ADDENDUM #1

KEARNEY PUBLIC SCHOOLS
NEW KEARNEY HIGH SCHOOL - FOUNDATION PACKAGE
KEARNEY, NEBRASKA
1355-F

Wilkins Hinrichs Stober Architects, L.L.C.
2908 West 39th Street, Suite A
Kearney, Nebraska 68845
308-237-5787

Date Issued: September 12, 2014

Bid Date: September 17, 2014

TO ALL BID DOCUMENT HOLDERS OF RECORD:

Acknowledge receipt of this addendum by inserting its number in the space provided on the BID FORM. Failure to do so may subject Bidder to disqualification. This Addendum forms a part of the BIDDING DOCUMENTS and modifies them as follows.

- -

Addendum #1 - #1 **SPECIFICATION SECTION - 00 8200 - GEO-TECHNICAL REPORT**
Add attached Geo-Technical Specification Section to the contract documents.

Addendum #1 - #2 **SHEET A1.01 – FLOOR PLAN – FIRST FLOOR - AREA A**
Substitute new sheet A1.01 as shown on Attachment A1 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #3 **SHEET A1.02 – FLOOR PLAN – FIRST FLOOR - AREA AA**
Substitute new sheet A1.02 as shown on Attachment A2 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #4 **SHEET A1.03 – FLOOR PLAN – FIRST FLOOR - AREA B**
Substitute new sheet A1.03 as shown on Attachment A3 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #5 **SHEET A1.04 – FLOOR PLAN – FIRST FLOOR - AREA C**
Substitute new sheet A1.04 as shown on Attachment A4 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #6 **SHEET A1.05 – FLOOR PLAN – FIRST FLOOR - AREA D**
Substitute new sheet A1.05 as shown on Attachment A5 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #7 **SHEET A1.06 – FLOOR PLAN – FIRST FLOOR - AREA E & F**
Substitute new sheet A1.06 as shown on Attachment A6 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #8 **SHEET A1.07 – FLOOR PLAN – FIRST FLOOR - AREA G**
Substitute new sheet A1.07 as shown on Attachment A7 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #9 **SHEET A1.08 – FLOOR PLAN – FIRST FLOOR - AREA H**
Substitute new sheet A1.08 as shown on Attachment A8 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #10 **SHEET A1.09 – FLOOR PLAN – FIRST FLOOR - AREA J**
Substitute new sheet A1.09 as shown on Attachment A9 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #11 **SHEET A1.10 – FLOOR PLAN – FIRST FLOOR - AREA K**
Substitute new sheet A1.10 as shown on Attachment A10 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #12 **SHEET A1.11 – FLOOR PLAN – FIRST FLOOR - AREA L**
Substitute new sheet A1.11 as shown on Attachment A11 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #13 **SHEET A1.12 – FLOOR PLAN – FIRST FLOOR - AREA M**

a. Substitute new sheet A1.12 as shown on Attachment A12 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.
b. GRANDSTAND - The grandstand footings are to be designed and provided by the grandstand contractor. Nothing for the grandstand should be included in the foundation package bidding.

Addendum #1 - #14 **SHEET A1.13 – FLOOR PLAN – FIRST FLOOR - AREA N**
Substitute new sheet A1.13 as shown on Attachment A13 to Addendum No. 1, dated September 12, 2014. Drawing modified to show stoops and stoop dimensions.

Addendum #1 - #15 **SHEET A1.60 – FLOOR PLAN – ARCHITECTURAL GENERAL NOTES & ASSEMBLIES**
Add new sheet A1.60 as shown on Attachment A14 to Addendum No. 1, dated September 12, 2014. This drawing sheet is included for reference to show the exterior and interior wall widths, which the assemblies correspond to the assembly tags shown on the architectural floor plans. If no wall assembly tag is shown for a wall, then the default interior wall assembly is Type “A” (8” CMU).

Addendum #1 - #16 **SHEET A7.01 – FLOOR PLAN – WALL SECTIONS – AREA A**
Substitute new sheet A7.01 as shown on Attachment A15 to Addendum No. 1, dated September 12, 2014. This drawing sheet is included for reference to show the extents of rigid insulation required at exterior footings/foundations.

Addendum #1 - #17 **SHEET S1.1 – FOUNDATION PLAN – AREA A**

c. Modify drawing as shown on Attachment S1.1-1 to Addendum No. 1, dated September 12, 2014. Drawing modified to show spread footing changes.
d. Modify drawing as shown on Attachment S1.1-2 to Addendum No. 1, dated September 12, 2014. Drawing modified to show spread footing changes.

Addendum #1 - #18 **SHEET S1.2 – FOUNDATION PLAN – AREAS B & AA**

a. Modify drawing as shown on Attachment S1.2-1 to Addendum No. 1, dated September 12, 2014. Drawing modified to add stoop.
b. Modify drawing as shown on Attachment S1.2-2 to Addendum No. 1, dated September 12, 2014. Drawing modified to adjust extent of stoop.

Addendum #1 - #19 **SHEET S1.3 – FOUNDATION PLAN – AREA C**

a. Modify drawing as shown on Attachment S1.3-1 to Addendum No. 1, dated September 12, 2014. Drawing modified to adjust extent of stoop.
b. Modify drawing as shown on Attachment S1.3-2 to Addendum No. 1, dated September 12, 2014. Drawing modified to add spread footing.
c. Modify drawing as shown on Attachment S1.3-3 to Addendum No. 1, dated September 12, 2014. Drawing modified to shift spread footing.

Addendum #1 - #20 **SHEET S1.5 – FOUNDATION PLAN – AREAS E & F**
Foundation Plan – Area F - Modify Drawing as shown on Attachment S1.5-1 to Addendum No. 1, dated September 12, 2014. Drawing modified to add stoop.

Addendum #1 - #21 **SHEET S1.6 – FOUNDATION PLAN – AREA G**
Modify Drawing as shown on Attachment S1.6-1 to Addendum No. 1, dated September 12, 2014. Drawing modified to clarify elevator pit wall extent.

Addendum #1 - #22 **SHEET S1.7 – FOUNDATION PLAN – AREA H**
Substitute new sheet S1.7 as shown on Attachment S1.07-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #23 **SHEET S1.8 – FOUNDATION PLAN – AREA J**
Substitute new sheet S1.8 as shown on Attachment S1.8-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #24 **SHEET S1.9 – FOUNDATION PLAN – AREA K**
Substitute new sheet S1.9 as shown on Attachment S1.9-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #25 **SHEET S1.10 – FOUNDATION PLAN – AREA L**
Substitute new sheet S1.10 as shown on Attachment S1.10-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #26 **SHEET S1.11 – FOUNDATION PLAN – AREA M**
Substitute new sheet S1.11 as shown on Attachment S1.11-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #27 **SHEET A1.12 – FOUNDATION PLAN – AREA N**
Substitute new sheet S1.12 as shown on Attachment S1.12-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #28 **SHEET S3.1 – STRUCTURAL DETAILS**
Structural Spread Footing Schedule – Modify schedule as shown on Attachment S3.1-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #29 **SHEET S3.2 – STRUCTURAL DETAILS**
Substitute new sheet S3.2 as shown on Attachment S3.2-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #30 **SHEET S3.3 – STRUCTURAL DETAILS**
Add new sheet S3.3 as shown on Attachment S3.3-1 to Addendum No. 1, dated September 12, 2014.

Addendum #1 - #31 **SHEET E2-29 - DUCTBANK ROUTING PLAN - AREAS AA-E**
Add new sheet electrical sheet as shown on attachment.

Addendum #1 - #32 **SHEET E2-30 - DUCTBANK ROUTING PLAN - AREAS F-K**
Add new sheet electrical sheet as shown on attachment.

Addendum #1 - #33 **SHEET E2-31 - DUCTBANK ROUTING PLAN - AREAS L-N**
Add new sheet electrical sheet as shown on attachment.

SECTION 00 8200

GEO-TECHNICAL REPORT

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Attached is the GeoTechnical Exploration Report from Geotechnical Services Inc of Grand Island for New Kearney High School.

GEOTECHNICAL EXPLORATION

KEARNEY PUBLIC SCHOOLS
NEW HIGH SCHOOL
WEST 11TH STREET AND 30TH AVENUE
KEARNEY, NEBRASKA

GSI JOB NO. 145025

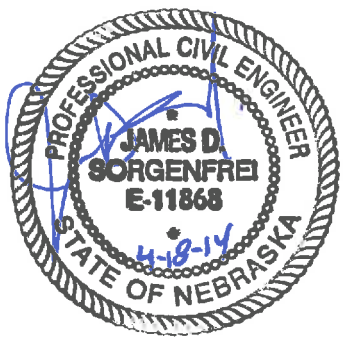
APRIL 18, 2014

Prepared By:

GSI Engineering Northern
Division, LLC
2960 North Diers Avenue
Grand Island, Nebraska
68803-1243

Prepared For:

Kearney Public Schools
Chris Nelson
310 West 24th Street
Kearney, Nebraska 68845



I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Nebraska.

James D. Sorgenfrei — 4-18-14 Date

My license renewal date is December 31, 2014.
Pages covered by this seal: 1-12, Appendices A, B, & C
Date issued: April 18, 2014

Important Information about Your

Geotechnical Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical engineering study conducted for a civil engineer may not fulfill the needs of a construction contractor or even another civil engineer. Because each geotechnical engineering study is unique, each geotechnical engineering report is unique, prepared *solely* for the client. No one except you should rely on your geotechnical engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply the report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

A Geotechnical Engineering Report Is Based on A Unique Set of Project-Specific Factors

Geotechnical engineers consider a number of unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical engineering report that was:

- not prepared for you,
- not prepared for your project,
- not prepared for the specific site explored, or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light industrial plant to a refrigerated warehouse,
- elevation, configuration, location, orientation, or weight of the proposed structure,
- composition of the design team, or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical engineering report is based on conditions that existed at the time the study was performed. *Do not rely on a geotechnical engineering report* whose adequacy may have been affected by: the passage of time; by man-made events, such as construction on or adjacent to the site; or by natural events, such as floods, earthquakes, or groundwater fluctuations. *Always* contact the geotechnical engineer before applying the report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ—sometimes significantly—from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are *Not* Final

Do not overrely on the construction recommendations included in your report. *Those recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations only by observing actual

subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's recommendations if that engineer does not perform construction observation.*

A Geotechnical Engineering Report Is Subject to Misinterpretation

Other design team members' misinterpretation of geotechnical engineering reports has resulted in costly problems. Lower that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Contractors can also misinterpret a geotechnical engineering report. Reduce that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Contractors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make contractors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give contractors the complete geotechnical engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise contractors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure contractors have sufficient time* to perform additional study. Only then might you be in a position to give contractors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and contractors do not recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations" many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform a *geoenvironmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical engineering report does not usually relate any geoenvironmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own geoenvironmental information, ask your geotechnical consultant for risk management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, a number of mold prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; ***none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.***

Rely on Your ASFE-Member Geotechncial Engineer for Additional Assistance

Membership in ASFE/The Best People on Earth exposes geotechnical engineers to a wide array of risk management techniques that can be of genuine benefit for everyone involved with a construction project. Confer with your ASFE-member geotechnical engineer for more information.

ASFE THE GEOPROFESSIONAL BUSINESS ASSOCIATION

8811 Colesville Road/Suite G106, Silver Spring, MD 20910
Telephone: 301/565-2733 Facsimile: 301/589-2017
e-mail: info@asfe.org www.asfe.org

IIGER05105.0MRP

GSI

TABLE OF CONTENTS

GSI

GEOTECHNICAL EXPLORATION FOR

KEARNEY PUBLIC SCHOOLS - NEW HIGH SCHOOL
KEARNEY, NEBRASKA

GSI JOB NO. 145025

APRIL 18, 2014

1. INTRODUCTION

1.1 General

This report presents a summary of the findings from our geotechnical exploration for the Kearney Public Schools new high school to be located south of 11th Street and west of 30th Avenue in Kearney, Nebraska. The scope of work was outlined in our proposal dated January 20, 2014. Written authorization was provided by Mr. Chad Nelson, with Kearney Public Schools, on January 30, 2014.

The purpose of this geotechnical study was to explore subsurface conditions at the proposed site, evaluate the engineering properties of the subsurface materials, and provide soils related recommendations for design and construction of the proposed school building and pavement. The scope of our exploration and report is limited to the following:

- Site Geology and Subsurface Conditions
- Groundwater Conditions
- Recommendations for Foundation Type(s)
- Foundation Design Parameters
- Estimate of Anticipated Settlement
- Lateral Earth Pressures for Below-Grade Foundation Walls
- Site Considerations and Earthwork Criteria
- Subgrade Recommendations and Design Parameters for Floor Slabs and Pavements
- Pavement Thickness Design Recommendations
- Seismic Site Classification per 2006 IBC Table 1613.5.2

The scope of services for this exploration did not include a wetlands evaluation, an environmental assessment, or an exploration for the presence of hazardous or toxic materials in the soil, surface water, groundwater, or air within or adjacent to this site. Any statements in this report or on the boring logs regarding odors noted, unusual or suspicious items, or conditions observed are strictly for the information of our client. If contamination is suspected or is a concern, we recommend that the scope of this study be expanded to include an environmental assessment.

The firm of GSI Engineering Northern Division, LLC (GSI) prepared this report. The report was prepared by a professional engineer registered in the State of Nebraska and in accordance with generally accepted soil and foundation engineering practices. This report has been prepared for the exclusive use of the client in accordance with generally accepted geotechnical engineering practices. Recommendations are based on the applicable standards of the profession at the time of this report within this geographic area.

1.2 Project Description

We understand the proposed project involves the construction of an irregular shaped new high school building to be located just south of 11th Street and east of 30th Avenue in Kearney, Nebraska. The structure will have a footprint on the order of approximately 240,000 square feet. We understand the proposed structure will be a combination one-story and two-story with a concrete slab on grade. There will also be a new athletic field building for storage that will be a slab-on-grade structure with a footprint of approximately 6,000 square feet.

We understand the structural support of the roof structure is expected to be provided by a combination of bearing walls and columns. Maximum column and wall loads have been estimated by the structural engineer to be approximately 250 kips and 8 to 16 kips per lineal foot, respectively.

The project will also include new access drives, parking areas, sidewalks, and a football field and track along with other sporting areas.

It is anticipated that there will be approximately three to four feet of fill added to facilitate drainage and level the building pad and approximately 2 feet of fill for the parking lot.

If any of the above information is changed in the final design, the recommendations presented here should be evaluated and modified, if necessary.

2. FIELD EXPLORATION

The geotechnical exploration included a total of ten exploratory borings within the building footprint, extending to depths of 15 to 35 feet and six exploratory borings in the parking and drive areas and the track area, extending to depths of 5 feet. GSI drilled the borings on February 12 and 13, 2014, using a Mobile B-61 and CME-45, truck mounted drilling rigs, advancing 3¼-inch, inside diameter hollow stem augers.

The site plan was provided by Jacob Sertich with Wilkins Hinrichs Stober Architects. GSI personnel established the boring locations in the field by using a rollatape wheel to measure distances from landmarks at the existing site. The location of the borings in relation to existing and proposed features is indicated on the Boring Location Diagram (Appendix A). The location of the borings should be considered accurate only to the degree implied by the methods used.

Our drill crew obtained four soil samples within the upper 10 feet and generally obtained the remaining samples at 5-foot intervals during the field exploration. Split-barrel samples (designated "S-#" sample) were obtained while performing Standard Penetration Tests (SPT) with a 1 3/8-inch I.D., thick-walled sampler, driven in general accordance with ASTM D1586, "*Penetration Test and Split-Barrel Sampling of Soils.*" The "N" value, reported in blows per foot, equals the number of blows required to drive the sampler over the last 12 inches of the sample interval using a 140-pound hammer falling 30 inches. Our drill crew obtained undisturbed samples (designated "U-#" sample) with thin-walled tube samplers, 3-inch outside diameter, hydraulically pushed in general accordance with ASTM D1587, "Thin Walled Tube Sampling of Soils." The recovered samples were sealed in plastic containers, labeled, and protected for transportation to the laboratory for further examination, testing, and classification.

The drill crew prepared the field boring logs during the field exploration. The field logs report drilling and sampling methods, sampling intervals, groundwater measurements, and the encountered subsurface conditions.

3. LABORATORY TESTING

The field boring logs were reviewed to outline the depth, thickness, and extent of the soil strata. The samples taken from the borings were examined in our laboratory and visually classified in general accordance with ASTM D2488, "*Description and Identification of Soils (Visual-Manual Procedure)*." A testing program was established to evaluate the engineering properties of the recovered samples. Specific tests that were performed include:

- Water Content (ASTM D2216, "*Laboratory Determination of Water (Moisture) Content of Soil and Rock*")
- Unit Weight (ASTM D2937, "Density of Soil In-place by the Drive-Cylinder Method")
- Unconfined Compressive Strength (ASTM D2166, "*Unconfined Compressive Strength of Cohesive Soil*")

All tests were conducted in general accordance with current ASTM or state-of-the-practice test procedures. Laboratory test results are presented on the boring logs and in Appendix C.

Water content and density tests were used to evaluate the existing moisture-density state of the soils. Unconfined compression tests were used to define the stress-strain characteristics and related shear strength of the soils.

Based on the results of this testing program, the field logs were reviewed and supplemented as presented in Appendix B. The final logs represent our interpretation of the field logs and reflect the additional information obtained from the laboratory testing. Stratification boundaries indicated on the boring logs were based on observations during drilling, an extrapolation of information obtained by inspecting samples from the borings, and comparisons of similar engineering characteristics. Locations of these boundaries are approximate and the transitions between soil types may be gradual rather than clearly defined.

4. GENERAL SITE CONDITIONS

4.1 Surface Conditions

At the time of the exploration, the terrain was fairly flat and consisted of a cornfield. 30th Avenue was located along the west side of the proposed site and 11th Street is located along the north side of the proposed site. The site consists of a rectangular shaped area and is approximately 82 acres.

4.2 Subsurface Conditions

According to the Soil Survey of Buffalo County, Nebraska, by the Soil Conservation Service (U.S. Department of Agriculture), the surficial site soils are comprised of Hord, Hall, and Cozad association. The Hord soil series is generally comprised of deep, well drained, nearly level to very gently sloping soils in the South Loup River valley and on loess capped alluvial stream terraces of the Platte and Wood River valleys.

Although there was some variability in the encountered subsurface conditions, a general soil profile could be developed. The soils encountered within the depths of exploration generally consisted of lean clay, poorly graded sand, and silt.

We encountered a developed zone in the borings at the surface, extending to depths of approximately 1 to 1½ feet below grade. The material was described as brown to dark brown, moist to very moist, lean clay. The developed zone contained roots and root holes in portions. SPT Blow Counts "N" of 8 indicates a firm consistency.

We encountered alluvial deposits of lean clay in the borings at depths of approximately 1 to 1½ feet below grade, extending to approximately 3 to 4½ feet below grade. The alluvial deposits were described as light olive brown to olive brown, moist to very moist, firm to stiff, lean clay. SPT Blow Counts "N" ranging from 7 to 10 indicates a firm to stiff consistency. Laboratory testing performed on this material indicated water contents ranging from 17 to 27 percent, dry densities of 95 to 110 pounds per cubic foot (pcf), and unconfined compressive strengths of 0.86 to 1.59 tons per square foot (tsf).

We encountered alluvial deposits of poorly graded sand in the borings at depths of approximately 3 to 4½ feet below grade, extending to approximately 5 to 30 feet below grade (full depth of exploration). The alluvial deposits were described as light brown, moist to wet, very loose to very dense, fine to coarse grained with some gravel, poorly graded sand. SPT Blow Counts "N" ranging from 3 to 52 indicates a very loose to very dense relative density. Laboratory testing performed on this material indicated water contents ranging from 3 to 10 percent above the water table.

We encountered alluvial deposits of silt in DH-1 at a depth of 28½ feet below grade extending to a depth of approximately 35 feet below grade (full depth of exploration). The material was described as light olive brown, wet, silt. SPT Blow Counts "N" of 4 to 9 indicates a soft to firm consistency.

4.3 Groundwater Observations

Groundwater observations were made during drilling and after completion of the borings to evaluate groundwater conditions. The groundwater depths are noted on the individual Boring Logs in Appendix B and are summarized in Table A.

TABLE A – ESTIMATED GROUNDWATER ELEVATION IN BORINGS

Boring I.D.	Total Boring Depth (feet)	Water Elevation at the End of Drilling (feet)
DH-1	35	7½
DH-2	30	7
DH-3	30	7
DH-4	30	7½
DH-5	30	7
DH-6	5	None Encountered
DH-7	5	None Encountered
DH-8	5	None Encountered
DH-9	15	7

Boring I.D.	Total Boring Depth (feet)	Water Elevation at the End of Drilling (feet)
DH-10	15	7
DH-11	15	7
DH-12	15	7
DH-13	5	None Encountered
DH-14	5	None Encountered
DH-15	15	7
DH-16	5	None Encountered

The groundwater conditions documented during our exploration program should not be construed to represent an absolute or permanent condition. Uncertainty is involved with short-term water level observations in boreholes. The groundwater level and the amount and level of any perched water on the site may be expected to fluctuate with variations in precipitation, site grading, drainage, adjacent land use, and will vary with the water levels in the adjacent streams and rivers.

Long-term monitoring in piezometers or observation wells would be required to evaluate the potential range of groundwater conditions. It should be noted that water table information taken at the time of the field explorations can be truly representative of the project site only if the soil is relatively pervious (consisting of sands and gravels). Several days may be required for the water level to reach an equilibrium point in clays and other fine-grained soils. Allowances should be made for the seasonal variation in the water table.

5. CONCLUSIONS AND RECOMMENDATIONS

5.1 General

In general, this site is considered suitable for the proposed construction. Groundwater was encountered at a depth of approximately 7 to 7½ feet below the existing grade.

5.2 General Earthwork

5.2.1 Site Preparation

In preparing the site for construction, surface vegetation and topsoil containing a significant percentage of organic matter should be removed from the areas beneath structures and any other areas that are to be paved, cut, or receive fill. GSI recommends a minimum removal depth of 18 inches for surface vegetation. Site preparation should also include relocation or abandonment of existing buried utilities. Abandoned underground utility lines should either be completely removed or capped and grouted full.

