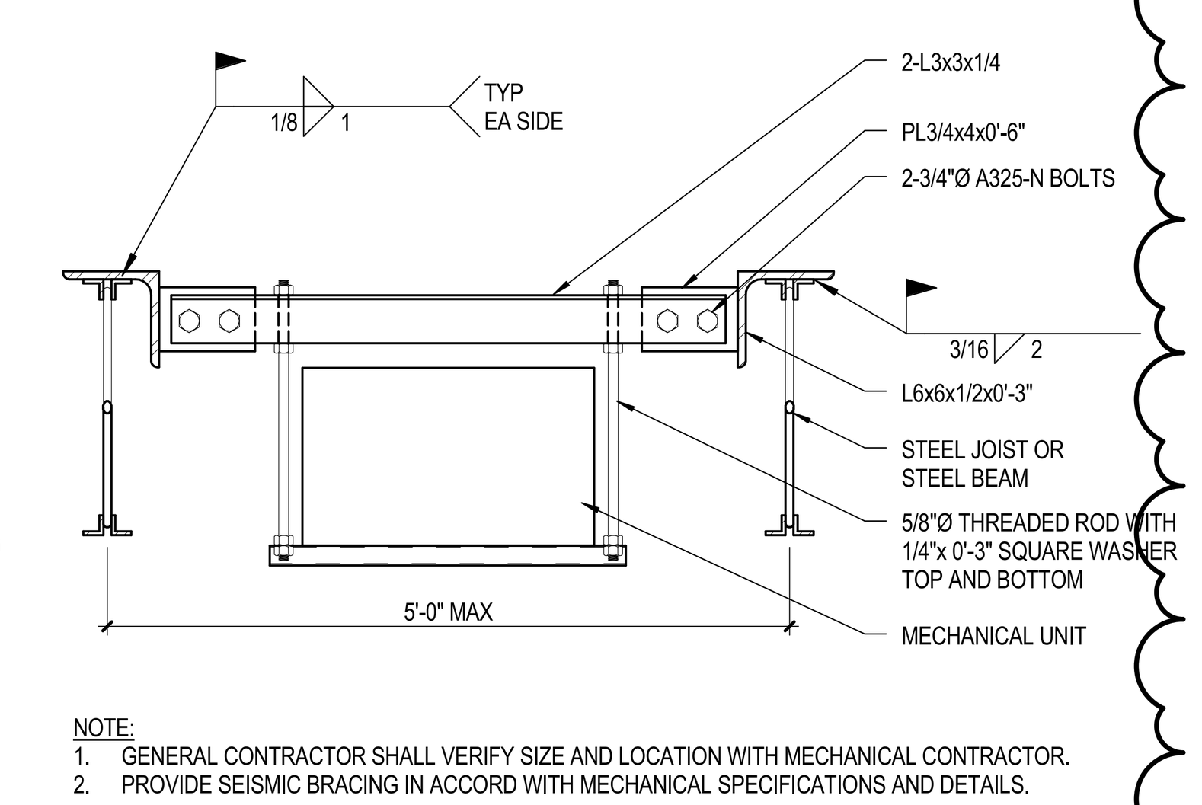
33 S5.1 TYP ROOF OPENING DETAIL
NO SCALE



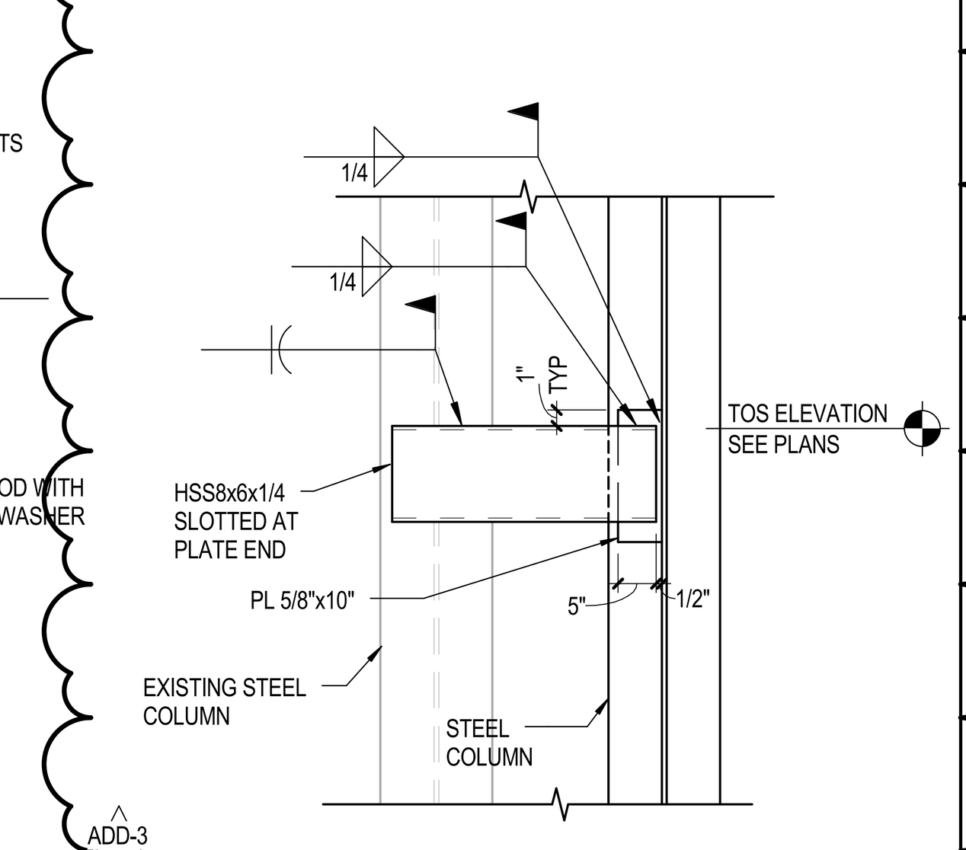
34 S5.1 TYP HANGING MECHANICAL UNIT DETAIL
NO SCALE (WT ≤ 500 LBS/CHANNEL)



35 S5.1 TYP ROOF TOP UNIT SUPPORT DETAIL
NO SCALE



36 S5.1 TYP HANGING MECHANICAL UNIT DETAIL
NO SCALE (500 LBS <WT < 800 LBS/ANGLE)



37 S5.1 CONNECTION DETAIL
SCALE: 1" = 1'-0"

CONNECTION SCHEDULE (LRFD)				
EACH BM SIZE	NO OF BOLTS	PLATE SIZE	WELD SIZE	ΦRn SHEAR CAPACITY
W8, W10	2	1/4"	3/16"	19K
W12, W14	3	1/4"	3/16"	35K
W16, W18	4	5/16"	1/4"	59K
W21	5	5/16"	1/4"	79K
W24	6	5/16"	1/4"	95K
W27	7	5/16"	1/4"	111K
W30, W33	8	5/16"	1/4"	127K
W36	9	5/16"	1/4"	142K

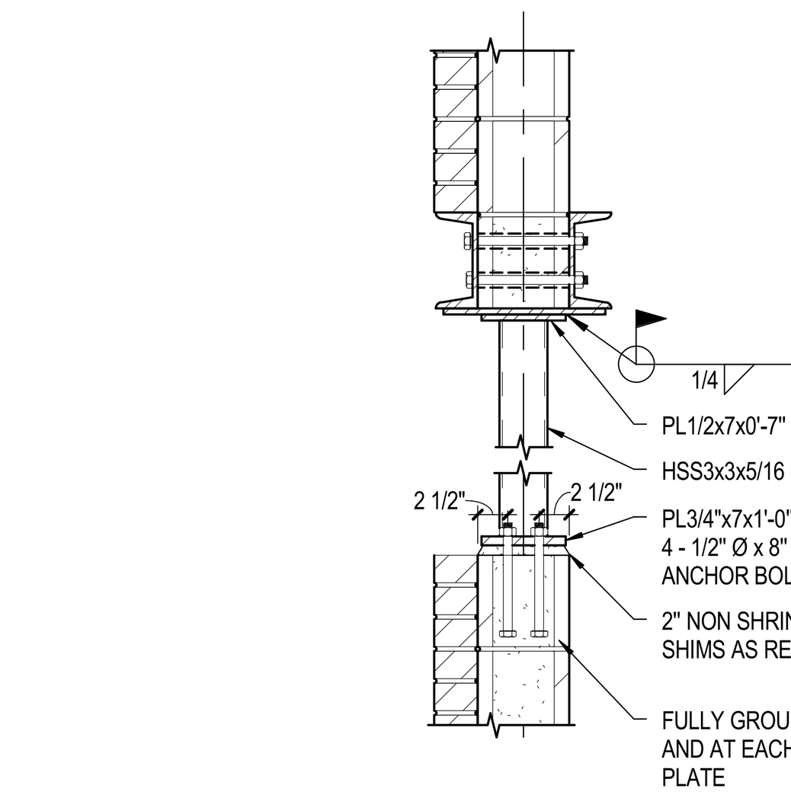
NOTES:
1. BOLTS ARE 3/4"Ø A325-N, HOLES ARE 13/16"Ø STANDARD HOLES.
2. LRFD CAPACITIES PER TABLE 10-9a, AISC STEEL CONSTRUCTION MANUAL, 13TH EDITION.