The developed zone material to be stripped is characterized as brown and dark brown, lean clay as indicated on the boring logs (Appendix B). This material should either be removed from the site or stockpiled for later use in unpaved non-structural areas. After removal of the surface materials, the subgrade in fill areas should be proofrolled with a heavy roller or loaded tandem axle dump truck. Any soft or unsuitable areas should be corrected using the procedures outlined in Sections 5.2.2 and 5.9.

5.2.2 Overexcavation and Structural Backfill (General)

If weak or otherwise unsuitable soils are encountered in the bottom of shallow foundation excavations or subgrade areas, implementation of an overexcavation and structural backfill

procedure may be necessary. This procedure should completely remove all existing unsuitable soils below foundation bearing level to the depth of overexcavation determined by the geotechnical engineer. The overexcavation should extend laterally 12 inches in all directions for each foot of overexcavated depth and be replaced with new structural backfill.

We recommend new structural backfill soils be free of rubble and organics, and have a Unified Soil Classification of CL, CL-ML, or ML. Our experience with similar projects indicates recycled concrete and crushed rock base aggregate work well for replacement of weak or saturated soils. Structural backfill soils should meet the minimum placement requirements outlined in Section 5.9.

5.3 Foundations

Based on the subsurface conditions revealed by the boring and testing program, this site appears suitable for use of a shallow spread foundation system if supported on the engineered structural fill. The selection of an allowable soil bearing pressure for shallow foundation elements must fulfill two requirements. First, the load must be sufficiently less than the ultimate bearing capacity of the foundation to insure stability. Second, the differential settlement must not exceed an amount which will produce adverse behavior of the superstructure.

In order to meet the previous criteria, we have explored both the bearing capacity and the load settlement characteristics of the site soils using estimated wall loads of 8 to 16 kips per lineal foot and a maximum column load of 250 kips. The bearing capacity is based on a factor of safety of 3.0 against the full dead load plus normal live load. A maximum total settlement of 1 inch or less and differential settlement of 1/2 inch for every 25 lineal feet or less are generally considered acceptable and were used in our analysis.

Continuous and individual footings bearing on the engineered structural fill should be sized for a maximum net soil bearing pressure of 2,000 pounds per square foot (psf), based on dead load plus 1/2 live load. The allowable bearing pressure is expressed in terms of the net pressure transferred to the soil.

Individual footings should be at least 3 feet square and continuous footings should be at least 16 inches wide. Lightly loaded interior partition walls (less than 750 pounds per lineal foot) can be supported directly on the floor slab. In no case, however, should footings be smaller than local code sizes. Exterior footings and footings in unheated areas should be founded at a minimum depth of 3½ feet below surrounding grade to provide frost protection. Interior footings, which will be protected from the effects of frost, should be founded at least 2 feet below finished floor elevation. Local building codes can supersede these frost depths. In addition, all footings should be reinforced with steel reinforcement.

Variations in the supporting soils and the building loads are expected to cause some differential movement of the building foundations. Continuous footing/foundation wall combinations should be designed to function as grade beams. Reinforcement should provide the capacity to span at least 10 feet when acting as a continuous beam under foundation loads. Frequent joints should be provided in masonry walls to accommodate some differential settlement and to reduce cracking of the walls. Construction joints should be provided between bearing walls supported on footings and partition walls supported on slabs.

We recommend that concrete should be placed as soon as practical after footing excavation, with as little disturbance to the bearing soils as possible. All footing excavations should be free of loose soil or debris. If water collects in the excavations, it should be promptly removed to prevent

softening of the foundation supporting soils prior to concrete placement. In addition, we recommend all excavations be observed by qualified geotechnical personnel before placement of concrete for the possible presence of unsuitable bearing soils at the base of an excavation.

We estimate total settlement of 1 inch or less and differential settlement of 1/2 inch or less for every 25 lineal feet for footings designed and constructed as recommended above. These estimates do not include settlement due to inadequate final preparation of the bearing surfaces.

5.4 Lateral Earth Pressure

Below grade wall foundations may be subject to lateral earth pressures. Estimated lateral earth pressures for granular soils are presented in Table B below. Natural or fill soils lateral earth pressure parameters may be used, respectively, where natural or fill soils surround the foundation element.

If the top of the foundation wall is restrained, higher lateral earth pressures will develop against the wall. Increased pressures can also develop from restricted soil drainage or surcharge loads adjacent to the wall.

TABLE B. ESTIMATED LATERAL EARTH (EQUIVALENT FLUID) PRESSURES [(a)]

	NATURAL COHESIVE SOILS	COHESIVE FILL
Approximate Wet Density	120 pcf	125 pcf
Approximate Friction Angle	18°	26°
Active Pressure Coeff.: K_a	0.53	0.39
At-Rest Pressure Coeff.: K_o	0.69	0.56
Passive Pressure Coeff.: K_p	1.89	2.56

(a) Excluding Cohesion Shear Strength Sliding Friction Effects

5.5 Seismic Site Classification

Building code requirements may include design for seismic forces associated with earthquake motions. The project site is classified as Site Class D according to Table 1613.5.2 in the 2006 version of the International Building Code.

5.6 Floor Slabs

The subgrade should be reworked to a depth of 12 inches and compacted as recommended in Section 5.9 immediately prior to concrete placement. Any soft or unsuitable areas should be corrected using the procedures outlined in Sections 5.2.2 and 5.9. The floor slab should be independent of any and all structural members and components. The system must be designed as a floor slab that is free to move without damaging the remainder of the building.

We recommend the floor slab, bearing on a well-prepared compacted subgrade as described above and in Section 5.9, be designed using a modulus of subgrade reaction (k-value) of 100 pounds per cubic inch (pci).

The flooring manufacturer may recommend placing a granular capillary moisture barrier or granular leveling course beneath the floor slab. If used, the granular capillary barrier should conform to the flooring manufacturer's requirements. The flooring manufacturer may also recommend placing a

vapor retarder beneath grade-supported slabs that will be covered with wood, tile, carpet, or other moisture-sensitive or impervious coverings. The type and location of the vapor retarder installed in areas where floor coverings are planned, as well as the thickness of any granular capillary moisture barrier, should conform to the requirements of the flooring manufacturer. Deviating from the manufacturer's requirements may void any warranty for the flooring used.

5.7 Pavement Subgrade Preparation

Pavements associated with the proposed new construction include sidewalks, parking lots, and access drives. Grading plans have not been completed at this time; however, we expect that minimum grading to establish the design pavement subgrade elevations will require only nominal amounts of cutting and filling. Following the minimum grading, soils at subgrade level for the pavements are expected to include a combination of new and existing fill, and natural clay soils at various locations. In order to provide satisfactory pavement performance, the subgrade must be relatively uniform with no abrupt changes in material type and degree of support. With the exception of properly selected and compacted new fill, the anticipated subgrade materials are not considered suitable for direct support of the pavement. Our recommendations for preparation of pavement subgrade are presented in the following paragraphs.

All surface vegetation and organic topsoil should be removed from within and at least 2 feet beyond the pavement limits. We recommend additional excavation of the existing soils, as required, to a minimum depth of 2 feet below the design subgrade elevation of medium- and heavy-duty vehicular pavements and to a minimum depth of 1 foot below the design subgrade elevation of light-duty vehicular pavements. Prior to the placement of new fill, the exposed subgrade should be proof-rolled with a loaded tandem-axle dump truck (or other equipment providing equivalent subgrade loading) in the presence of a geotechnical engineer from GSI or his designated representative. Proof-rolling aids in delineating shallow zones of weak subgrade, which may require additional compaction, removal, or stabilization. Where practical, soft or unstable subgrade material should be completely removed. In areas where soft or unstable materials are thicker than approximately 2 feet, we recommend stabilizing the subgrade with a layer of geogrid (Tensar BX1100 or equal) and at least 12 inches of crushed limestone or recycled concrete. All fill placed beneath pavements should be cohesive soils compacted to the requirements of structural fill as discussed in Section 5.9. A granular base should not be placed beneath the pavement unless it is properly drained.

The subgrade for sidewalks should be prepared by scarifying the soil to a depth of at least 6 inches below the bottom of the sidewalk, adjusting the water content to the range recommended for structural fill, and compacting the subgrade soil to structural fill requirements.

Surface drainage around the pavement is important to long-term performance. Curbs should be backfilled as soon as possible after construction of the pavement. Backfill should be compacted and sloped to prevent water from ponding behind curbs and infiltrating under the pavement. All pavement joints should be caulked and any cracks should be promptly sealed to prevent moisture intrusion into the subgrade.

We estimate that a California Bearing Ratio (CBR) of 4 and a subgrade modulus of 100 pci could be used for design of vehicular pavements if the above recommendations for pavement subgrade preparation are met. Adequate site drainage should be provided to reduce future wetting of the pavement subgrade and the potential for frost heave.

5.8 Design of Pavement Type and Thickness

In our opinion, both flexible and rigid type pavements are feasible at the project site. Pavement design is influenced by anticipated traffic loads and volume, site subgrade condition, pavement material, and target design life. The pavement thickness design is derived by comparing equivalent pavement sections. These sections are based on recognized structural coefficients using locally available materials.

We have not been provided with the anticipated traffic loading for the parking lot and drive areas. Therefore, we have assumed three loading conditions for the proposed pavement areas. The parking areas are assumed to be light-duty areas, with passenger vehicles and small trucks anticipated. The drive areas are assumed to be medium-duty areas, with some large heavy trucks (delivery trucks) anticipated. Areas where heavy stationary loads or large heavy trucks are anticipated, such as dumpster areas, are considered heavy-duty areas.

We recommend that a pavement section of at least 6 inches of Asphaltic Cement Concrete (ACC) or at least 5 inches of Portland Cement Concrete (PCC) pavement be placed in the light-duty areas. We recommend that a pavement section of at least 7 inches of ACC or at least 6 inches of PCC be placed in the medium-duty areas. We recommend that a pavement section of at least 7 inches of PCC be placed in the heavy-duty areas. Both ACC and PCC should be placed in accordance with the most current Nebraska Department of Roads Specifications or governing local requirements.

The owner should be aware that the reliability of ACC pavement is dependent on the workmanship of the pavement contractor. Poor workmanship leading to segregation of the aggregate, improper binder content, or improper compaction can drastically reduce the design life of ACC pavement.

Pavement thicknesses may be reduced and section capacity can be increased if the pavement subgrade includes a properly drained granular base. This can also result in an increase in the design life of the pavement and a reduction in maintenance costs. A single layer of non-woven biaxial geogrid (Tensar BX-1100 or approved equal) placed below the granular base can further increase these benefits. A 6-inch layer of granular fill with a minimum of 12 percent fines (passing #200 sieve) placed on a layer of Tensar BX-1100 geogrid (or approved equal) may reduce pavement thicknesses in some areas by as much as 1 inch and nearly double the section capacity. Please contact GSI if there is a desire to further explore the benefits of a geogrid-reinforced granular base.

5.9 Earthwork

In areas to accept new fill, the top 8 to 12 inches of the ground surface should be scarified and compacted to eliminate a plane of weakness along the contact surface. All import material should be lean clay. Cohesive soils should have a liquid limit less than 40, plasticity index less than 20, and contain less than 1.5 percent organic material. Fill should be placed in loose lifts of 6 to 8 inches maximum thickness and compacted to meet minimum requirements in Table C.

Water content of fill at the time of compaction should be controlled between optimum and +3 percentage points of optimum water content as determined by the standard Proctor test (ASTM D698). We recommend a technician working under the supervision of a geotechnical engineer from our firm periodically monitor earthwork operations to evaluate compliance with the recommendations in this section.

TABLE C - RECOMMENDED GUIDELINES FOR DEGREE-OF-COMPACTION

Construction Application	Standard Proctor (ASTM D698) Cohesive Soil	Standard Proctor (ASTM D698) Cohesionless Soil	*Relative Density D4253 & D4254 Cohesionless Soil
Building Foundation, Roadway Subgrades, Pavement, Sidewalks, and Critical Backfill Areas	98%	98%	85%
Backfill Adjacent to Structures Not Supporting Other Structures – Minor Subsidence Possible	95%	95%	70%

*Use Relative Density technique (ASTM D4253 & D4254) where standard Proctor technique (ASTM D698) does not result in a definable maximum dry density and optimum water content.

5.10 Excavation Slopes

Vertical cuts and excavations may stand for short periods of time, but should not be considered stable in any case. All excavations should be sloped back, shored, or shielded for protection of workers. Trenching and excavation activities should conform to federal and local regulations as a minimum. The soils encountered in the test borings generally classify as a type "B" soils according to OSHA's Construction Standards for Excavations. In general, the maximum allowable slope for shallow excavations of less than 20 feet in a type "B" soil is 1H:1V, although other provisions and restrictions may apply. If different soil types are encountered, the maximum allowable slope may be different.

The Contractor is responsible for designing any excavation slopes or temporary shoring. The Contractor should be aware that slope height, slope inclination, and excavation depths (including utility trench excavations) should, in no case, exceed those specified in federal, state, or local safety regulations, such as OSHA Health and Safety Standards for Excavations, 29 CFR Part 1926, or successor regulations.

5.11 Construction Observation

Site grading, including proofrolling, replacement or recompaction of material, and placement of fill and backfill, should be observed by a qualified technician from GSI under the direction of a registered professional engineer. The technician should perform density tests and make any other observations necessary to assure that the requirements of the specifications are being achieved. GSI requires that observation of construction by the geotechnical engineer of record or his designated representative to complete the design process. Field observation services are viewed as essential and a continuation of the design process. Unless these services are provided, the geotechnical engineer will not be responsible for improper use of recommendations, or failure by others to recognize conditions which may be detrimental to the successful completion of the project. We recommend that a preconstruction meeting be planned prior to the site preparation work to discuss our recommendations, project requirements, and the contractor's plan of action.

5.12 Frost Protection

The on-site clays and any imported clays used as structural fill are frost-susceptible materials. Frost action is a factor in design of slabs and foundations exposed to exterior temperatures and supported on clay soils at this site. Seasonal movements of surface slabs exposed to freezing temperatures should be expected.

Structural stoops should be provided at all exterior entrances to the building. The stoop slabs should be structurally supported over void spaces. Foundations around the exterior perimeter of heated structures should be embedded at least 3½ feet below final exterior grades. Exterior footings and footings in unheated areas should be founded at a minimum depth of 3½ feet below surrounding grade to provide frost protection. Interior footings, which will be protected from the effects of frost, should be founded at least 2 feet below finished floor elevation.

PCC pavements and sidewalks should be jointed to reduce cracking due to frost-induced movements and should be detailed to reduce the effects of movement relative to the building and structural stoops.

5.13 Surface Drainage and Landscaping

The success of the shallow foundation system and slab-on-grade floor system is contingent upon keeping the subgrade soils at a fairly constant moisture content and by not allowing surface drainage to have a path to the subsurface. Positive surface drainage away from structures must be maintained at all times. Landscaped areas should be designed and built such that irrigation and other surface water will be collected and carried away from foundation elements.

During construction, temporary grades should be established to prevent runoff from entering excavations or footing trenches. Backfill should be placed as soon as concrete structural strength requirements are met and should be graded to drain away from the structures.

The final grade of the foundation backfill and any overlying pavements should have a positive slope away from foundation walls on all sides. A minimum slope of 1 inch per foot for the first 5 to 10 feet is recommended for uncovered surfaces. A minimum slope of 2 percent is recommended for other areas of the site. Irrigation within 10 feet of the foundation should be carefully controlled and minimized.

5.14 Construction Considerations

If construction of the project is to be performed during winter, steps should be taken to prevent the soils under floor slabs, footings, or pavements from freezing. **In no case** should the floor slab, foundations, pavements, or other exterior flat work be placed on frozen or partially frozen materials. Frozen materials should be removed and replaced with a suitable material as described in earlier sections of this report.

6. CLOSING REMARKS

6.1 Limitations

This report is presented in broad terms to provide an assessment of the subsurface conditions and their potential effect on the adequate design and economical construction of the proposed structure and pavement. The analyses, conclusions, and recommendations contained in this report are based on the site conditions existing at the time of the exploration, the project layout described herein, and the assumption that the information obtained from our sixteen borings is representative of subsurface conditions throughout the site. Any changes in the design or location of the proposed structures should be assumed to invalidate the conclusions and recommendations given in this report until we have had the opportunity to review the changes and, if necessary, modify our conclusions and recommendations accordingly. If subsurface conditions different from those encountered in the exploration are observed during construction or appear to be present beneath excavations, GSI should be advised at once so that the conditions can be reviewed and recommendations reconsidered where necessary. If there is a substantial

lapse in time between the submission of this report and the start of construction, or if site conditions or the project layout have significantly changed (due to further development of grading plans, natural causes, or construction operations at or adjacent to the site), we recommend that this report be reviewed to determine the applicability of our previous conclusions and recommendations.

Our geotechnical exploration and subsequent recommendations address only the design and construction considerations contained in this report. Other details commonly found in structures such as vapor barriers, or slab and foundation wall insulation are not normally within the scope of a geotechnical investigation, but should be given consideration by the designer.

We make no warranty for the contents of this report, neither expressed nor implied, except that our professional services were performed in accordance with engineering principles and practices generally accepted at this time and location.

6.2 Additional Services

We require that GSI be provided the opportunity for a general review of the final design plans and specifications. This is to ensure that earthwork and foundation recommendations have been properly interpreted in the design and specifications. GSI will not be responsible for misrepresentation of this report resulting from partial reproduction or paraphrasing of its contents.

We also require that GSI be retained to provide continuous engineering services during construction of the foundation, excavation, and earthwork phases of the work. This is to observe compliance with the design concepts, specifications, and recommendations and to modify recommendations in the event that subsurface conditions differ from those anticipated. Please review the ASFE document *"Important Information About Your Geotechnical Engineering Report"* located ahead of the Table of Contents for additional information regarding this report.

Respectfully submitted,
Geotechnical Services, Inc.

Prepared by,

James D. Sorgenfrei, P.E.
Senior Engineer

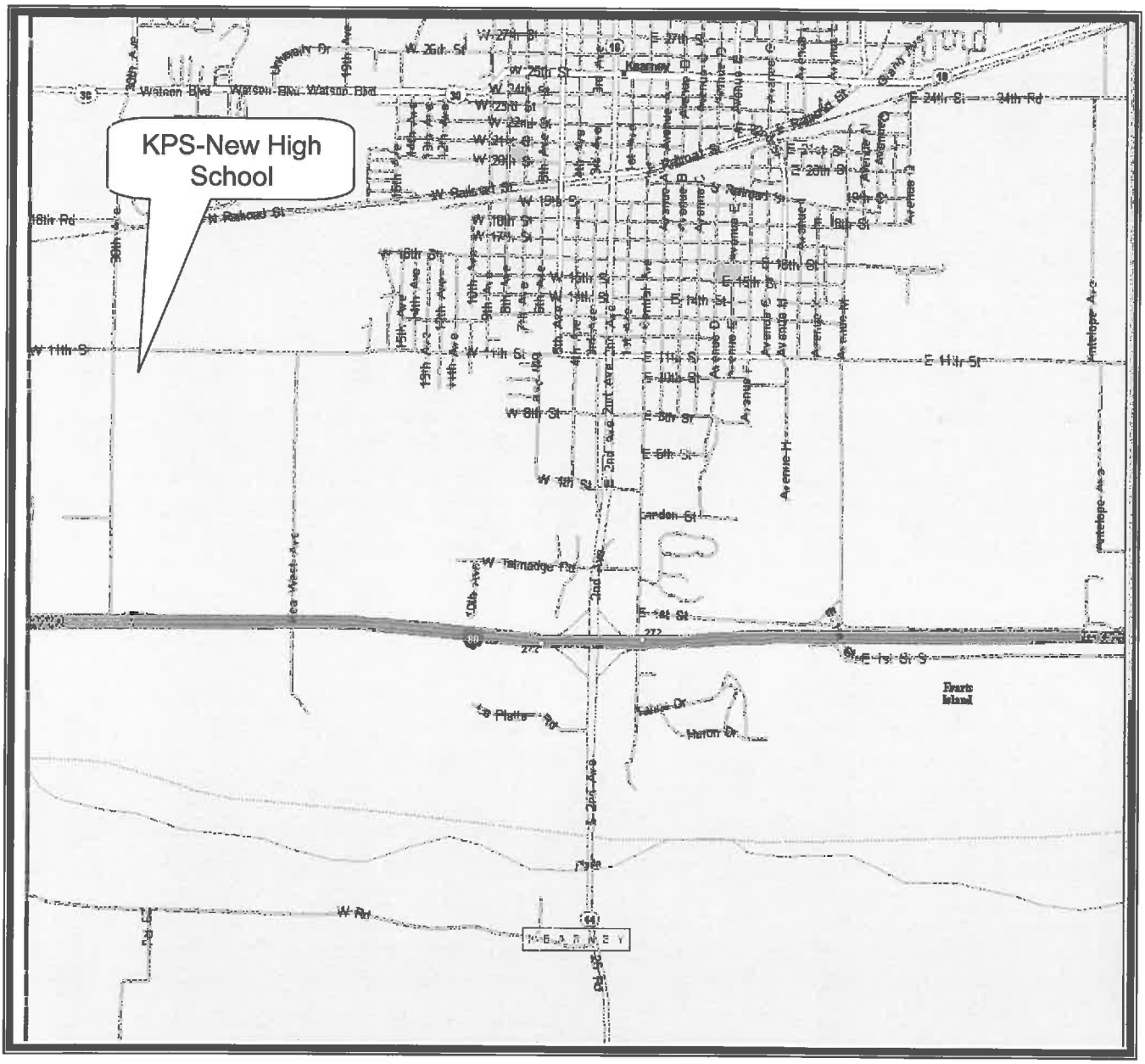
Reviewed by,

Clifford G. Plato, P.E.
Project Engineer

Legend

Not to Scale

N



General Vicinity Map

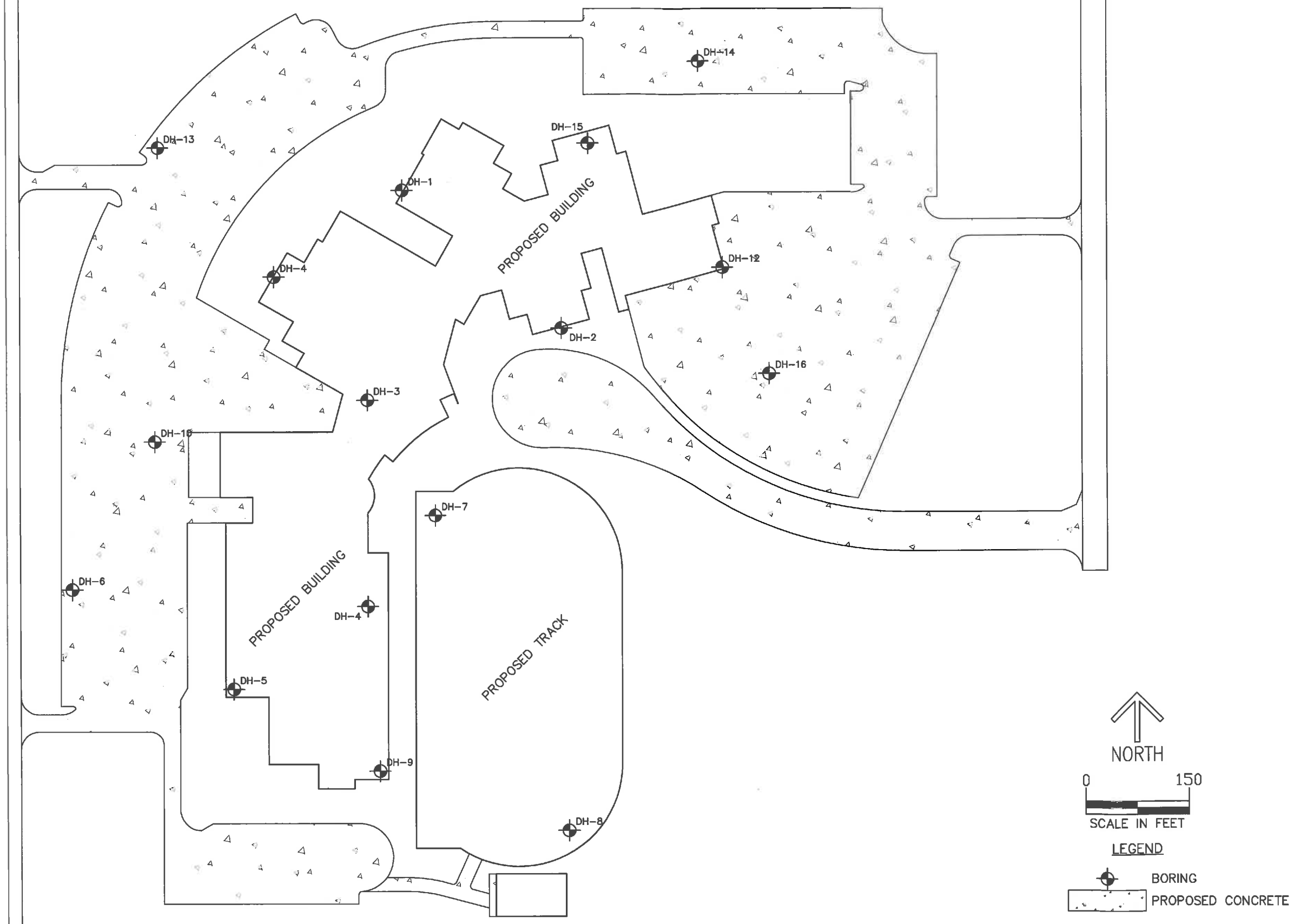
Map obtained from Microsoft Streets & Trips

GSI Engineering

A division of Alt & Witzig Engineering

2960 N. Diers Avenue * Grand Island, Nebraska 68803 * 308.381.1987

Project	KPS-New High School
Location	Kearney, Nebraska
Job No.	145025
Date	4/17/14



REVISED/REPRINTED	
DATE	BY

GSI
2960 North Diers Avenue
Grand Island, Nebraska
Telephone: (308) 381-1987
Fax: (308) 381-2467

BORING LOCATION DIAGRAM

KEARNEY PUBLIC NEW HIGH SCHOOL
WEST 11TH STREET AND 30TH AVENUE
KEARNEY, NEBRASKA

FILE NAME	145025 BORING LOCATION
DRAWN	LAD
CHECKED	JS
ISSUE:	1

BORING LOG No. DH-1

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-1	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
6 1/2 Feet	7 1/2 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	35 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	"N" BLOWS (FT)	% REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	DRY DENS. pcf	Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Moist, Lean Clay	CL				
				1.0'					
	S-1	10		ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay with Rust and Some Fine Sand	CL	28.2			
				Light Olive Brown Below a Depth of 3 1/ 2 Feet					
5	U-2			4.5'		18.4	105.6	0.86	5
				ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust					
	S-3	14		Wet Below a Depth of 6 1/2 Feet					
10	S-4	10		Medium to Coarse Grained with Some Gravel Below a Depth of 8 1/2 Feet					10
15	S-5	3		Very Loose and Coarse Grained with Gravel Below a Depth of 13 1/2 Feet	SP				15
20	S-6	14		Medium Dense and Medium to Coarse Grained Below a Depth of 18 1/2 Feet					20
25	S-7	20							25
				28.5'					
30	S-8	4		ALLUVIAL DEPOSITS Light Olive Brown, Wet, Soft, Silt	ML				30
	S-9	4							
35	S-10	9		Firm Below a Depth of 33 1/2 Feet					35
				35.0'					
				Bottom of Boring @ 35'					

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/12/14

BORING LOG No. DH-2

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-2	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
7 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	30 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Moist, Lean Clay — 1.0'	CL				
	U-1			ALLUVIAL DEPOSITS Olive Brown, Moist, Stiff, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 Feet — 3.0'	CL	16.6	109.5	1.89	
5	S-2	14		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust		4.3			5
	S-3	14		Wet Below a Depth of 7 Feet					
10	S-4	7		Loose and Medium to Coarse Grained Below a Depth of 8 1/2 Feet					10
15	S-5	13		Medium Dense and Medium to Coarse Grained Below a Depth of 13 1/2 Feet	SP				15
20	S-6	11							20
25	S-7	33		Dense and Fine to Coarse Grained Below a Depth of 23 1/2 Feet					25
30	S-8	20		Medium Dense and Medium to Coarse Grained Below a Depth of 28 1/2 Feet — 30.0'					30
				Bottom of Boring @ 30'					
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/12/14

BORING LOG No. DH-3

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-3	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
6 1/2 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	30 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	"N" BLOWS (FT)	% REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	DRY DENS. pcf	Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Moist, Lean Clay	CL				
				1.0'					
	S-1	7		ALLUVIAL DEPOSITS Olive Brown, Moist, Firm, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 Feet	CL	20.1			
				4.0'					
5	S-2	21		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust		10.4			5
	S-3	7		Loose and Medium to Coarse Grained Below a Depth of 6 Feet Wet Below a Depth of 6 1/2 Feet					
10	S-4	8							10
15	S-5	12		Medium Dense and Medium to Coarse Grained with Some Gravel Below a Depth of 13 1/2 Feet	SP				15
20	S-6	12							20
25	S-7	17		Medium to Coarse Grained with Gravel Below a Depth of 23 1/2 Feet					25
30	S-8	23		Medium to Fine Grained Below a Depth of 28 1/2 Feet					30
				30.0'					
				Bottom of Boring @ 30'					
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-4

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-4	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
8 Feet	7 1/2 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	30 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	"N" BLOWS (FT)	% REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	DRY DENS. pcf	Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Moist, Lean Clay	CL				
				1.0'					
	U-1			ALLUVIAL DEPOSITS Olive Brown, Moist, Stiff, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 Feet	CL	21.4	102.1	1.59	
				4.0'					
5	S-2	17		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust					5
	S-3	16							
				Wet Below a Depth of 8 Feet Loose and Medium to Coarse Grained Below a Depth of 8 1/2 Feet					
10	S-4	8							10
15	S-5	8							15
					SP				
				Medium Dense and Medium to Fine Grained Below a Depth of 18 1/2 Feet					
20	S-6	15							20
				Medium to Coarse Grained with Some Gravel Below a Depth of 23 1/2 Feet					
25	S-7	16							25
				Very Dense and Medium to Fine Grained Below a Depth of 28 1/2 Feet					
30	S-8	52							30
				30.0'					
				Bottom of Boring @ 30'					
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-5

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-5	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
7 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	30 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	"N" BLOWS (FT)	% REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	DRY DENS. pcf	Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Moist, Lean Clay	CL				
				1.0'					
	S-1	9		ALLUVIAL DEPOSITS Olive Brown, Moist, Firm, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 1 1/ 2 Feet	CL	19.6			
				4.0'					
	S-2	14		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust		5.9			
5									5
	S-3	14		Wet Below a Depth of 7 Feet					
	S-4	8		Loose and Medium to Coarse Grained with Some Gravel Below a Depth of 8 1/2 Feet					
10									10
	S-5	11		Medium Dense and Medium to Coarse Grained Below a Depth of 13 1/2 Feet					
15									15
	S-6	14			SP				
20									20
	S-7	31		Dense Below a Depth of 23 1/2 Feet					
25									25
	S-8	32		Medium Grained Below a Depth of 28 1/2 Feet					
30									30
				30.0'					
				Bottom of Boring @ 30'					
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-6

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-6	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay	CL				
	U-1			1.0' ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	23.4	98.0	1.14	
5	S-2	10		4.0' ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust 5.0' Bottom of Boring @ 5'	SP				5
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-7

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-7	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay — 1.0'	CL				
	U-1			ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	23.4	96.2	1.27	
5	S-2	10		ALLUVIAL DEPOSITS — 4.5' Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust — 5.0' Bottom of Boring @ 5'	SP				5
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-8

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-8	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay — 1.0'	CL				
	S-1	9		ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	25.3			
				— 3.5'					
5	S-2	20		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust — 5.0'	SP	2.9			5
				Bottom of Boring @ 5'					
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-9

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-9	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
7 1/2 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	15 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Dark Brown, Moist, Lean Clay — 1.0'	CL				
	U-1			ALLUVIAL DEPOSITS Olive Brown, Moist, Stiff, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 1/ 2 Feet — 3.5'	CL	20.3	103.4	1.35	
5	S-2	15		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust		5.0			5
	S-3	12		Wet Below a Depth of 7 1/2 Feet					▽ ▽
10	S-4	9		Loose and Medium to Coarse Grained with Some Gravel Below a Depth of 8 1/2 Feet	SP				10
15	S-5	9		Bottom of Boring @ 15' — 15.0'					15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-10

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-10	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
7 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	15 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	"N" BLOWS (FT)	% REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	DRY DENS. pcf	Qu tsf	DEP. FT.
				DEVELOPED ZONE Dark Brown, Moist, Lean Clay	CL				
	S-1	10		1.5' ALLUVIAL DEPOSITS Olive Brown, Moist, Stiff, Lean Clay with Rust and Some Fine Sand		20.5			
				Light Olive Brown Below a Depth of 3 1/ 2 Feet	CL				
	S-2	10		4.5' ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust Loose and Medium to Coarse Grained with Some Gravel Below a Depth of 6 Feet Wet Below a Depth of 7 Feet		10.3			
5									5
	S-3	7							
	S-4	7							
10					SP				10
	S-5	8							
15				15.0' Bottom of Boring @ 15'					15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-11

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-11	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
7 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	15 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Dark Brown, Very Moist, Lean Clay	CL				
	U-1			1.5' ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 1/ 2 Feet	CL	23.1	99.9	1.09	
5	S-2	10		4.5' ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust Loose and Medium to Coarse Grained with Some Gravel Below a Depth of 6 Feet Wet Below a Depth of 7 Feet		13.9			5
	S-3	8							
10	S-4	9			SP				10
15	S-5	10		Medium Dense Below a Depth of 13 1/2 Feet 15.0' Bottom of Boring @ 15'					15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-12

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-12	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
7 1/2 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	15 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Dark Brown, Very Moist, Lean Clay	CL				
	S-1	9		1.0' ALLUVIAL DEPOSITS Olive Brown, Very Moist, Stiff, Lean Clay with Rust and Some Fine Sand Light Olive Brown Below a Depth of 2 1/ 2 Feet	CL	27.1			
				4.0'					
5	S-2	12		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust		8.9			5
	S-3	9		Loose and Medium to Coarse Grained with Some Gravel Below a Depth of 6 Feet Wet Below a Depth of 7 1/2 Feet					▽ ▽
10	S-4	11		Medium Dense Below a Depth of 8 1/2 Feet	SP				10
15	S-5	6		Loose and Medium Grained Below a Depth of 13 1/2 Feet					15
				15.0' Bottom of Boring @ 15'					
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-13

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-13	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay 1.0'	CL				
	S-1	10		ALLUVIAL DEPOSITS Olive Brown, Very Moist, Stiff, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	24.0			
				3.5'					
5	S-2	16		ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust 5.0'	SP	13.9			5
				Bottom of Boring @ 5'					
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-14

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-14	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	Mobile B-61
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay 1.0'	CL				
	U-1			ALLUVIAL DEPOSITS Olive Brown, Very Moist, Stiff, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	27.0	94.7	1.73	
5	S-2	18		4.0' ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust 5.0'	SP	12.4			5
				Bottom of Boring @ 5'					
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-15

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-15	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
7 Feet	7 Feet			3 1/4 Inch Inside Diameter Hollow Stem Augers	15 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Dark Brown, Very Moist, Lean Clay — 1.0'	CL				
	U-1			ALLUVIAL DEPOSITS Olive Brown, Very Moist, Stiff, Lean Clay with Rust and Some Fine Sand		28.0	94.9	1.45	
				Light Olive Brown Below a Depth of 3 Feet	CL				
5	S-2	13		— 4.5'		13.0			5
				ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust					
	S-3	14		Wet Below a Depth of 7 Feet					▽ ▽
10	S-4	12			SP				10
				Loose and Medium Grained Below a Depth of 13 1/2 Feet					
15	S-5	6		Loose and Medium to Coarse Grained Below a Depth of 13 1/2 Feet — 15.0'					15
				Bottom of Boring @ 15'					
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

BORING LOG No. DH-16

BORING NO.	LOCATION OF BORING	ELEVATION	DATUM	DRILLER	LOGGER
DH-16	See Boring Location Diagram			James Tinnell	James Sorgenfrei

WATER LEVEL OBSERVATIONS				TYPE OF SURFACE	DRILL RIG
WHILE DRILLING	END OF DRILLING	24 HOURS AFTER DRILLING		Corn Stuble	CME-45
				DRILLING METHOD	TOTAL DEPTH
None	None			3 1/4 Inch Inside Diameter Hollow Stem Augers	5 Feet

DEP. FT.	SAMPLE DATA: SAMPLE NO. & TYPE	SAMPLE DATA: "N" BLOWS (FT)	SAMPLE DATA: % REC.	SOIL DESCRIPTION: COLOR, MOISTURE, CONSISTENCY / GEOLOGIC DESCRIPTION & OTHER REMARKS	USCS CLASS.	LABORATORY DATA: % MC	LABORATORY DATA: DRY DENS. pcf	LABORATORY DATA: Qu tsf	DEP. FT.
				DEVELOPED ZONE Brown, Very Moist, Lean Clay	CL				
				1.0'					
	U-1			ALLUVIAL DEPOSITS Olive Brown, Very Moist, Firm, Lean Clay Light Olive Brown Below a Depth of 2 Feet	CL	25.3	97.0	0.90	
	S-2	6				21.2			
5				4.5'					5
				ALLUVIAL DEPOSITS Light Brown, Moist, Medium Dense, Medium to Fine Grained, Poorly Graded Sand with Rust	SP				
				5.0'					
				Bottom of Boring @ 5'					
10									10
15									15
20									20
25									25
30									30
35									35

GSI Engineering
A division of Alt & Witzig Engineering

PROJECT: Kearney Public Schools-New High School
LOCATION: Kearney, Nebraska
JOB NO.: 145025
DATE: 2/13/14

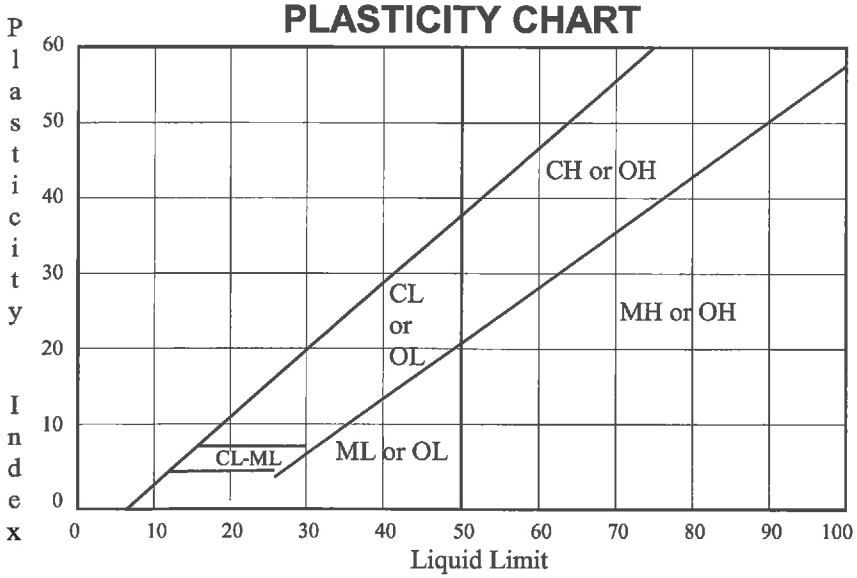
UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)

GROUP NAME	GROUP SYMBOL	SOIL DESCRIPTION	Comments
Peat	Pt	Highly organic soils	50% or more is smaller than No. 200 sieve
Fat Clay	CH	Clay - Liquid limit > 50% *	
Plastic Silt	MH	Silt - Liquid limit > 50% *	
Lean Clay	CL	Clay - Liquid limit < 50% *	
Silt	ML	Silt - Liquid limit < 50% *	
Silty Clay	CL-ML	Silty Clay *	
Clayey Sand	SC	Sands with 12 to 50 percent smaller than No. 200 sieve *	More than 50% is larger than No. 200 sieve and % sand > % gravel
Silty Sand	SM		
Poorly Graded Sand with Clay	SP-SC	Sands with 5 to 12 percent smaller than No. 200 sieve *	
Poorly Graded Sand with Silt	SP-SM		
Well Graded Sand with Clay **	SW-SC		
Well Graded Sand with Silt **	SW-SM		
Poorly Graded Sand	SP	Sands with less than 5 percent smaller than No. 200 sieve *	
Well Graded Sand **	SW		
Clayey Gravel	GC	Gravels with 12 to 50 percent smaller than No. 200 sieve *	More than 50% is larger than No. 200 sieve and % gravel > % sand
Silty Gravel	GM		
Poorly Graded Gravel with Clay	GP-GC	Gravels with 5 to 12 percent smaller than No. 200 sieve *	
Poorly Graded Gravel with Silt	GP-GM		
Well Graded Gravel with Clay **	GW-GC		
Well Graded Gravel with Silt **	GW-GM		
Poorly Graded Gravel	GP	Gravels with less than 5 percent smaller than No. 200 sieve *	
Well Graded Gravel **	GW		

* See Plasticity Chart for definition of silts and clays.