CONNECTION SCHEDULE (LRFD)				
BM SIZE	NO OF BOLTS	L=ANGLE LENGTH	ANGLE THICKNESS	ΦRn SHEAR CAPACITY
W8, W10	2	5 1/2"	1/4"	19K
W12, W14	3	8 1/2"	1/4"	35K
W16, W18	4	11 1/2"	1/4"	59K
W21	5	14 1/2"	1/4"	125K
W24	6	17 1/2"	1/4"	141K
W27	7	20 1/2"	5/16"	193K
W30, W33	8	23 1/2"	5/16"	227K
W36	9	26 1/2"	5/16"	278K

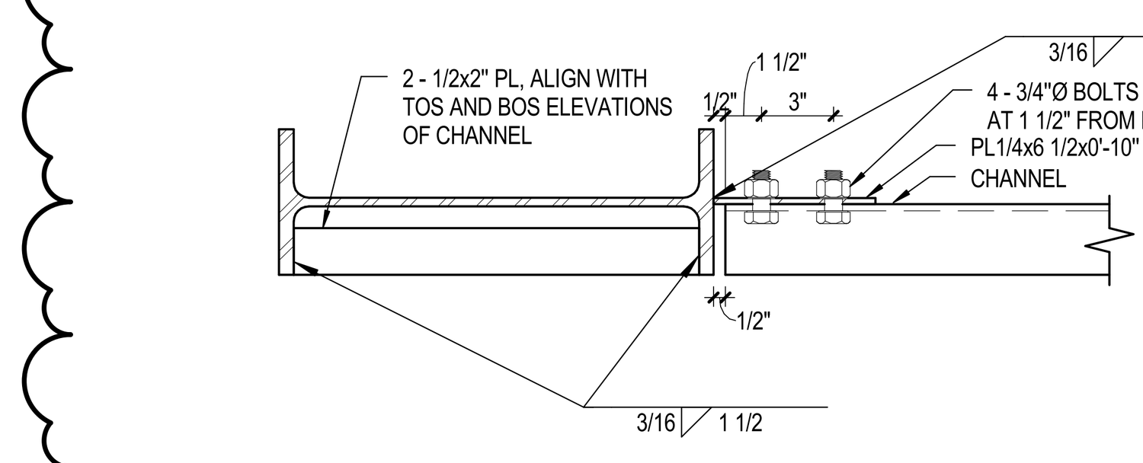
NOTES:
1. BOLTS ARE 3/4"Ø A325-N, HOLES ARE 13/16"Ø.
2. USE SIMILAR CONNECTION AT WIDE FLANGE COLUMN FLANGE OR WEB.
3. LRFD CAPACITIES PER TABLES 10-1 AND 10-2, AISC STEEL CONSTRUCTION MANUAL, 13TH EDITION.
4. STEEL BEAMS ARE ASTM A992, ANGLES ARE ASTM A36.

CONNECTION SCHEDULE (LRFD)				
BM SIZE	NO OF BOLTS	L=PLATE LENGTH	PLATE THICKNESS	SHEAR CAPACITY Φ RN
W8, W10	2	5 1/2"	1/4"	24K
W12, W14	3	8 1/2"	1/4"	52K
W16, W18	4	11 1/2"	1/4"	89K
W21	5	14 1/2"	1/4"	130K
W24	6	17 1/2"	1/4"	154K
W27	7	20 1/2"	1/4"	179K
W30, W33	8	23 1/2"	5/16"	241K
W36	9	26 1/2"	5/16"	273K

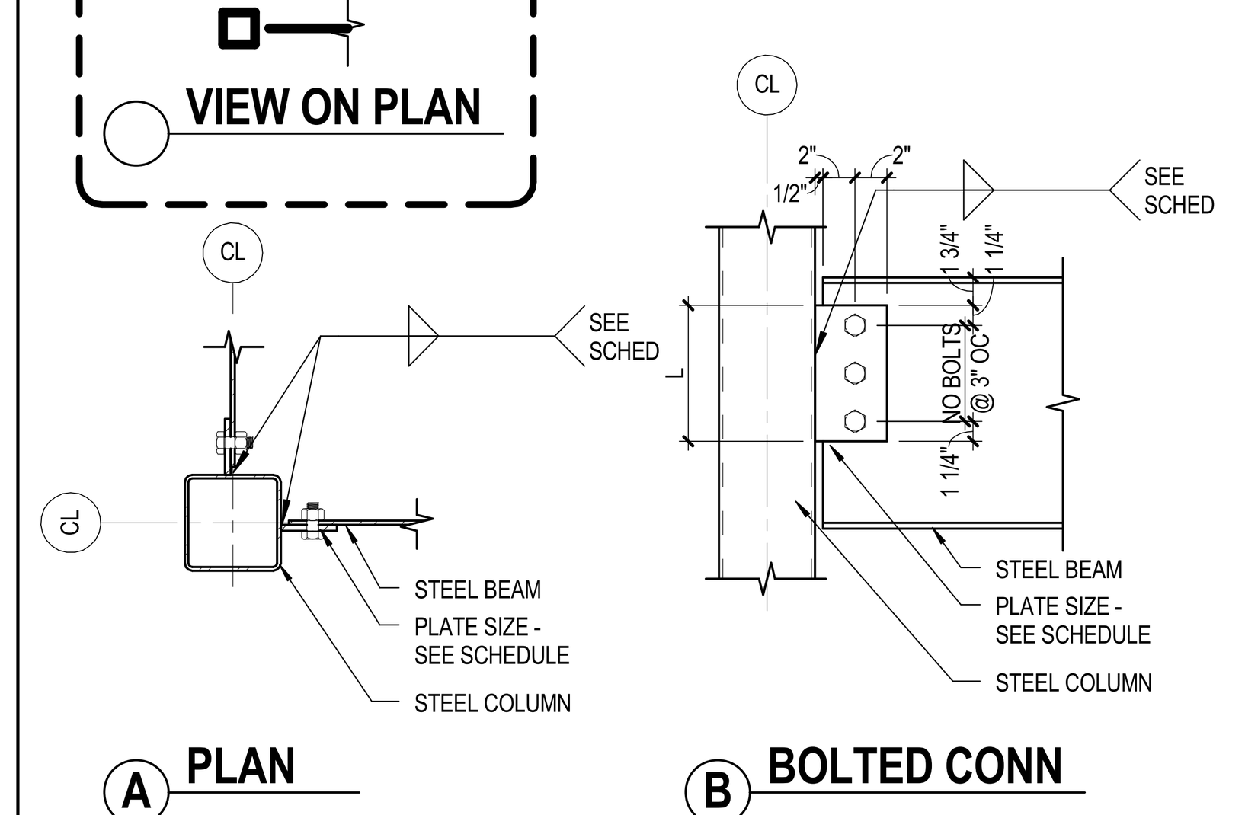
NOTES:
1. CAPACITIES ARE LRFD.