** See definition for well graded.

PLASTICITY CHART



LEGEND OF TERMS

SAMPLE IDENTIFICATION

U - Undisturbed (Shelby tube)
S – Split-barrel/SPT (disturbed)
C - California sampler
L - Lasky continuous sampler
A - Auger cuttings (sack sample)
B - Bulk sample (auger cuttings)
H - Head-space sample

CONSISTENCY OF COHESIVE SOILS

Unconfined Compressive Strength, Qu, psf

<500	Very Soft
500-1000	Soft
1000-2000	Medium Stiff (Firm)
2000-4000	Stiff
4000-8000	Very Stiff
>8000	Hard

RELATIVE DENSITY OF GRANULAR SOILS

N - Blows per foot

0-3	Very Loose
4-9	Loose
10-29	Medium Dense
30-49	Dense
50-80	Very Dense

CLASSIFICATION CRITERIA FOR SANDS AND GRAVELS

Well graded sands (SW) $C_u = D_{60}/D_{10} \geq 6$ and $C_c = (D_{30})^2/(D_{10} \times D_{60}) \leq 3$ and ≥ 1

Well graded gravels (GW) $C_u = D_{60}/D_{10} \geq 4$ and $C_c = (D_{30})^2/(D_{10} \times D_{60}) \leq 3$ and ≥ 1

Boulders	Cobbles	Coarse Gravel	Fine Gravel	Coarse Sand	Medium Sand	Fine Sand	FINES (silt or clay)
Sieve sizes	12"	3"	3/4"	#4	#10	#40	#200

GSI Engineering
A division of Alt & Witzig Engineering

Drill Hole No.	Sample No.	Sample Depth (ft.)	Sample Dia. (in.)	Sample Length (in.)	Water Content (%)	Density		Void Ratio	Sat (%)	Unconfined Comp		Atterberg Limits			Cons Test *	% Passing #200	Classification
						Wet (pcf)	Dry (pcf)			QU (tsf)	Strain (%)	LL	PL	PI			
DH-1	S-1	1 - 2½			28.2												
	U-2	3½ - 5	2.85	5.45	18.4	125.0	105.6	0.584	84	0.86	3.1						
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
	S-6	18½ - 20															
	S-7	23½ - 25															
	S-8	28½ - 30															
	S-9	30 – 31½															
	S-10	38½ - 40															
DH-2	U-1	1 - 2½	2.85	5.59	16.6	127.7	109.5	0.527	85	1.89	5.4						
	S-2	3½ - 5			4.3												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
	S-6	18½ - 20															
	S-7	23½ - 25															
	S-8	28½ - 30															
DH-3	S-1	1 - 2½			20.1												
	S-2	3½ - 5			10.4												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
	S-6	18½ - 20															
	S-7	23½ - 25															
	S-8	28½ - 30															

GSI Engineering
A division of Alt & Witzig Engineering
2960 N. Diers Avenue * Grand Island, NE 68803 * Phone: 308.381.1987

SUMMARY OF SOIL TESTS

Project
Kearney Public New High School

Location
Kearney, Nebraska

Job Number 145025

Date 4/17/14

Drill Hole No	Sample No	Sample Depth (ft)	Sample Dia (in.)	Sample Length (in.)	Water Content (%)	Density		Void Ratio	Sat (%)	Unconfined Comp		Atterberg Limits			Cons Test	% Passing #200	Classification
						Wet (pcf)	Dry (pcf)			QU (tsf)	Strain (%)	LL	PL	PI			
DH-4	U-1	1 - 2½	2.85	5.59	21.4	124.0	102.1	0.638	90	1.59	4.5						
	S-2	3½ - 5			8.4												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
	S-6	18½ - 20															
	S-7	23½ - 25															
	S-8	28½ - 30															
DH-5	S-1	1 - 2½			19.6												
	S-2	3½ - 5			5.9												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
	S-6	18½ - 20															
	S-7	23½ - 25															
	S-8	28½ - 30															
DH-6	U-1	1 - 2½	2.85	5.59	23.4	120.9	98.0	0.706	89	1.14	3.6						
	S-2	3½ - 5			6.8												
DH-7	U-1	1 - 2½	2.85	5.59	23.4	118.7	96.2	0.739	85	1.27	5.4						
	S-2	3½ - 5			21.5												
DH-8	S-1	1 - 2½			25.3												
	S-2	3½ - 5			2.9												

GSI Engineering
A division of Alt & Witz'g Engineering
2960 N. Diers Avenue * Grand Island, NE 68803 * Phone: 308.381.1987

SUMMARY OF SOIL TESTS

Project
Kearney Public New High School

Location
Kearney, Nebraska

Job Number 145025

Date 4/17/14

Drill Hole No.	Sample No.	Sample Depth (ft.)	Sample Dia. (in.)	Sample Length (in.)	Water Content (%)	Density		Void Ratio	Sat. (%)	Unconfined Comp		Atterberg Limits			Cons. Test *	% Passing #200	Classification
						Wet (pcf)	Dry (pcf)			QU (tsf)	Strain (%)	LL	PL	PI			
DH-9	U-1	1 - 2½	2.85	5.59	20.3	124.5	103.4	0.617	88	1.35	5.4						
	S-2	3½ - 5			5.0												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
DH-10	S-1	1 - 2½			20.5												
	S-2	3½ - 5			10.3												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
DH-11	U-1	1 - 2½	2.85	5.59	23.1	122.9	99.9	0.674	92	1.09	4.0						
	S-2	3½ - 5			13.9												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
DH-12	S-1	1 - 2½			27.1												
	S-2	3½ - 5			8.9												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
DH-13	S-1	1 - 2½			24.0												
	S-2	3½ - 5			13.9												

GSI Engineering
A division of Alt & Witzig Engineering
2960 N. Diers Avenue * Grand Island, NE 68803 * Phone: 308.381.1987

SUMMARY OF SOIL TESTS

Project: Kearney Public New High School

Location: Kearney, Nebraska

Job Number: 145025

Date: 4/17/14

Drill Hole No.	Sample No.	Sample Depth (ft.)	Sample Dia. (in.)	Sample Length (in.)	Water Content (%)	Density		Void Ratio	Sat. (%)	Unconfined Comp.		Atterberg Limits			Cons. Test	% Passing #200	Classification
						Wet (pcf)	Dry (pcf)			QU (tsf)	Strain (%)	LL	PL	PI			
DH-14	U-1	1 - 2½	2.85	5.59	27.0	120.2	94.7	0.766	94	1.73	5.4						
	S-2	3½ - 5			12.4												
DH-15	U-1	1 - 2½	2.85	5.59	28.0	121.5	94.9	0.762	98	1.45	5.4						
	S-2	3½ - 5			13.0												
	S-3	6 - 7½															
	S-4	8½ - 10															
	S-5	13½ - 15															
DH-16	U-1	1 - 2½	2.85	4.80	25.3	121.4	97.0	0.725	93	0.90	5.4						
	S-2	3½ - 5			21.2												

GSI Engineering
A division of Alt & Witzig Engineering
2960 N. Diers Avenue * Grand Island, NE 68803 * Phone: 308.381.1987

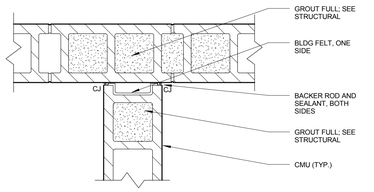
SUMMARY OF SOIL TESTS

Project
Kearney Public New High School

Location
Kearney, Nebraska

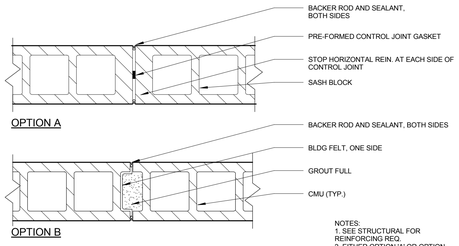
Job Number 145025

Date 4/17/14



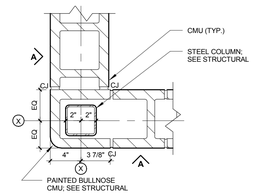
NOTES:
1. SEE STRUCTURAL FOR REINFORCING REQ.

8 A1.01 PLAN DETAIL - TYP. CMU INTERSECTION
SCALE: 1 1/2" = 1'-0"

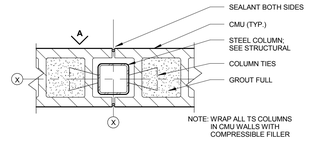


NOTES:
1. SEE STRUCTURAL FOR REINFORCING REQ.
2. EITHER OPTION 'A' OR OPTION 'B' MAY BE USED AT INTERIOR CONTROL JOINT LOCATIONS.

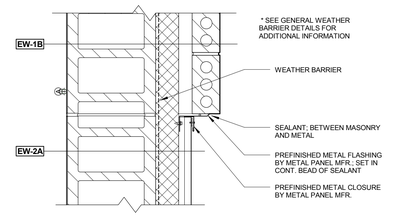
7 A1.01 PLAN DETAIL - TYP. CMU CONTROL JOINT (CJ)
SCALE: 1 1/2" = 1'-0"



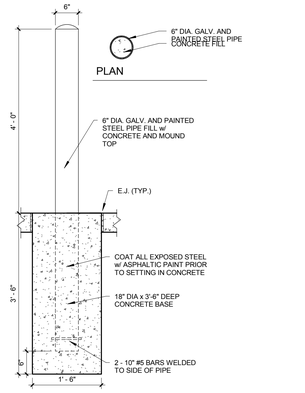
6 A1.01 PLAN DETAIL - TYP. EMBEDDED CORNER COLUMN (4")
SCALE: 1 1/2" = 1'-0"



5 A1.01 PLAN DETAIL - TYP. EMBEDDED COLUMN (4")
SCALE: 1 1/2" = 1'-0"

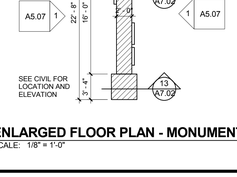


4 A1.01 PLAN DETAIL - TYP. BRICK / MET. PANEL CONNECTION
SCALE: 1 1/2" = 1'-0"

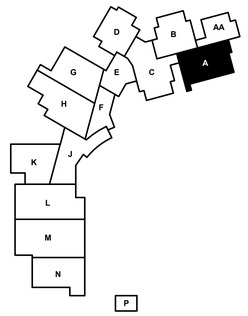


3 A1.01 TYP. BOLLARD SECTION AND PLAN DETAIL
SCALE: 3/4" = 1'-0"

1 A1.01 FLOOR PLAN - FIRST FLOOR - AREA A
SCALE: 1/8" = 1'-0"



2 A1.01 ENLARGED FLOOR PLAN - MONUMENT SIGN
SCALE: 1/8" = 1'-0"



Wilkins Hinrichs Stober Architects L.L.C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

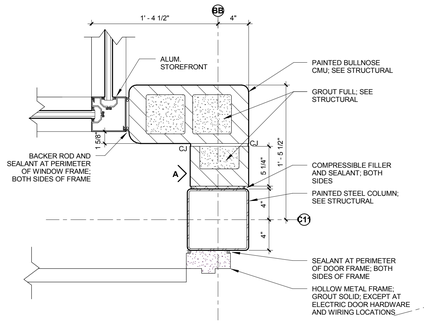
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

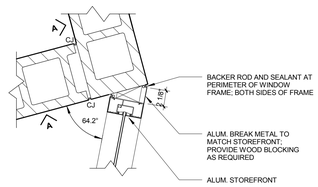
Floor Plan - First Floor - Area A

ProjectNumber: **1355**
Date: **September 2, 2014**

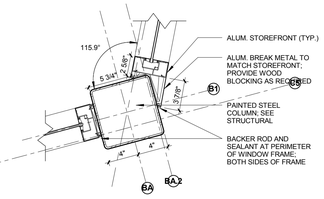
Sheet Number:
A1.01



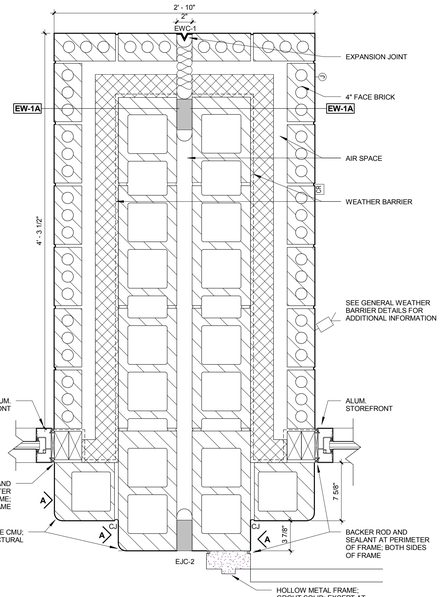
1 A1.02 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



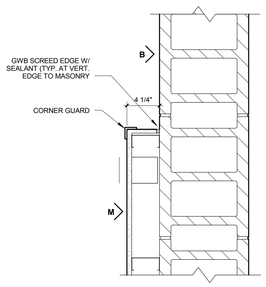
6 A1.02 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



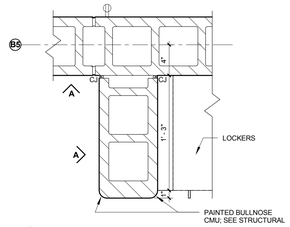
7 A1.02 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



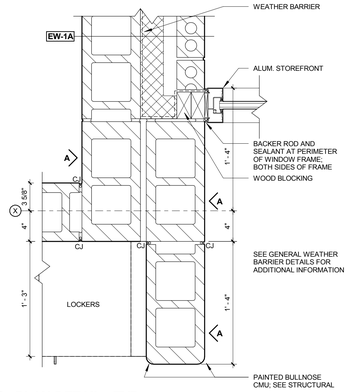
5 A1.02 **PLAN DETAIL - TYP. PILASTER**
SCALE: 1 1/2" = 1'-0"



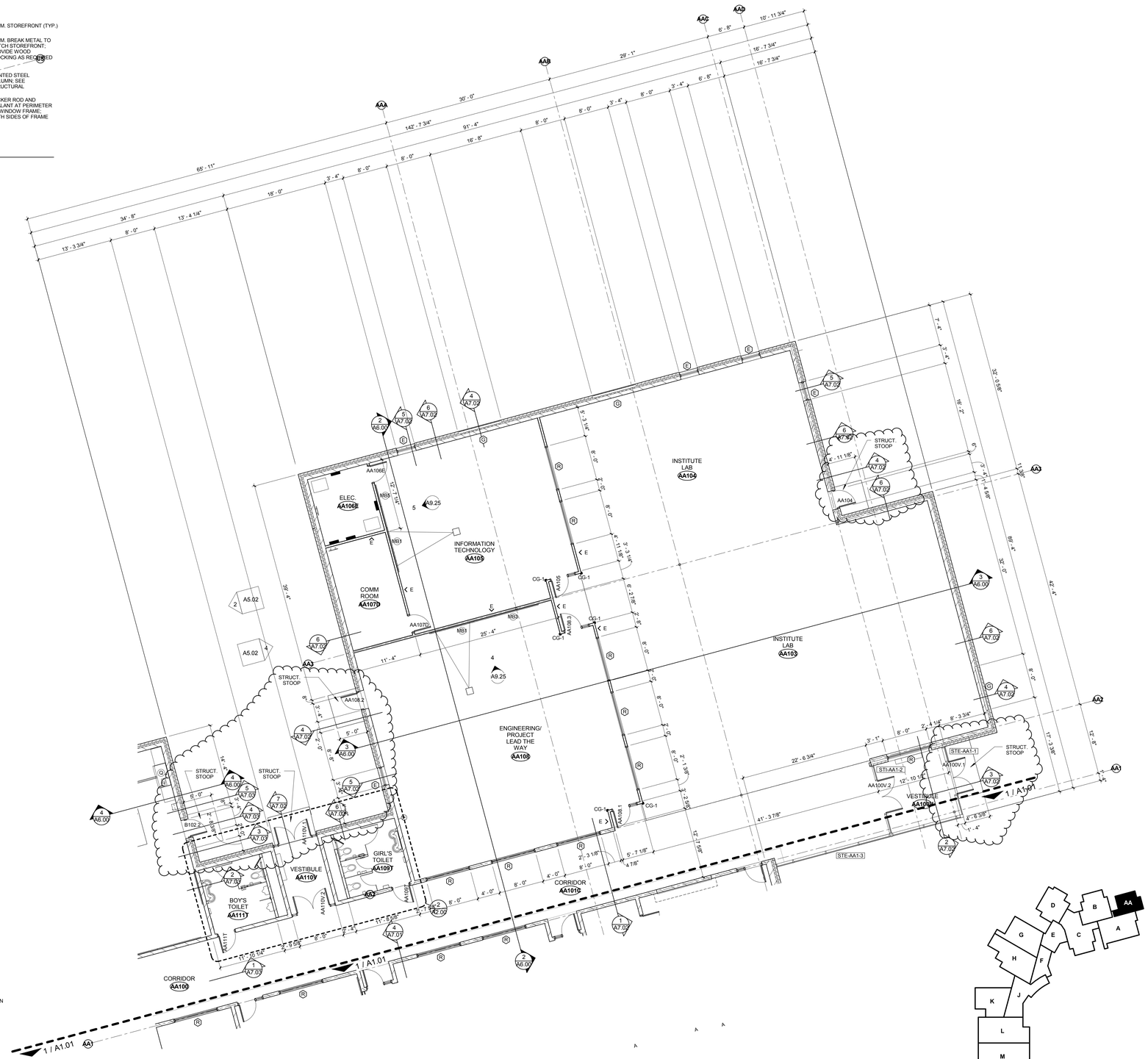
1 A1.02 **PLAN DETAIL - TYP. 3 5/8" METAL FURRING**
SCALE: 1 1/2" = 1'-0"



3 A1.02 **PLAN DETAIL - TYP.**
SCALE: 1 1/2" = 1'-0"



2 A1.02 **PLAN DETAIL - TYP.**
SCALE: 1 1/2" = 1'-0"



N 1 A1.02 **FLOOR PLAN - FIRST FLOOR - AREA B**
SCALE: 1/8" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

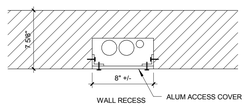
Revision/Issue	Date
Addendum No. 1	09/12/2014

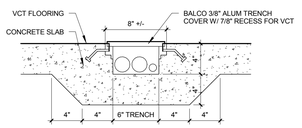
Floor Plan - First Floor - Area AA

ProjectNumber: 1355
Date: September 2, 2014

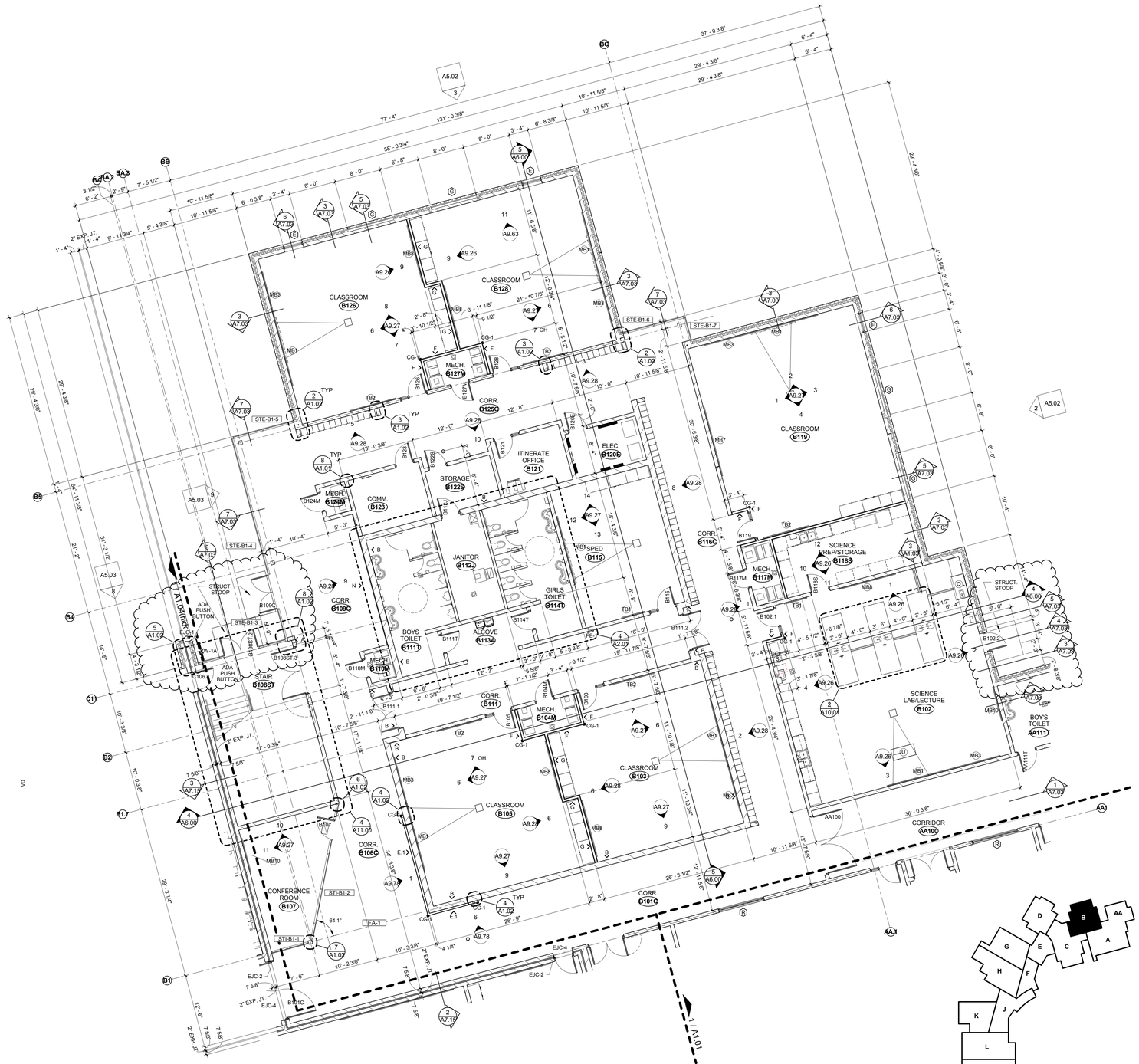
Sheet Number:
A1.02

3 A1.03 **GAS TRENCH SECTION**
SCALE: 1/2" = 1'-0"





2 A1.03 **GAS TRENCH COVER**
SCALE: 1 1/2" = 1'-0"



N 1 A1.03 **FLOOR PLAN - FIRST FLOOR - AREA B**
SCALE: 1/8" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area B

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:
A1.03

Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area C

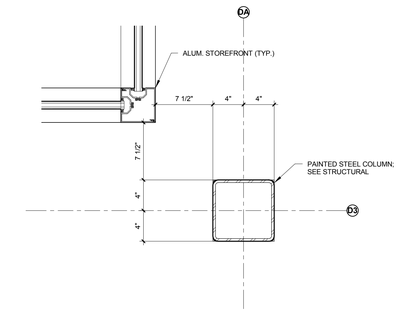
ProjectNumber: 1355
Date: September 2, 2014

Sheet Number:

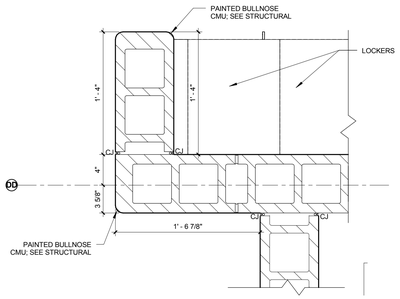
A1.04(hidden)

FLOOR PLAN - FIRST FLOOR - AREA C

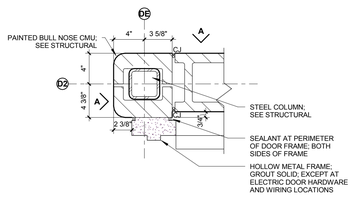
SCALE: 1/8" = 1'-0"



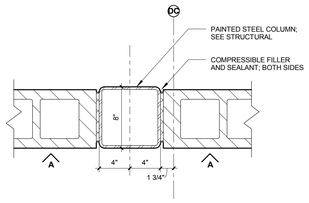
1 A1.06 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



4 A1.06 **PLAN DETAIL (TYP.)**
SCALE: 1 1/2" = 1'-0"



3 A1.06 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



2 A1.06 **PLAN DETAIL - TYP. CMU / EMBEDDED COLUMN (8")**
SCALE: 1 1/2" = 1'-0"

N 1 A1.06 **FLOOR PLAN - FIRST FLOOR - AREA D**
SCALE: 1/8" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

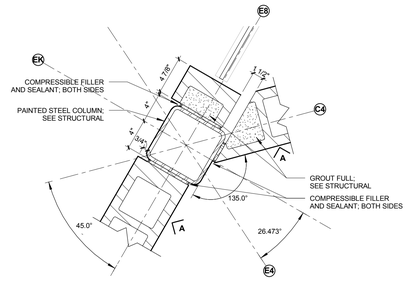
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