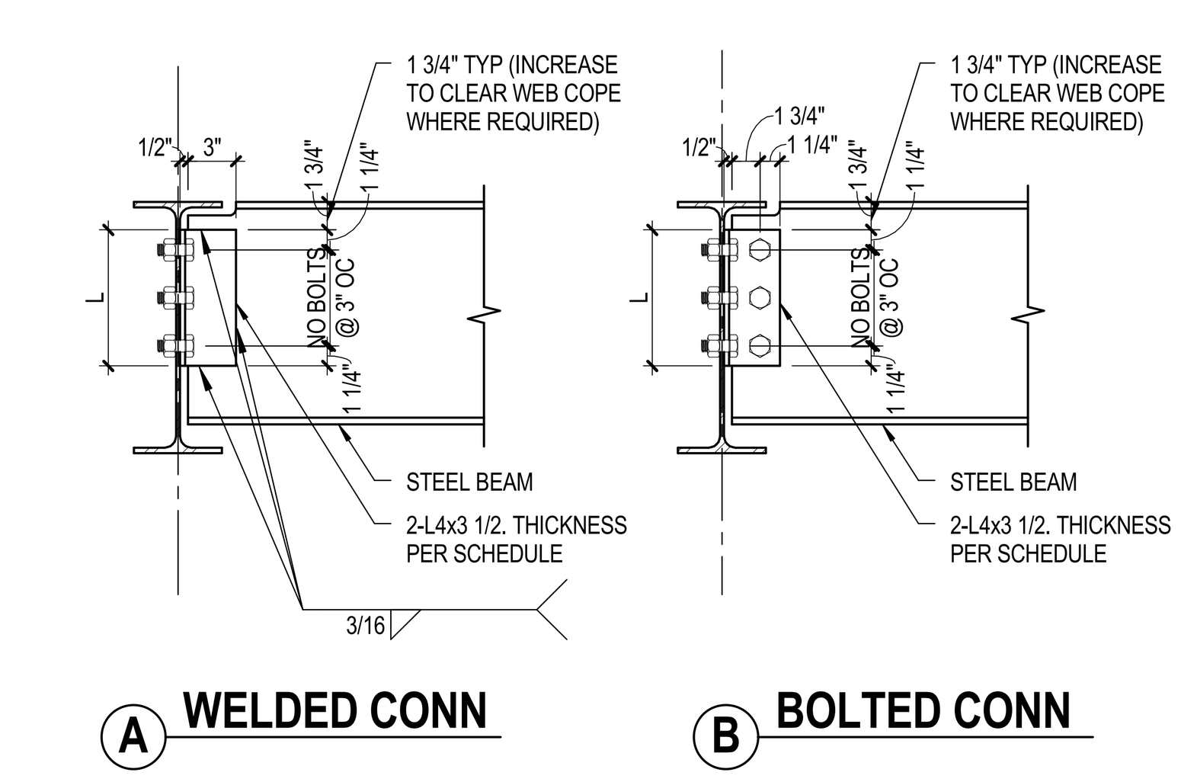
44 S5.1 DETAIL
NO SCALE



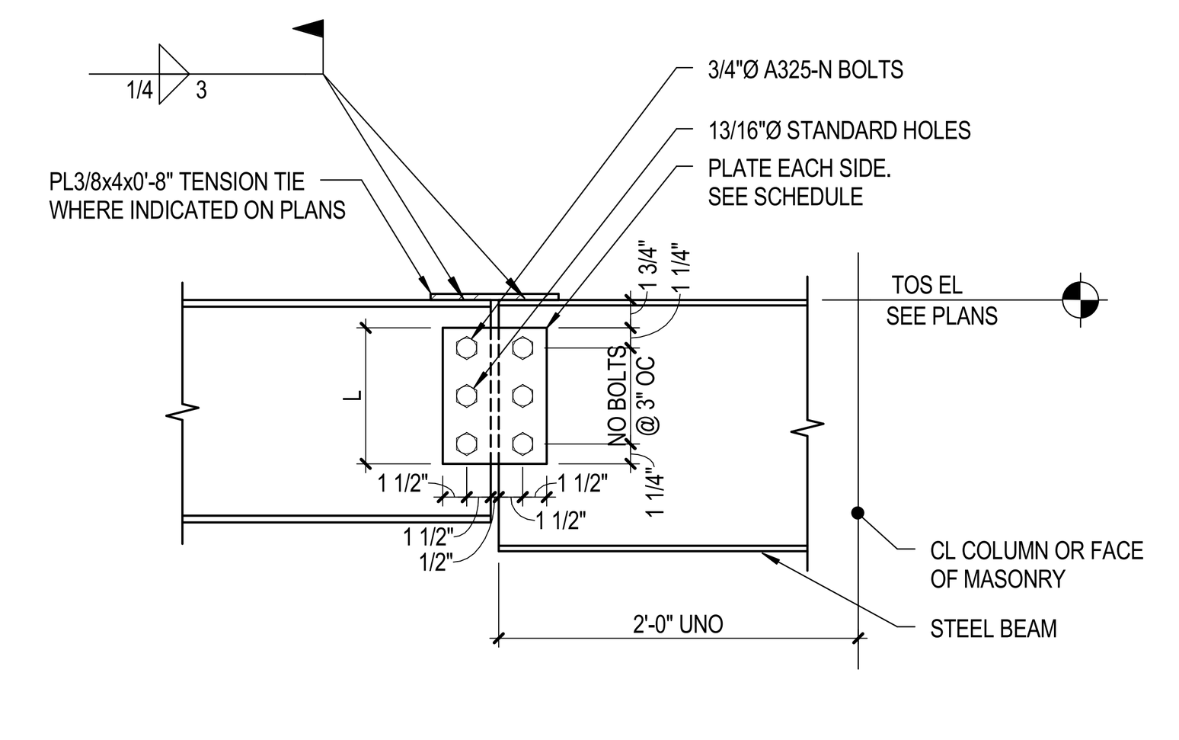
45 S5.1 CONNECTION DETAIL
NO SCALE



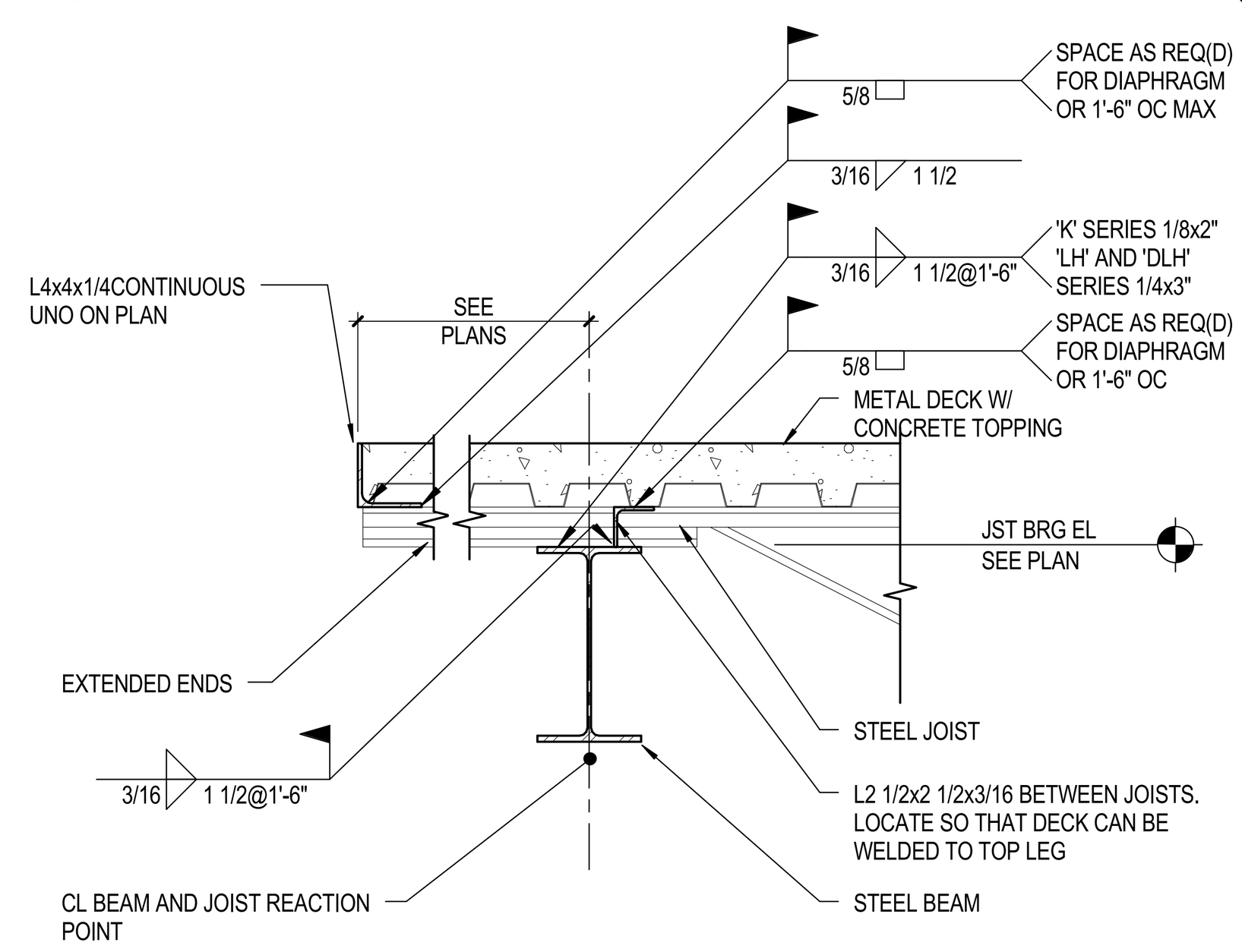
51 S5.1 TYP STEEL BEAM CONNECTION DETAIL
NO SCALE (SINGLE PLATE CONNECTION)