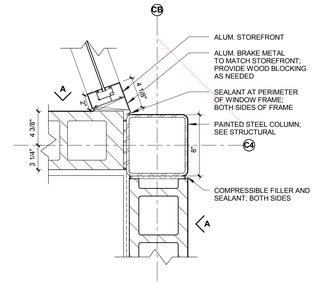
Floor Plan - First Floor - Area D

ProjectNumber: **1355**
Date: **September 2, 2014**

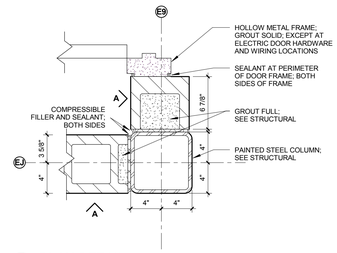
Sheet Number:
A1.06



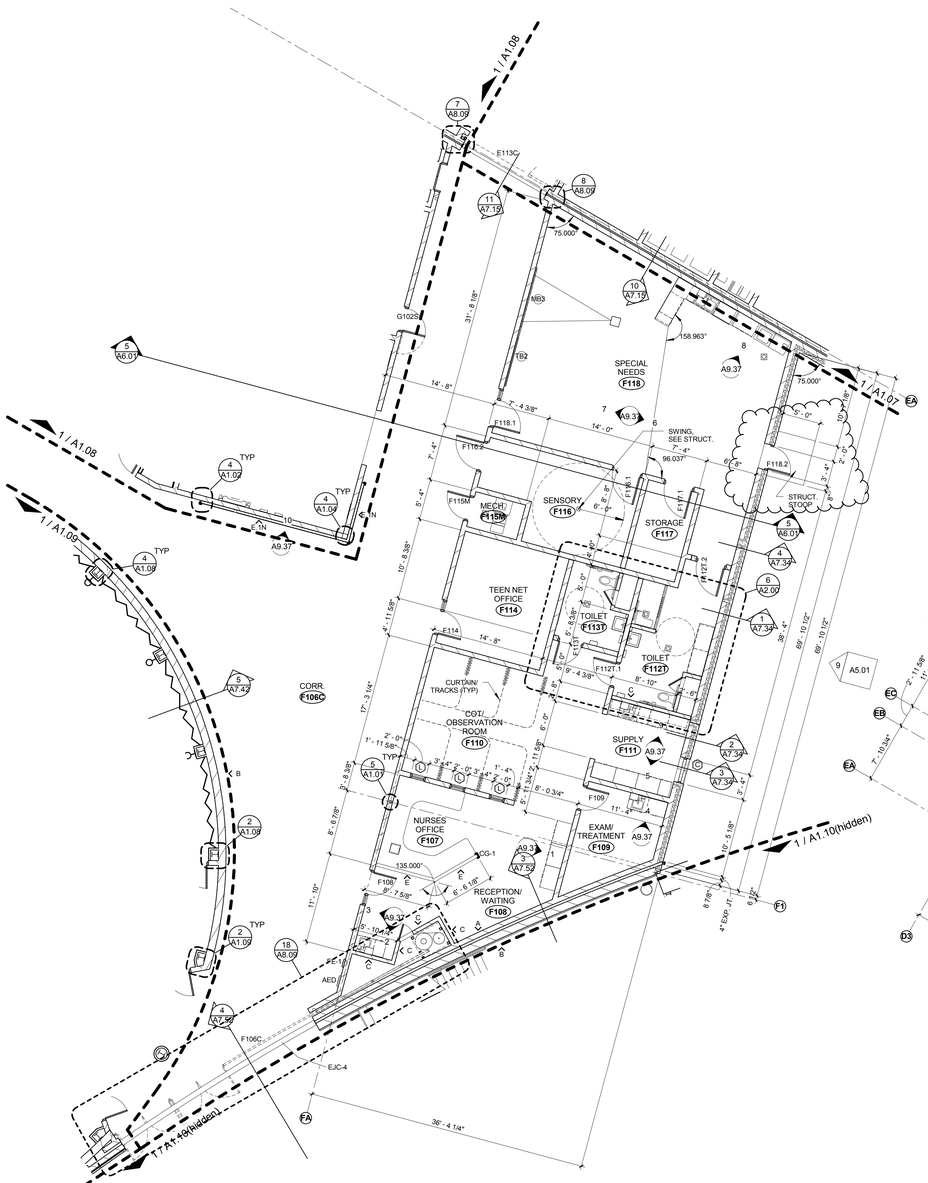
5 A1.07 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



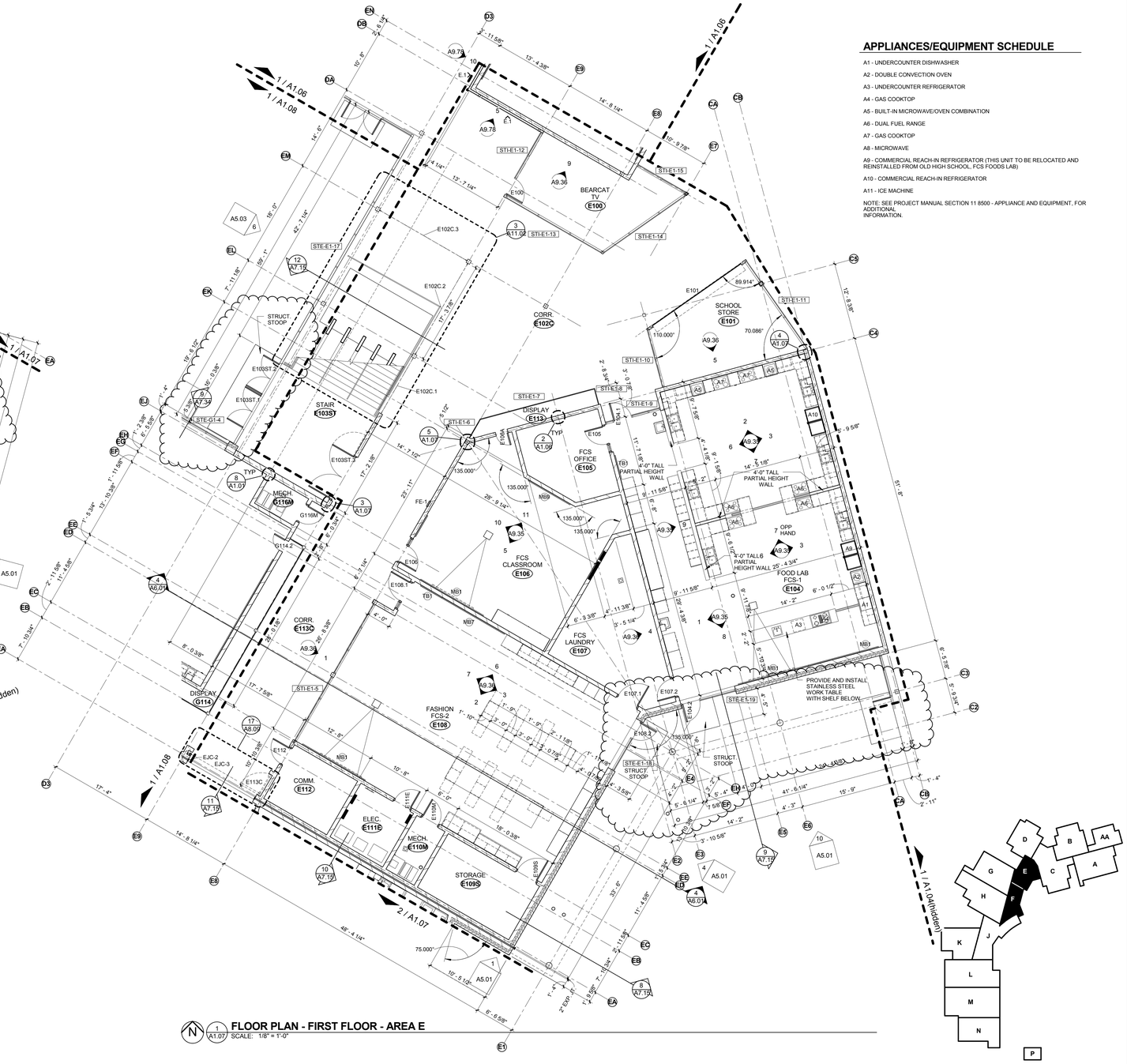
4 A1.07 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



3 A1.07 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



N 2 A1.07 **FLOOR PLAN - FIRST FLOOR - AREA F**
SCALE: 1/8" = 1'-0"



N 1 A1.07 **FLOOR PLAN - FIRST FLOOR - AREA E**
SCALE: 1/8" = 1'-0"

APPLIANCES/EQUIPMENT SCHEDULE

A1 - UNDERCOUNTER DISHWASHER

A2 - DOUBLE CONVECTION OVEN

A3 - UNDERCOUNTER REFRIGERATOR

A4 - GAS COOKTOP

A5 - BUILT-IN MICROWAVE/OVEN COMBINATION

A6 - DUAL FUEL RANGE

A7 - GAS COOKTOP

A8 - MICROWAVE

A9 - COMMERCIAL REACH-IN REFRIGERATOR (THIS UNIT TO BE RELOCATED AND REINSTALLED FROM OLD HIGH SCHOOL, FCS FOODS LAB)

A10 - COMMERCIAL REACH-IN REFRIGERATOR

A11 - ICE MACHINE

NOTE: SEE PROJECT MANUAL SECTION 11 8500 - APPLIANCE AND EQUIPMENT, FOR ADDITIONAL INFORMATION.

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

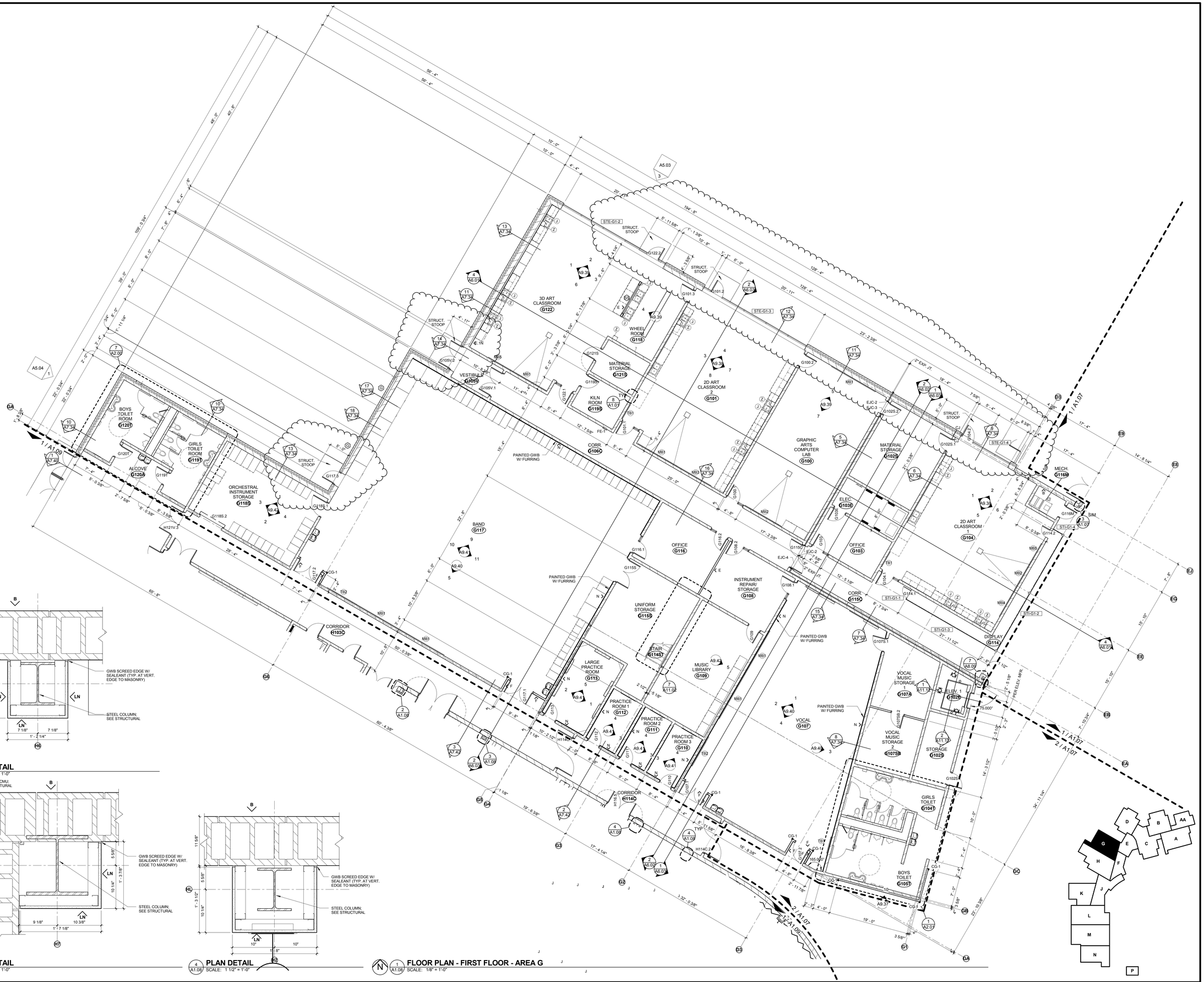
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area E & F

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:
A1.07



3 / A1.08 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"

2 / A1.08 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"

4 / A1.08 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"

N 1 / A1.08 **FLOOR PLAN - FIRST FLOOR - AREA G**
SCALE: 1/8" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

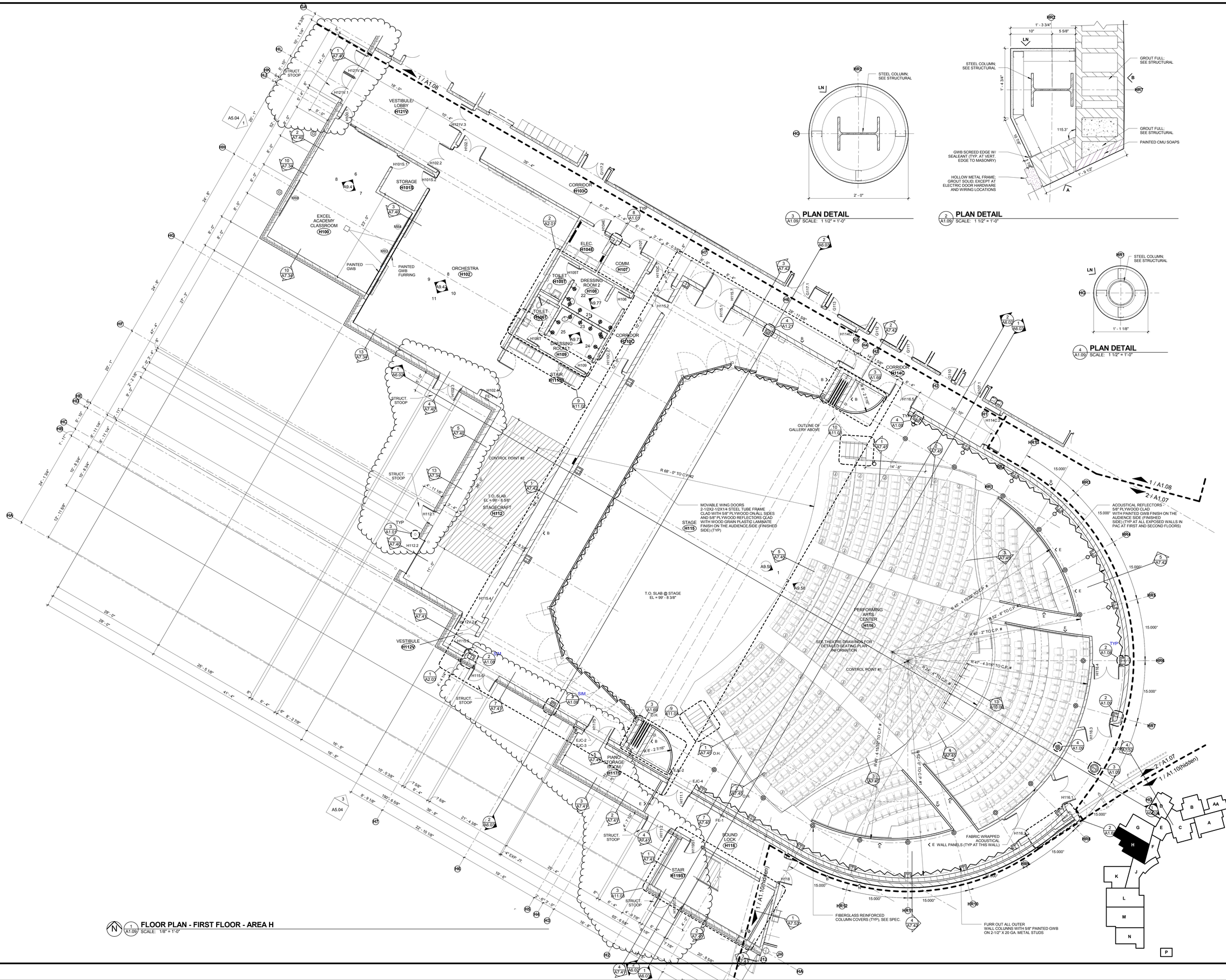
Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area G

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

A1.08



FLOOR PLAN - FIRST FLOOR - AREA H

1 / A1.09 SCALE: 1/8" = 1'-0"

PLAN DETAIL

3 / A1.09 SCALE: 1 1/2" = 1'-0"

PLAN DETAIL

2 / A1.09 SCALE: 1 1/2" = 1'-0"

PLAN DETAIL

4 / A1.09 SCALE: 1 1/2" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area H

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

A1.09

Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

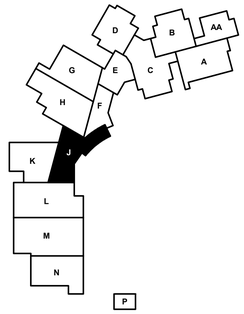
Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area J

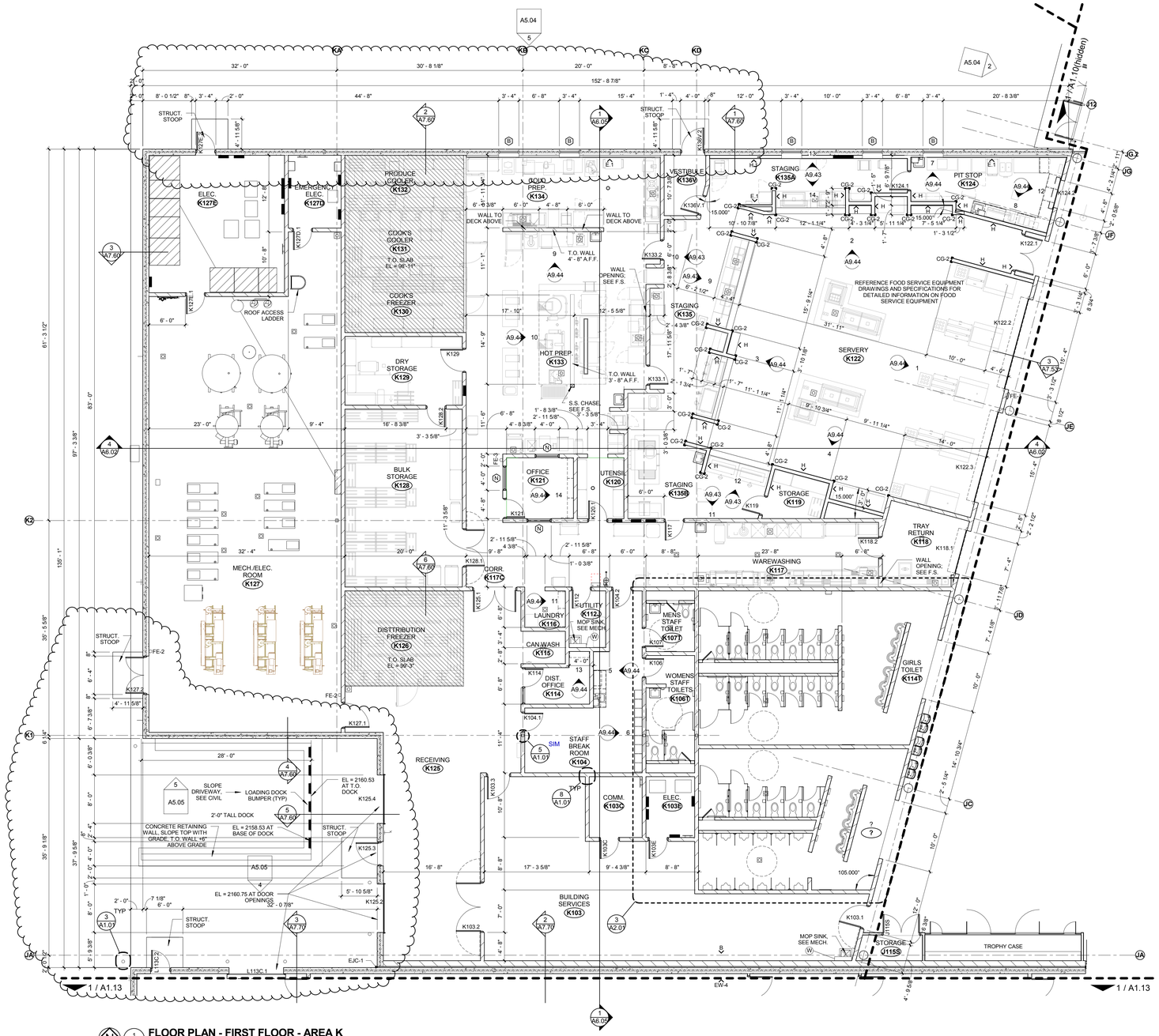
ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

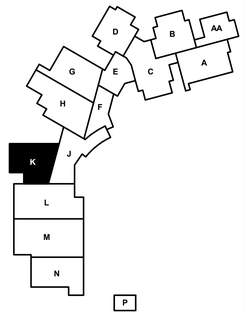
A1.10(hid



FLOOR PLAN - FIRST FLOOR - AREA J
SCALE: 1/8" = 1'-0"



FLOOR PLAN - FIRST FLOOR - AREA K
SCALE: 1/8" = 1'-0"