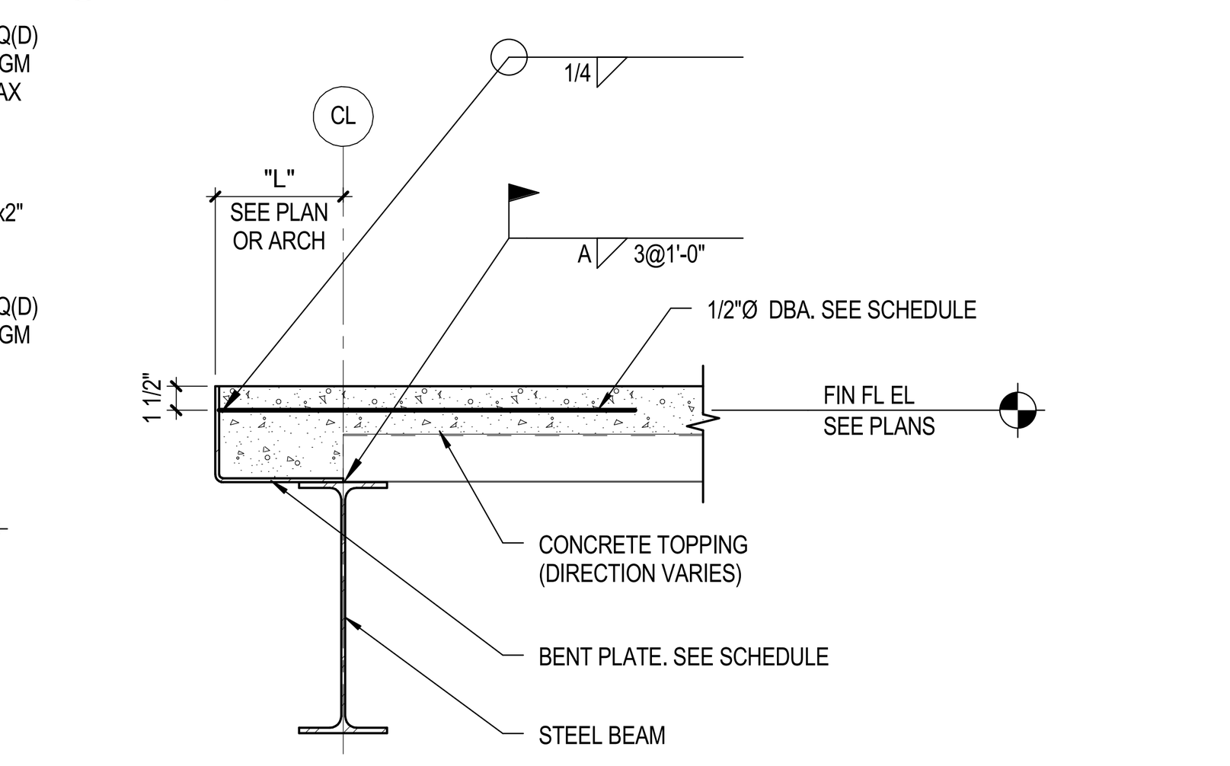
52 S5.1 TYP STEEL BEAM CONNECTION DETAIL
NO SCALE (CLIP ANGLE CONNECTION)



53 S5.1 TYP BEAM SPLICE DETAIL
NO SCALE (STEEL PLATE SPLICE)

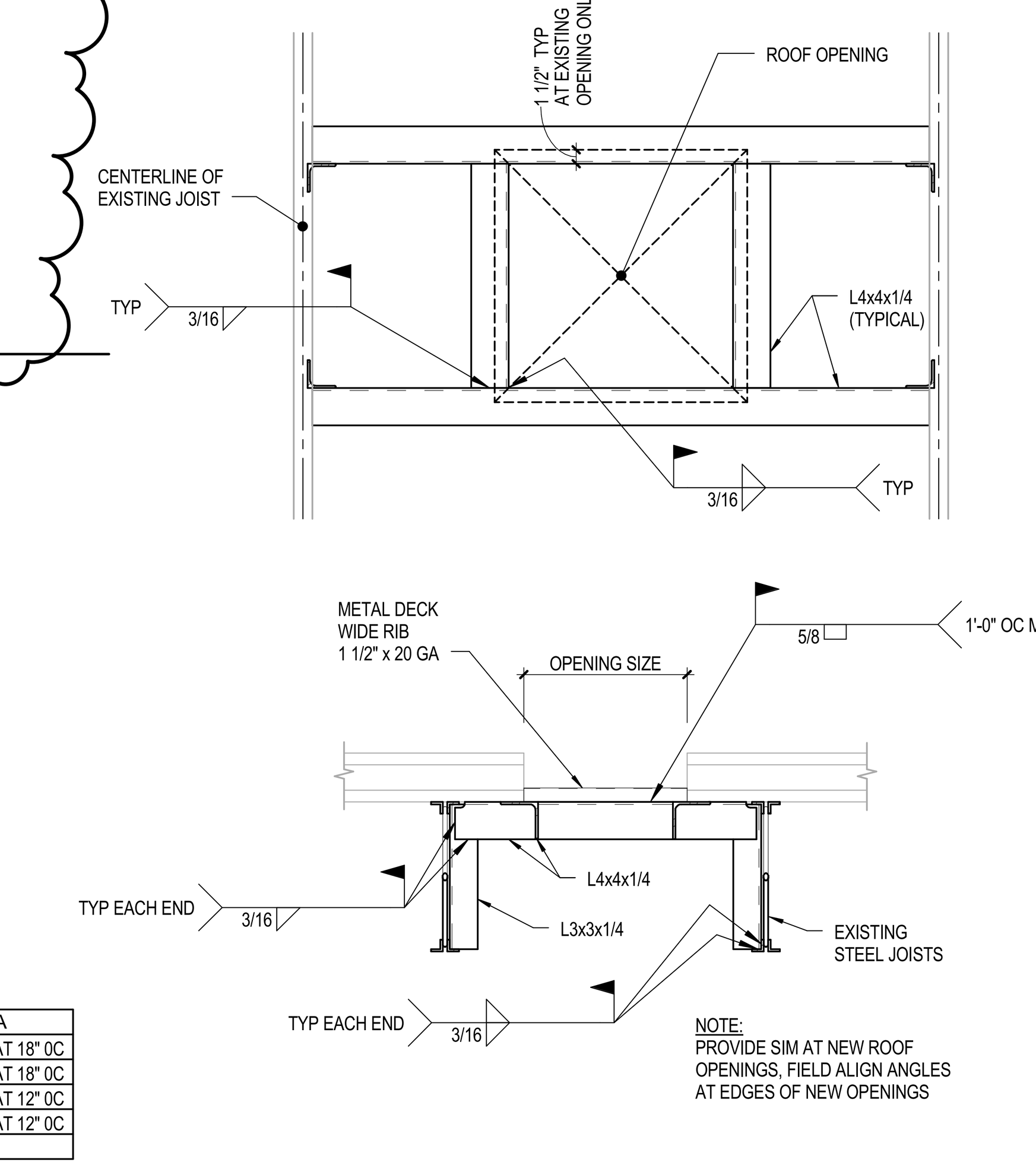


54 S5.1 STEEL JOIST EXTENDED ENDS
NO SCALE



"L"	PLATE THICKNESS	A	DBA
0"-6"	1/4"	3/16	1/2"Øx2'-0" AT 18" OC
7"-12"	1/4"	3/16	1/2"Øx2'-6" AT 18" OC
13"-18"	5/16"	1/4	1/2"Øx3'-6" AT 12" OC
19"-24"	5/16"	1/4	1/2"Øx4'-0" AT 12" OC