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

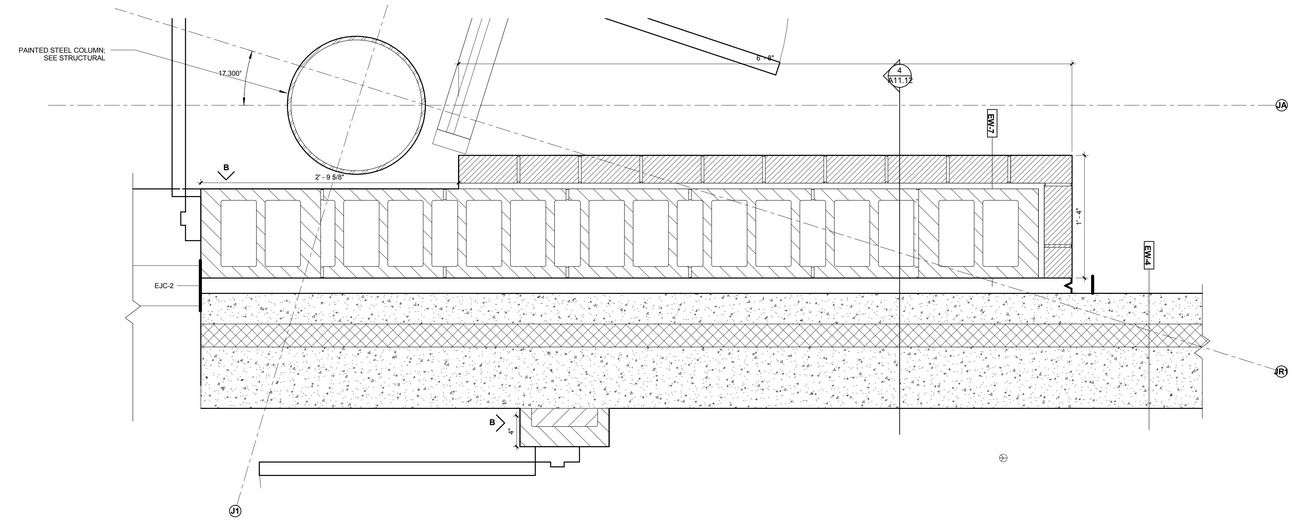
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

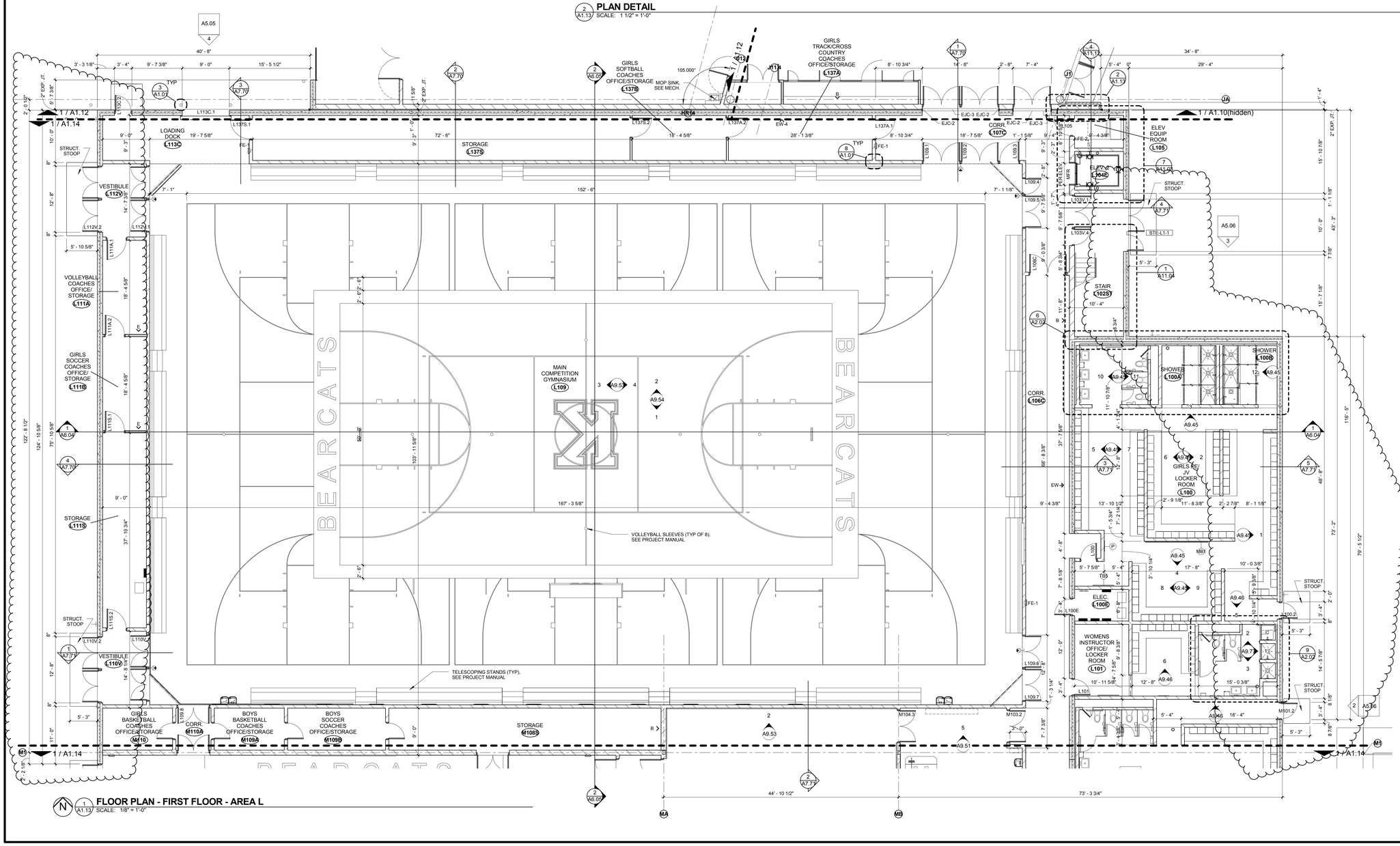
Floor Plan - First Floor - Area K

ProjectNumber: 1355
Date: September 2, 2014

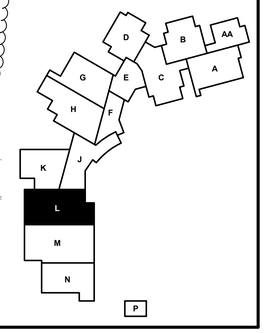
Sheet Number:
A1.12



2 / A1.13 PLAN DETAIL
SCALE: 1 1/2" = 1'-0"



N 1 / A1.13 FLOOR PLAN - FIRST FLOOR - AREA L
SCALE: 1/8" = 1'-0"



Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

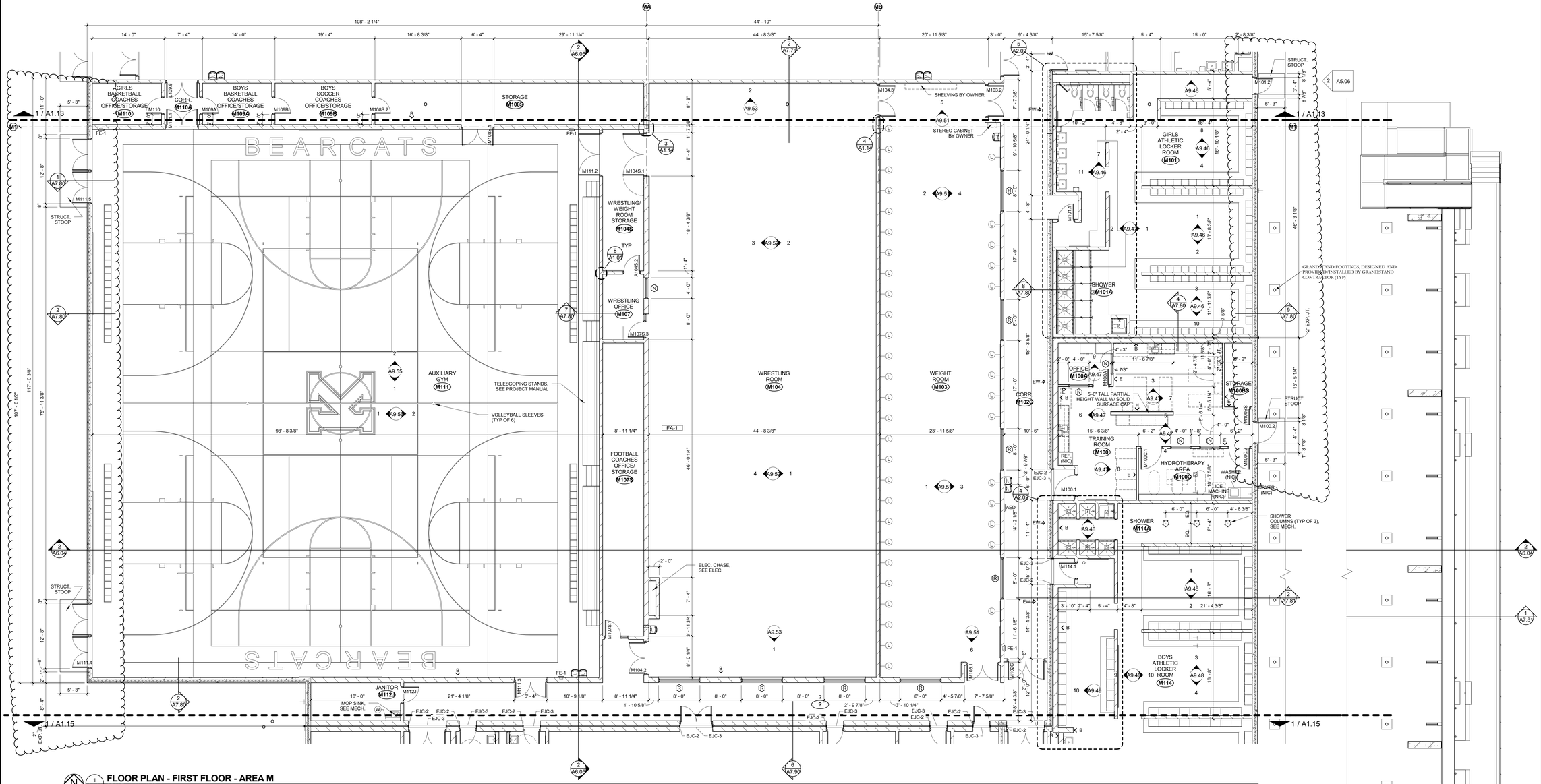
Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area L

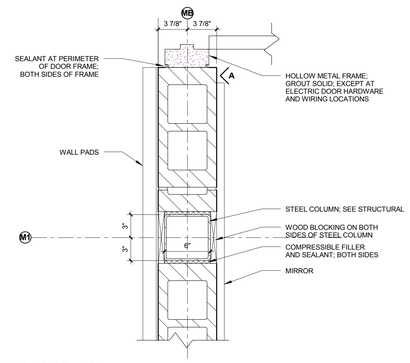
ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

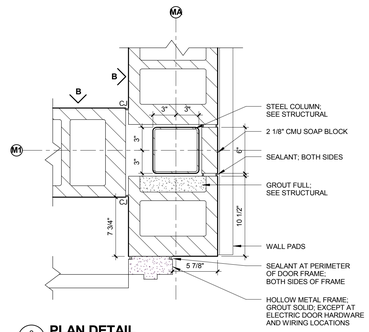
A1.13



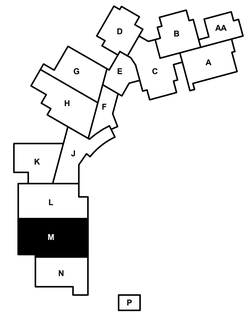
N 1 A1.14 **FLOOR PLAN - FIRST FLOOR - AREA M**
SCALE: 1/8" = 1'-0"



4 A1.14 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



3 A1.14 **PLAN DETAIL**
SCALE: 1 1/2" = 1'-0"



N 2 A1.14 **FLOOR PLAN - FIRST FLOOR - AREA M**
SCALE: 1/8" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

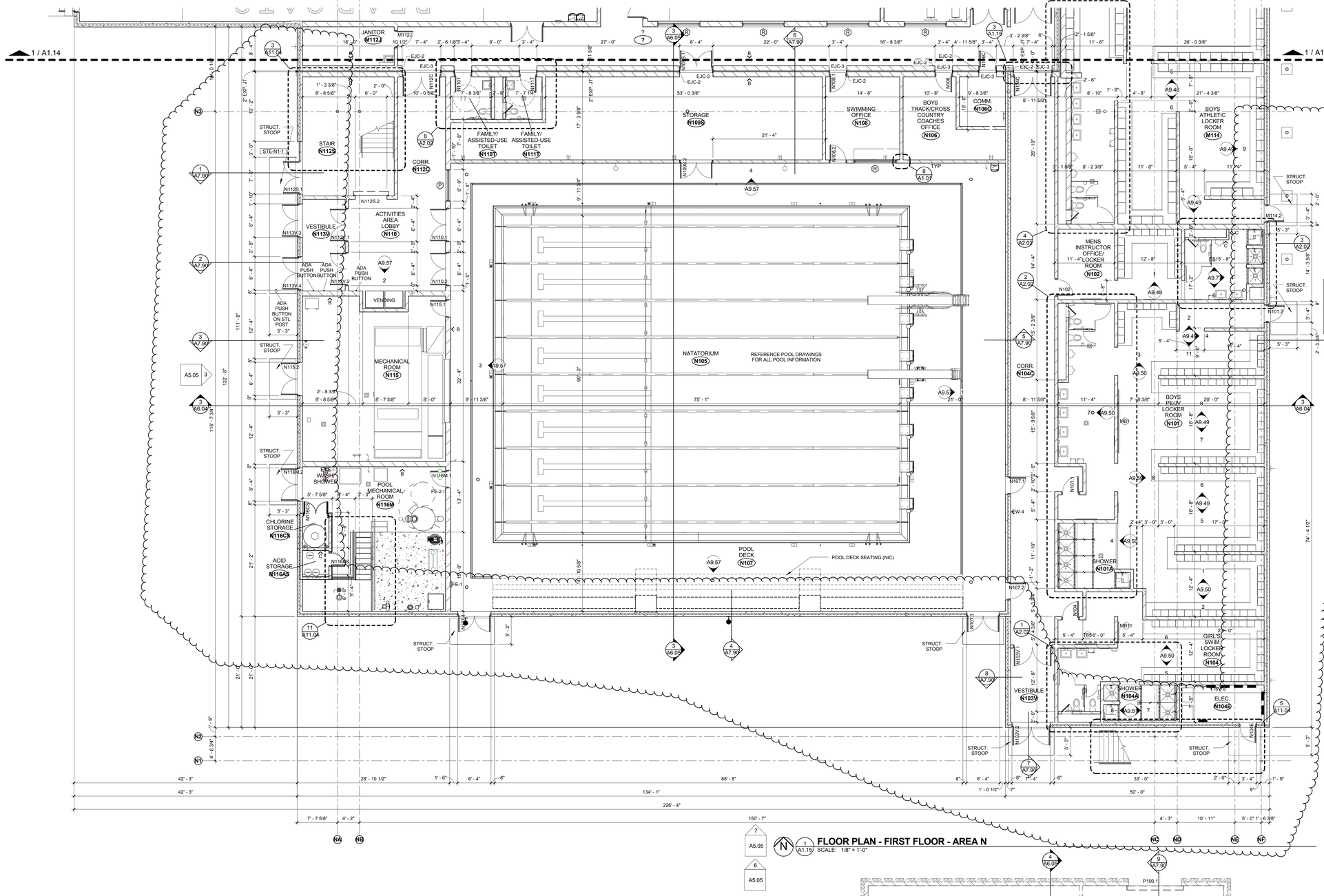
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

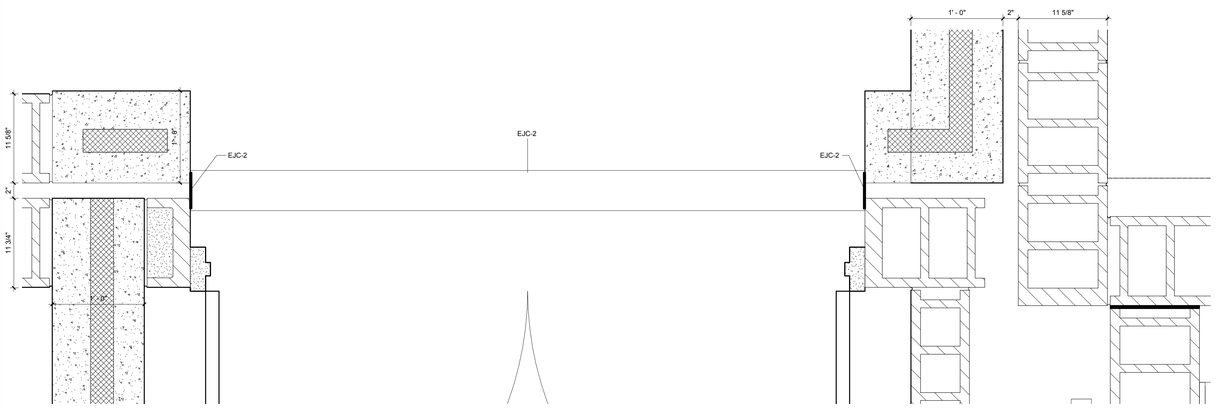
Floor Plan - First Floor - Area M

ProjectNumber: 1355
Date: September 2, 2014

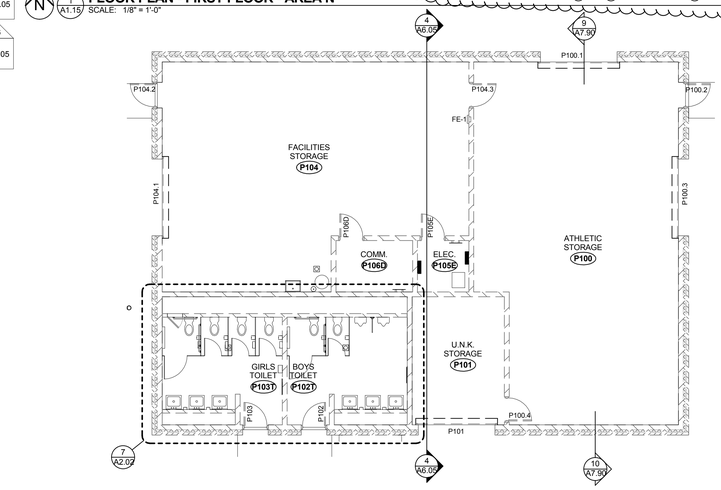
Sheet Number:
A1.14



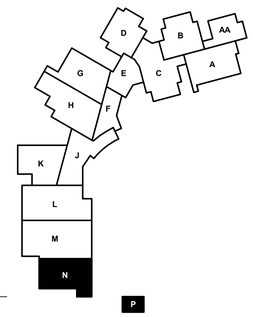
N 1 / A1.15 **FLOOR PLAN - FIRST FLOOR - AREA N**
SCALE: 1/8" = 1'-0"



3 A1.15 **PLAN DETAIL - AREA N1 - DOOR / EXT. PRECAST / INT. CMU**
SCALE: 1 1/2" = 1'-0"



N 2 / A1.15 **FLOOR PLAN - FIRST FLOOR - AREA P (BY OWNER, NOT IN CONTRACT)**
SCALE: 1/8" = 1'-0"



Wilkins Hinrichs Stober Architects L.L.C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747

DLR Group Project No.: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Floor Plan - First Floor - Area N & P

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

A1.15

ARCHITECTURAL GENERAL NOTES

1. GENERAL NOTES APPLY TO ALL SHEETS.

2. ALL INTERIOR DIMENSIONS ARE ACTUAL AND ARE TO FACE OF G.W.B., FACE OF CMU WALLS, FACE OF CONCRETE WALLS, FACE OF FRAMES, OR CENTERLINE OF COLUMNS, UNLESS NOTED OTHERWISE.

3. FLOOR ELEVATIONS ARE SHOWN THUS TYP. FIN. FLOOR 100' - 0"

4. FLOOR SPOT ELEVATIONS ARE SHOWN THUS XX' - X"

5. ALL INTERIOR WALLS ARE TYPE "A" INTERIOR WALL ASSEMBLIES, UNLESS NOTED OTHERWISE.

6. CONTRACTOR IS RESPONSIBLE FOR INSTALLING MASONRY CONTROL JOINTS (CJ) WHERE INDICATED AND AS SPECIFIED.

7. ALL WALLS SHALL EXTEND TO UNDERSIDE OF FLOOR OR ROOF DECK ABOVE, UNLESS NOTED OTHERWISE.

8. SCRIBE GYPSUM BOARD OF WALL AND PARTITIONS TO IRREGULARITIES OF DECK ABOVE. SEAL TIGHTLY AROUND ANY PARTITIONS. FILL IRREGULARITIES BETWEEN TOP OF WALL AND DECK ABOVE WITH FIRE SAFING INSULATION OR FIRE STOPPING MATERIALS AS REQUIRED TO MEET FIRE RATING OF RESPECTIVE WALLS.

9. SEE CODE PLAN SHEETS FOR LOCATION OF WALLS OF FIRE-RESISTIVE CONSTRUCTION. ALL WALLS OF FIRE-RESISTIVE CONSTRUCTION SHALL EXTEND TO UNDERSIDE OF FLOOR OR ROOF DECK ABOVE AND ACHIEVE PROPER FIRE-RESISTIVE RATING.

10. SEAL ALL PENETRATIONS THROUGH ALL WALLS. RATED WALLS SHALL BE SEALED WITH FIRE STOPPING MATERIAL AS REQUIRED TO ACHIEVE THE RESPECTIVE FIRE-RESISTIVE RATING AND SMOKE STOPPAGE. SEE PROJECT MANUAL.

11. THE CONTRACTOR SHALL FURNISH AND INSTALL WOOD BLOCKING IN METAL STUD PARTITIONS FOR THE PROPER ANCHORAGE OF ALL WALL ATTACHED ITEMS: I.E. TOILET ACCESSORIES, CASEWORK, MILLWORK, WALL-MOUNTED FIXTURES, MARKER BOARDS, TACK BOARDS, ETC.

12. GYPSUM BOARD SURFACES SHALL BE ISOLATED WITH CONTROL JOINTS WHERE SHOWN ON DRAWINGS AND AS DESCRIBED IN THE SPECIFICATIONS.

13. THE CONTRACTOR SHALL FURNISH AND INSTALL WALL BASE (TYP) AROUND ALL CASEWORK AND MILLWORK.

14. THE OWNER SHALL FURNISH AND INSTALL THE FOLLOWING ITEMS:
    1. FURNITURE NOT IN CONTRACT (NIC)
    2. EQUIPMENT NOT IN CONTRACT (NIC)

15. THE OWNER SHALL BE RESPONSIBLE FOR PROVIDING THE CONTRACTOR WITH ROUGH-IN INFORMATION NECESSARY TO ACCOMMODATE THE INSTALLATION OF OWNER FURNISHED AND INSTALLED ITEMS.

16. THE CONTRACTOR SHALL INCLUDE ALL OWNER FURNISHED AND INSTALLED ITEMS IN THE CONSTRUCTION SCHEDULE, AND SHALL COORDINATE WITH THE OWNER TO ACCOMMODATE THESE ITEMS.

17. THE OWNER SHALL BE RESPONSIBLE FOR ADHERING TO THE CONSTRUCTION SCHEDULE AS ESTABLISHED BY THE CONTRACTOR.

18. CONTRACTOR SHALL COORDINATE ALL MECHANICAL CHASE SIZES WITH THE MECHANICAL SUB-CONTRACTOR AND ELECTRICAL CHASE SIZES WITH THE ELECTRICAL SUB-CONTRACTOR.

19. CONTRACTOR SHALL COORDINATE WITH MECHANICAL AND ELECTRICAL SUB-CONTRACTORS SIZE AND LOCATION OF EQUIPMENT PADS. ALL HOUSEKEEPING PADS TO BE 4" RAISED SLABS AS DETAILED, UNLESS NOTED OTHERWISE.

20. DIMENSIONS FOR DOOR AND WINDOW OPENINGS ARE SHOWN NOMINAL. ALLOW FOR 1/4"-INCH (10) SHIM AND SEALANT OF EXTERIOR FRAMES.

21. ARCHITECTURAL FINISH FLOOR ELEVATION 100'-0" EQUALS ACTUAL SITE REFERENCE ELEVATION OF FINISH FLOOR 2160.75 FEET.

22. EXTEND FURRING CHANNELS AND GYPSUM BOARD UP TO UNDERSIDE OF MTL. DECK ON ALL WALLS, UNLESS NOTED OTHERWISE.

23. SEAL ALL PENETRATIONS THROUGH ALL WALLS.

24. CORNER GUARDS ARE NOTED ON PLANS AS "CG."

25. SEE WALL SECTIONS, ENLARGED PLANS AND STRUCTURAL FOR EXTERIOR AND INTERIOR WALL CONSTRUCTION NOT SHOWN ON THE FLOOR PLANS.

26. ALL EXTERIOR DIMENSIONS ARE TO FACE OF BRICK, FACE OF METAL WALL PANEL, CAST-IN-PLACE CONCRETE FOUNDATION WALLS, OR CENTERLINE OF COLUMNS.

27. ALL FIRE EXTINGUISHERS CABINETS (FE-1) TO BE MOUNTED WITH TOP OF CABINET AT 54" A.F.F. WITH BOTTLE CONTROLS NO HIGHER THAN 48". FIRE EXTINGUISHERS w/ BRACKETS (FE-2) TO BE MOUNTED w/ TOP OF BOTTLE AT 48" ABOVE FINISHED FLOOR.

28. PROVIDE CONTROL JOINTS IN GWB WALLS ABOVE ALL DOOR FRAMES. PROVIDE CONTROL JOINTS AT BOTH CORNERS ON BOTH SIDES OF THE DOOR FRAME FROM TOP OF DOOR TO CEILING. PROVIDE CONTROL JOINTS AT ALL PAINT COLOR CHANGES/TRANSITIONS IN ALL CORRIDORS, BOTH VERTICAL AND/OR HORIZONTAL. ALL CONTROL JOINTS MUST TERMINATE AT ONE OF THE FOLLOWING: FLOOR, DOOR/FRAME, CEILING, OR ADJACENT PERPENDICULAR WALL.

29. ARCHITECT TO SELECT ALL PRODUCT / MATERIAL COLORS FROM ALL AVAILABLE MANUFACTURER'S COLORS, UNLESS NOTED OTHERWISE.

30. ALL STRUCTURAL STOOPS TO HAVE 1/50 SLOPE (MAX.). SEE STRUCTURAL.

31. ALL DOOR AND WINDOW JAMBS IN MASONRY WALLS SHALL BE OFFSET 8" FROM ADJACENT PERPENDICULAR WALLS, UNLESS NOTED OTHERWISE. ALL DOOR AND WINDOW JAMBS IN METAL STUD WALLS SHALL BE OFFSET 4" FROM ADJACENT PERPENDICULAR WALLS, UNLESS NOTED OTHERWISE.

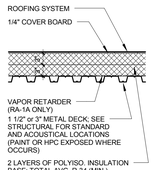
32. CONTRACTOR IS RESPONSIBLE FOR VERIFYING THAT ALL DOORS CLEAR ANY AND ALL FLOOR FINISH TRANSITIONS/TRANSITION STRIPS AND REDUCER STRIPS.

33. CONTRACTOR'S ARE REQUIRED TO SCHEDULE A "MEET ME LOCATE" FROM DIGGERS HOTLINE OF NEBRASKA (NEBRASKA ONE CALL) 1-800-331-5666 PRIOR TO BEGINNING ANY EXCAVATION OR WORK ON THE PROJECT SITE. ATTENDANCE IS REQUIRED BY CONSTRUCTION MANAGER/OWNER'S REPRESENTATIVE, PERTINENT CONTRACTORS, AND ALL UTILITY COMPANIES.

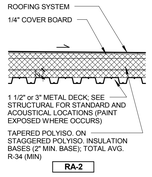
34. PROVIDE AND INSTALL RECESSED "KNOXBOX COMPANY" (800) 552-5669 KEY LOCK BOX MODEL 3224 w/ ALUMINUM FINISH AND 3240 LIFT OFF DOOR RECESSED MOUNTING KIT. COORDINATE WITH ARCHITECT AND LOCAL FIRE ADMINISTRATOR FOR AUTHORIZED SIGNATURE REQUIRED AND LOCATION.

35. CONTRACTOR SHALL COORDINATE WITH ELECTRICAL SUB-CONTRACTORS FOR SIZE AND LOCATIONS OF PLYWOOD REQUIRED FOR WALL MOUNTED EQUIPMENT; ALL PLYWOOD TO BE PAINTED (COLOR TO MATCH WALL FINISH); SEE DETAIL ON ELECTRICAL DRAWINGS

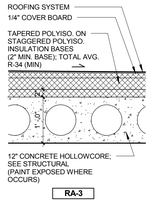
ROOF ASSEMBLIES



RA-1
RA-1A

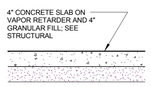


RA-2



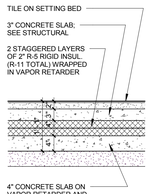
RA-3

FLOOR ASSEMBLIES

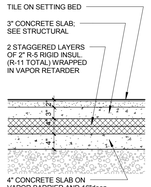


FA-1

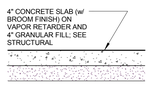
FA-2



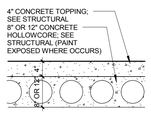
FA-3A



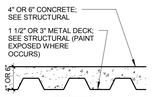
FA-3B



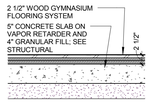
FA-4



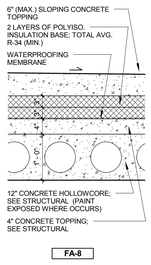
FA-5A 8" CMU
FA-5B 12" CMU



FA-6A 4"
FA-6B 6"

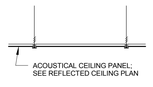


FA-7

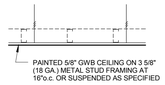


FA-8

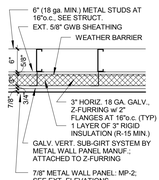
CEILING ASSEMBLIES



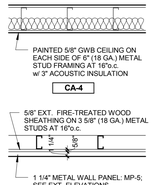
CA-1



CA-2



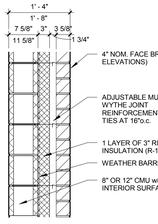
CA-3



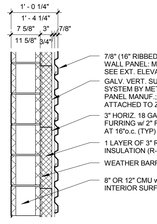
CA-4

CA-5

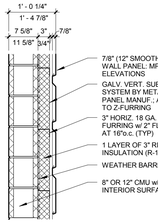
EXTERIOR WALL ASSEMBLIES



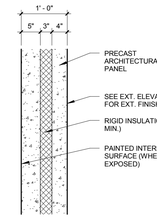
EW-1A 8" CMU
EW-1B 12" CMU



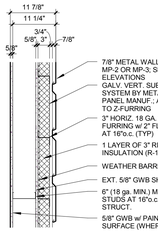
EW-2A 8" CMU
EW-2B 12" CMU



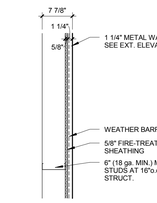
EW-3A 8" CMU
EW-3B 12" CMU



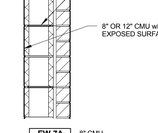
EW-4



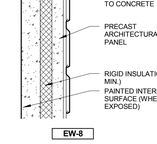
EW-5A MP-1
EW-5B MP-2



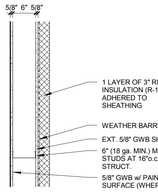
EW-6



EW-7A 8" CMU
EW-7B 12" CMU

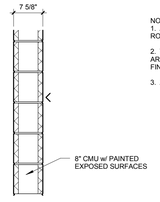


EW-8



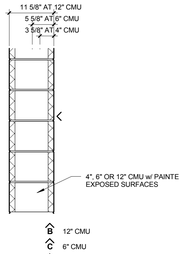
EW-9

INTERIOR WALL ASSEMBLIES

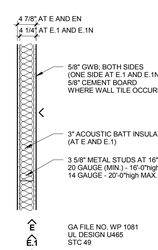


A

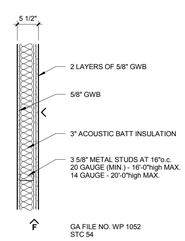
NOTES:
1. ALL INTERIOR WALLS WILL EXTEND FROM FINISHED FLOOR TO BOTTOM OF ROOF/FLOOR STRUCTURE ABOVE, UNLESS NOTED OTHERWISE.
2. THE SIDE OF THE WALL THAT THE INTERIOR ASSEMBLY NUMBER/LETTER AND ARROW SYMBOL IS LOCATED WILL INDICATE THE SIDE THAT RECEIVES THE FINISH/MATERIAL AND/OR DOUBLE LAYER AS DESCRIBED IN THE ASSEMBLY.
3. ALL INTERIOR WALLS ARE TYPE A UNLESS NOTED OTHERWISE.



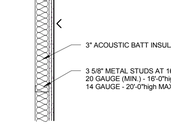
A 12" CMU
B 6" CMU
C 4" CMU



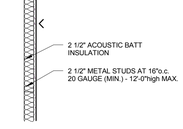
E GA FILE NO. WP 1081 UL DESIGN U465 STC 49
EN (N = NO INSULATION)
E.1N



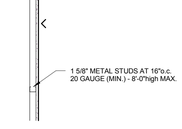
F GA FILE NO. WP 1052 STC 54



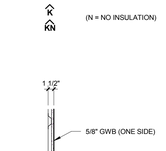
G GA FILE NO. WP 1522 STC 59



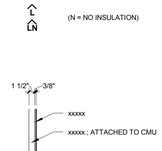
H GA FILE NO. WP 1081 UL DESIGN U465 STC 49
HN (N = NO INSULATION)
H.1N



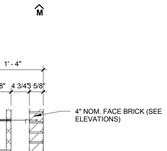
J GA FILE NO. WP 1081 UL DESIGN U465 STC 49
JN (N = NO INSULATION)
J.1N



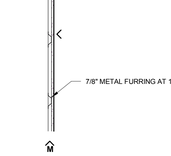
K GA FILE NO. WP 1081 UL DESIGN U465 STC 49
KN (N = NO INSULATION)
K.1N



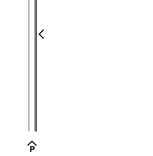
L GA FILE NO. WP 1081 UL DESIGN U465 STC 49
LN (N = NO INSULATION)
L.1N



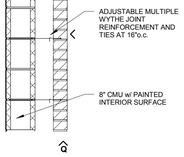
M GA FILE NO. WP 1081 UL DESIGN U465 STC 49
MN (N = NO INSULATION)
M.1N



P GA FILE NO. WP 1081 UL DESIGN U465 STC 49
PN (N = NO INSULATION)
P.1N

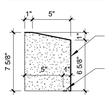


Q GA FILE NO. WP 1081 UL DESIGN U465 STC 49
QN (N = NO INSULATION)
Q.1N

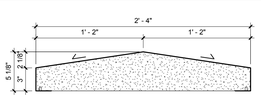


R GA FILE NO. WP 1081 UL DESIGN U465 STC 49
RN (N = NO INSULATION)
R.1N

STONE PROFILES



PROFILE A



PROFILE B

PROFILE C

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-6747
DLR Group Project No.: 10-13113-00

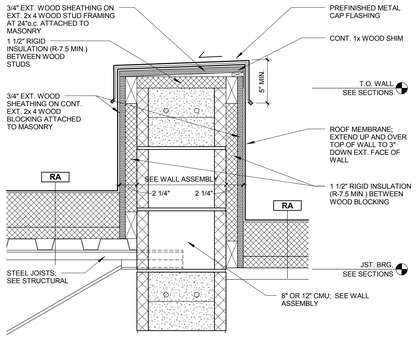
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

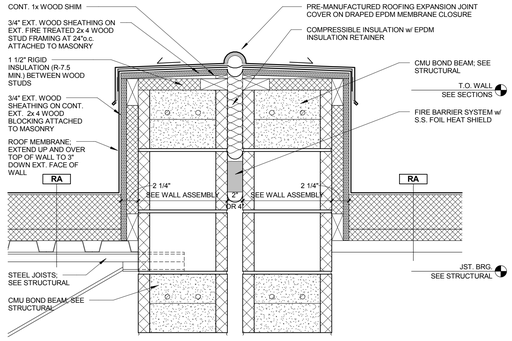
Architectural General Notes & Assemblies

ProjectNumber: 1355
Date: September 2, 2014

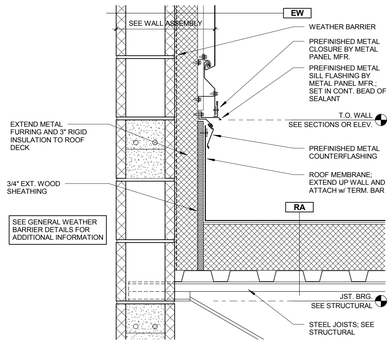
Sheet Number:
A1.60



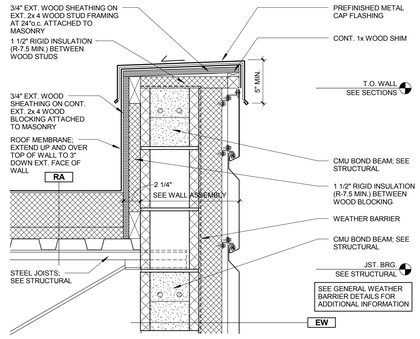
18 A7.01 **TYP. ROOF CMU PARAPET DETAIL**
SCALE: 1 1/2" = 1'-0"



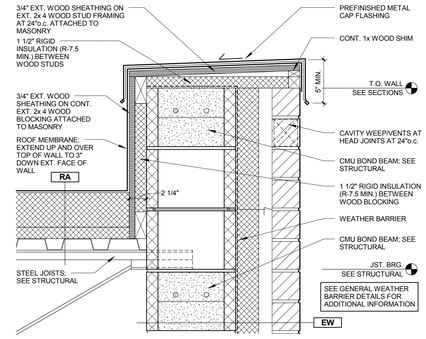
17 A7.01 **TYP. MASONRY WALL ROOF EXPANSION DETAIL**
SCALE: 1 1/2" = 1'-0"



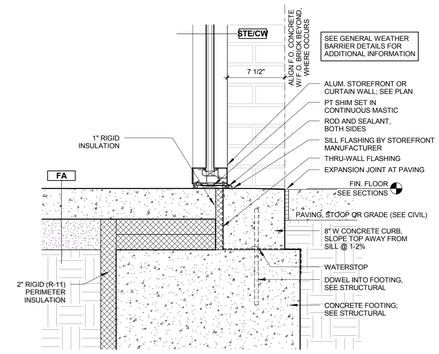
16 A7.01 **TYP. METAL WALL PANEL ROOF DETAIL**
SCALE: 1 1/2" = 1'-0"



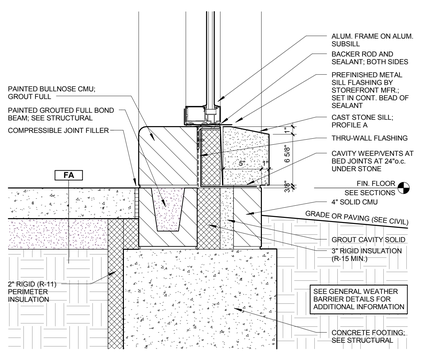
15 A7.01 **TYP. METAL WALL PANEL CMU PARAPET DETAIL**
SCALE: 1 1/2" = 1'-0"



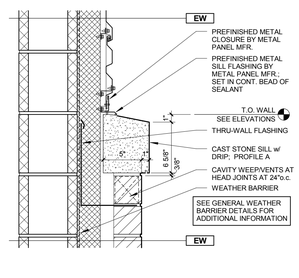
14 A7.01 **TYP. MASONRY WALL PARAPET DETAIL**
SCALE: 1 1/2" = 1'-0"



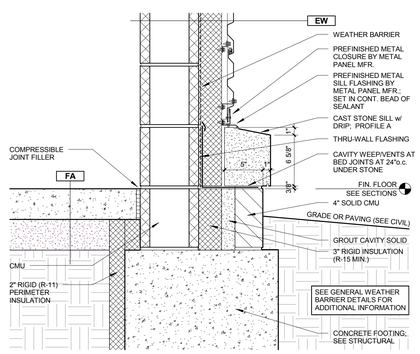
13 A7.01 **STOREFRONT SILL DETAIL AT GRADE**
SCALE: 1 1/2" = 1'-0"



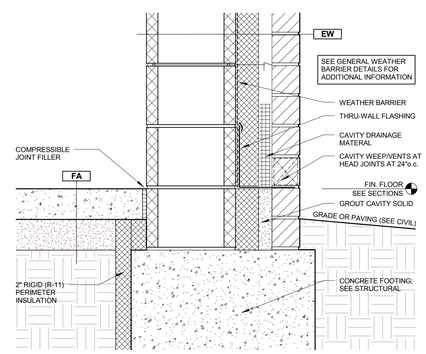
12 A7.01 **TYP. STOREFRONT CMU BASE DETAIL**
SCALE: 1 1/2" = 1'-0"



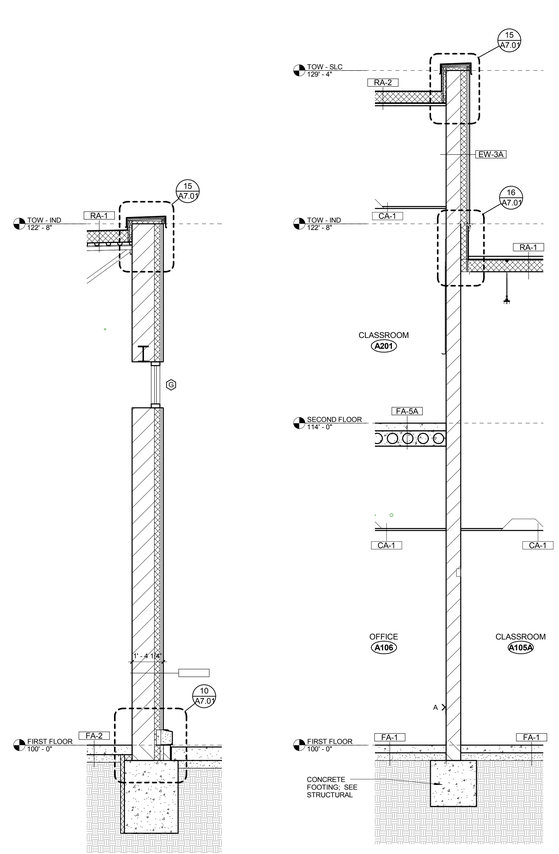
11 A7.01 **TYP. METAL WALL PANEL TO MASONRY WALL DETAIL**
SCALE: 1 1/2" = 1'-0"



10 A7.01 **TYP. METAL WALL PANEL CMU BASE DETAIL**
SCALE: 1 1/2" = 1'-0"



9 A7.01 **TYP. MASONRY WALL BASE DETAIL**
SCALE: 1 1/2" = 1'-0"



6 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

5 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

4 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

3 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

2 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

1 A7.01 **WALL SECTION - AREA A**
SCALE: 3/8" = 1'-0"

Wilkins Hinrichs Stober Architects L.L.C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOR REFERENCE ONLY, NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors
6457 Frances Street, Suite 200
Omaha, NE 68106
tel (402) 393-4100 fax (402) 393-8747
DLR Group Project No.: 10-13113-00