55 S5.1 TYP SLAB EDGE DETAIL
NO SCALE



56 S5.1 TYP ROOF PATCH DETAIL
NO SCALE

Attachment No. S5.1-1
to: Addendum #3
Dated: 03/21/2016

STRUCTURAL DETAILS
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

S5.1
10-16202-00
2/24/2016

DLR Group
Architecture Engineering Planning Interiors

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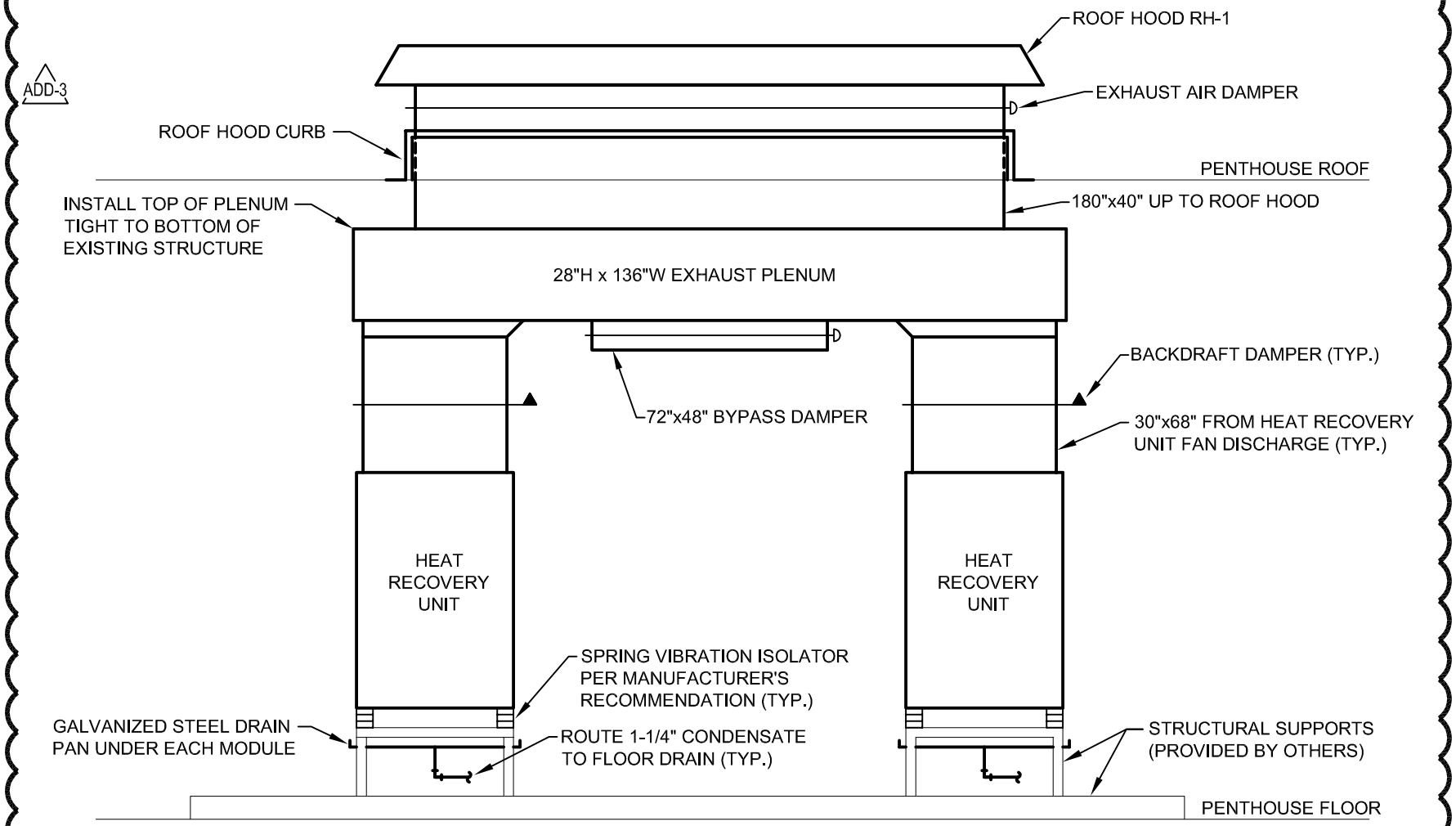

9 / M5.2

PENTHOUSE HEAT RECOVERY UNIT DETAIL

NOT TO SCALE

DWG: F:\Projects\015-2779\40-Design\AutoCAD\Final Plans\Sheets\MECH\Mechanical\M_DTL_52779.dwg USER: trojewski
DATE: Mar 21, 2016 10:32am XREFS: A_MTBLK3042_52779 Xref - B 5x11H

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(Sheet Number) **M5.2**
(Project Number) 015-2779
(Drawing Number)
(Date) 02-26-2016

MECHANICAL DETAILS AND GAS RISER
TELEGRAPH DISTRICT
401 BUILDING RENOVATION

ADD-3 Attachment No. M5.2
Addendum 3
Dated: March 21, 2016