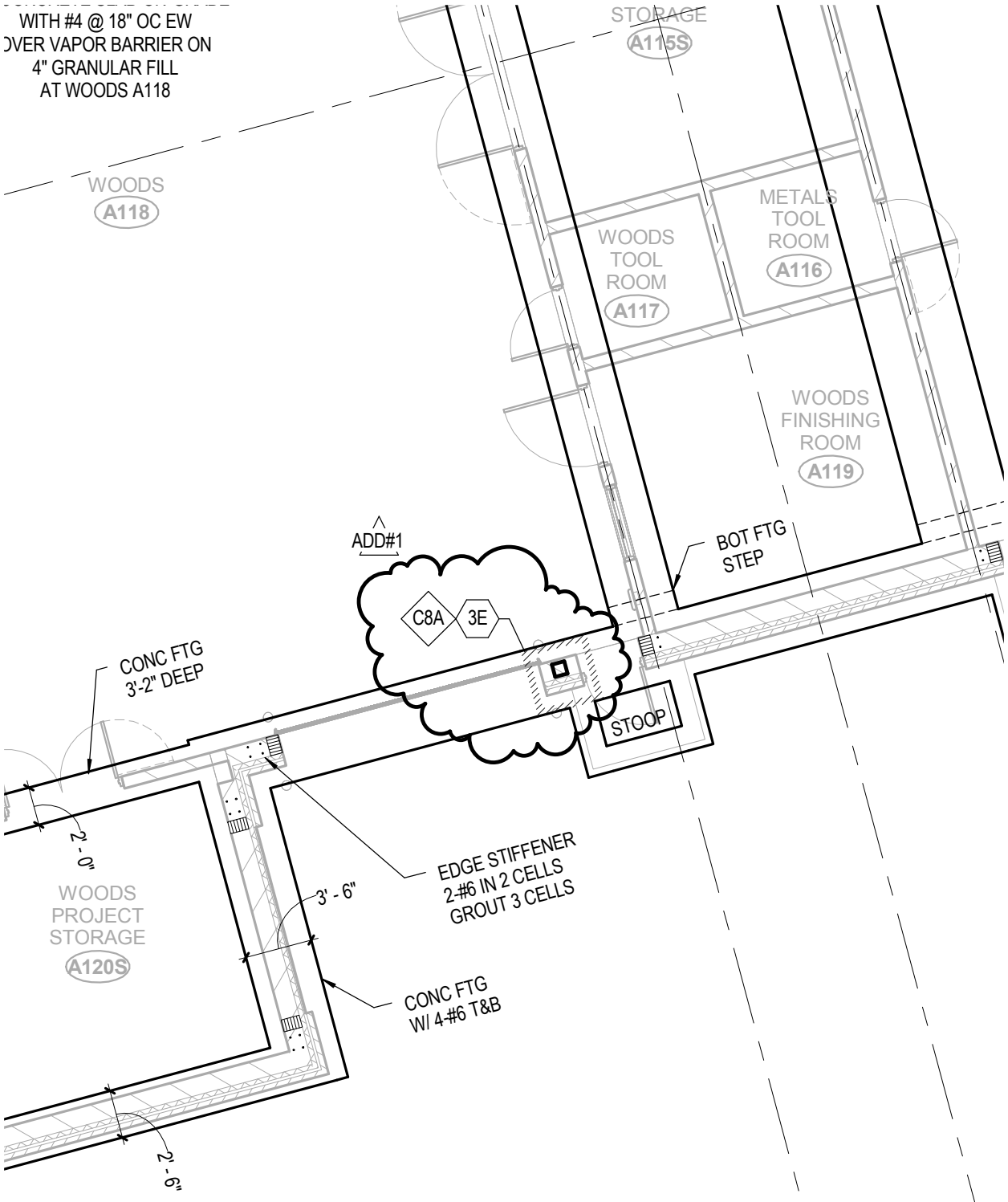
Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue	Date
Addendum No. 1	09/12/2014

Wall Sections - Area A

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:
A7.01



Attachment No. S1.1-1
to Addendum No. 1
Dated: September 12, 2014

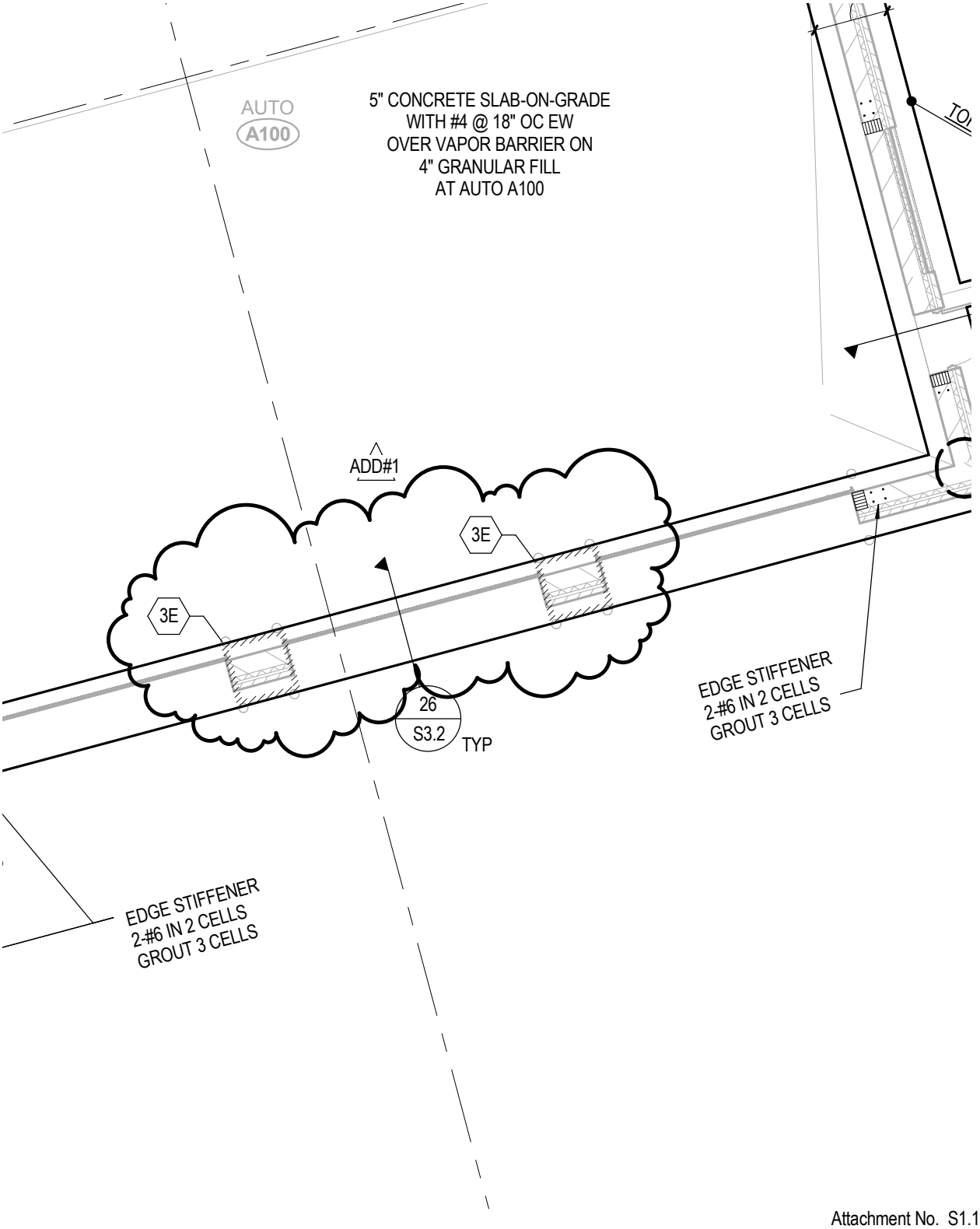
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.1-2
to Addendum No. 1
Dated: September 12, 2014

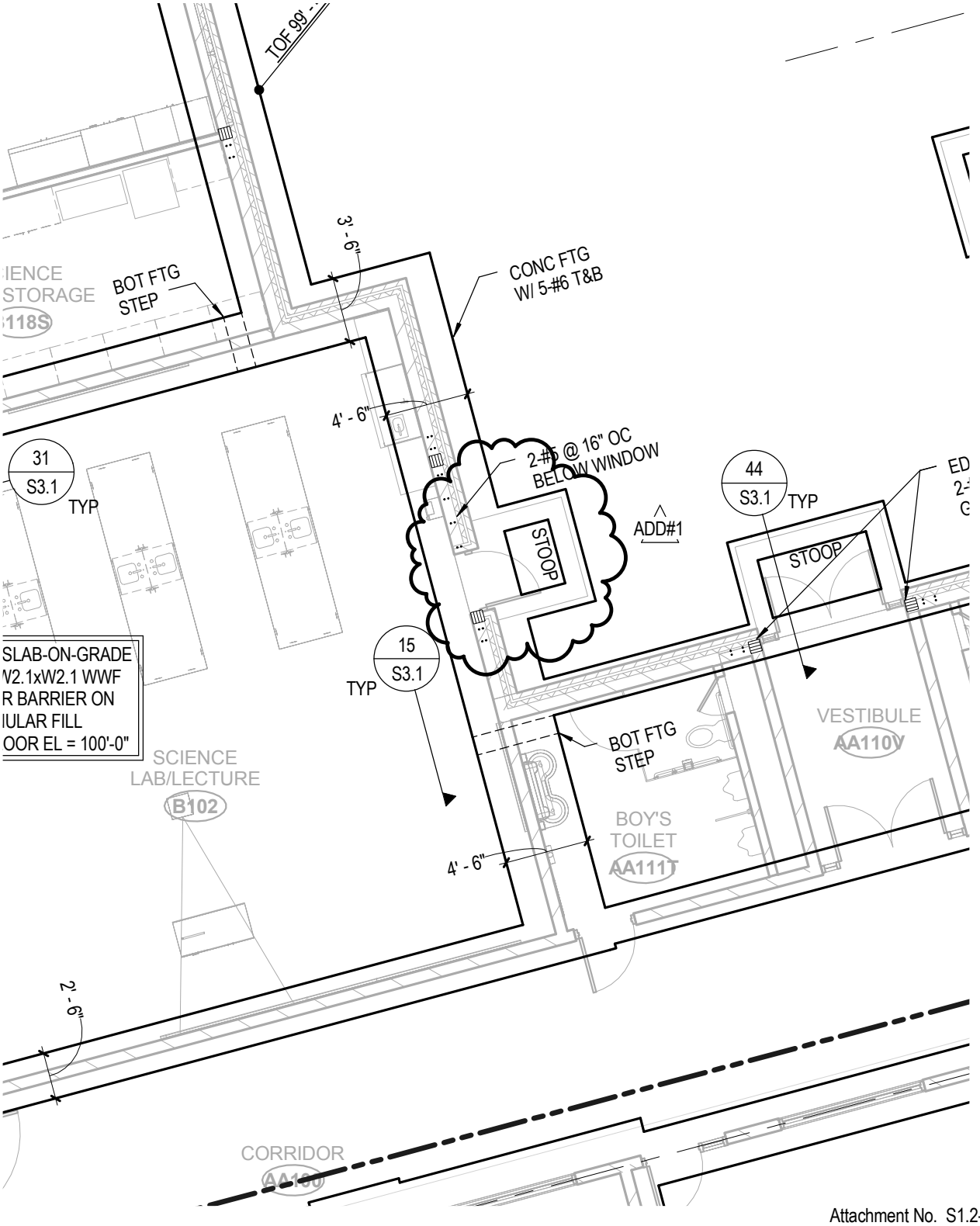
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.2-1
to Addendum No. 1
Dated: September 12, 2014

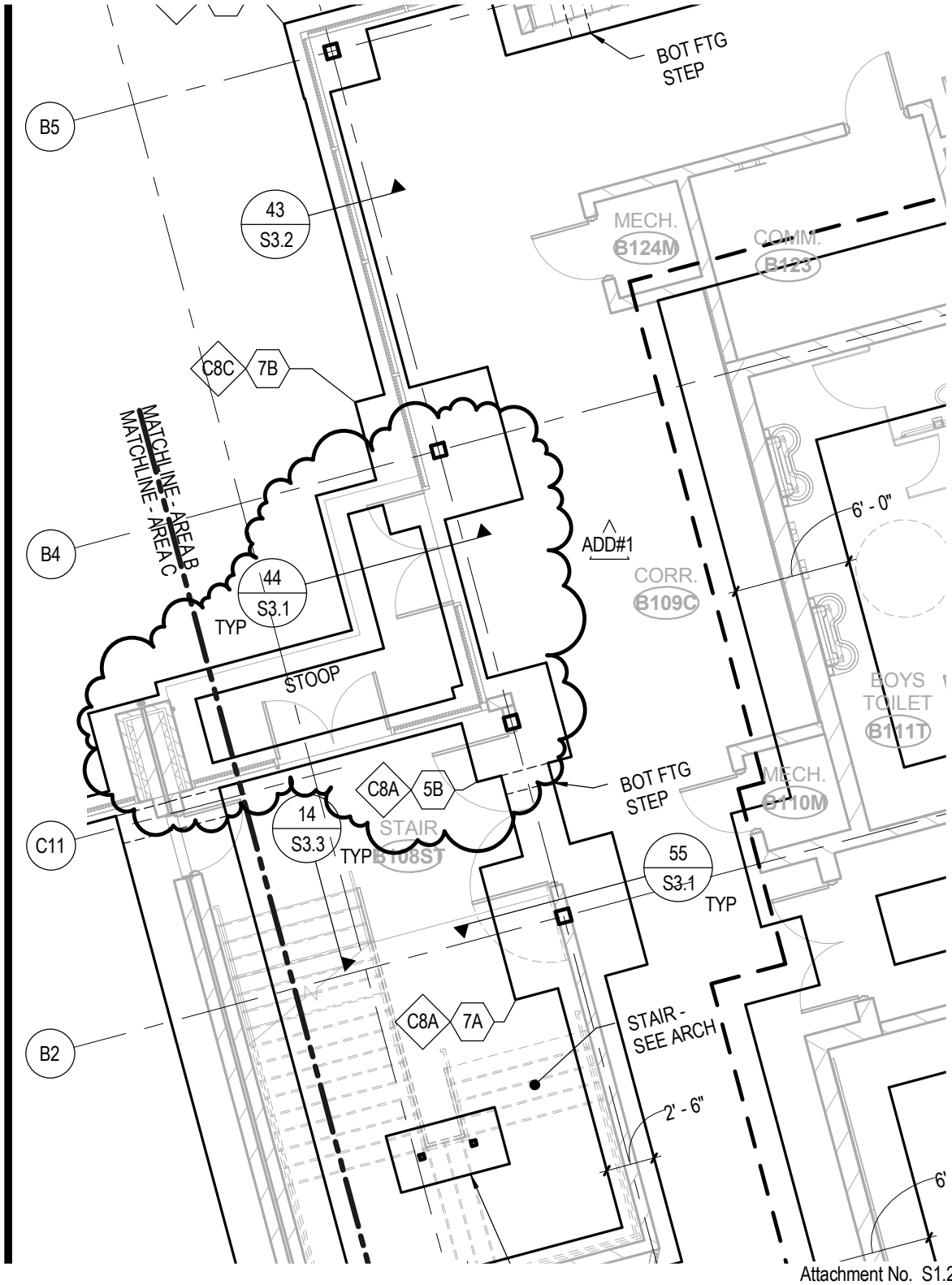
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.2-2
to Addendum No. 1
Dated: September 12, 2014

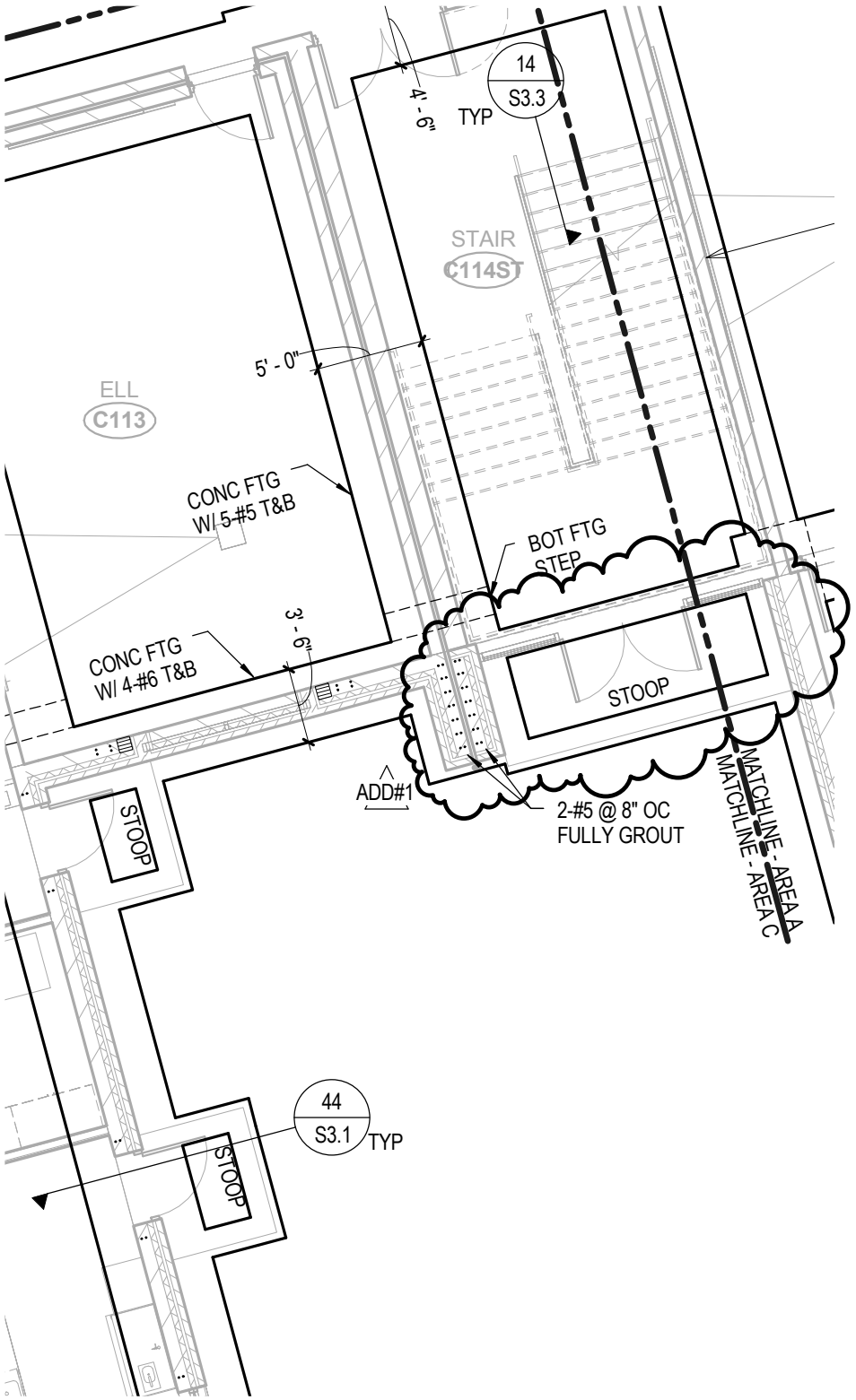
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.3-1
to Addendum No. 1
Dated: September 12, 2014

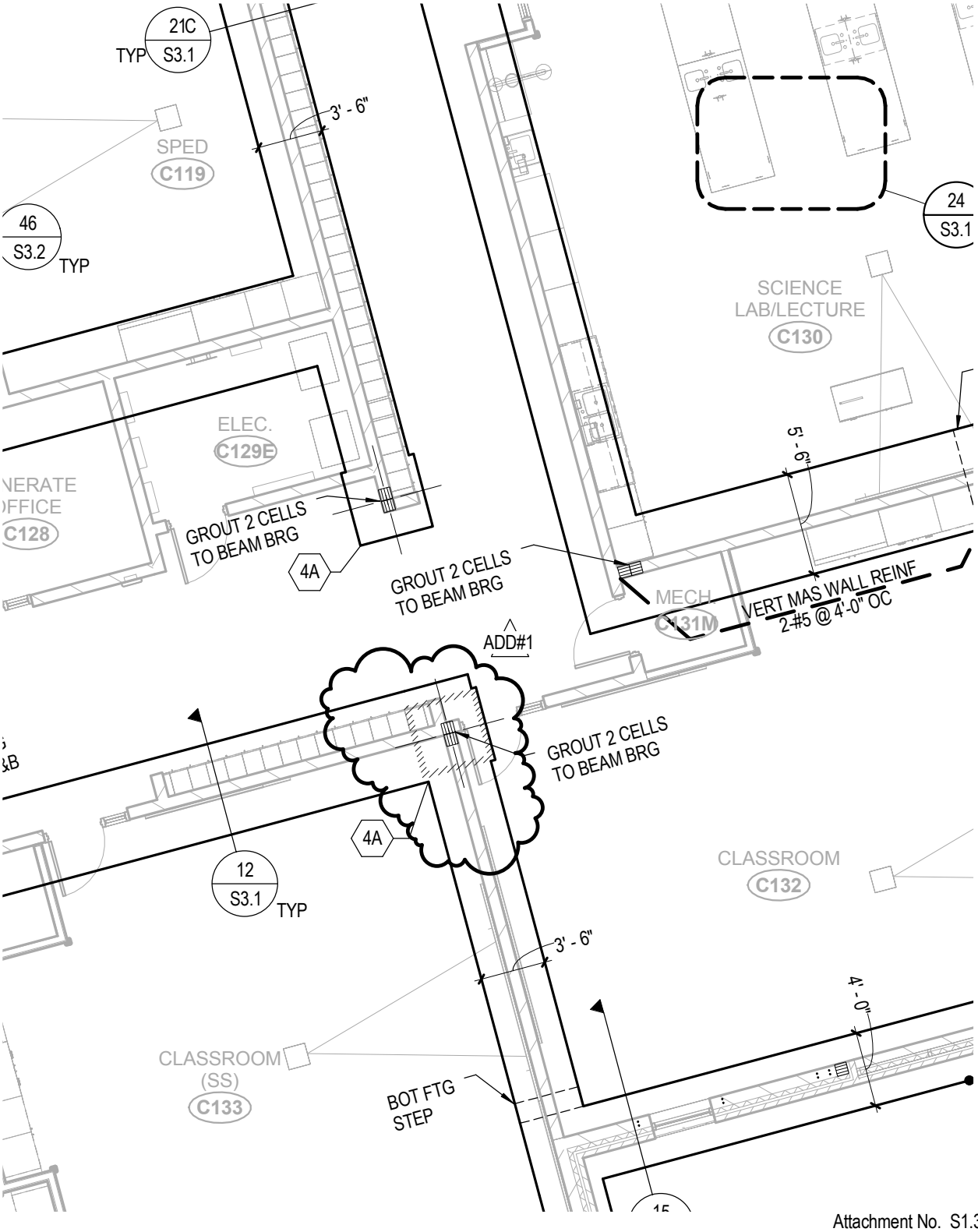
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.3-2
to Addendum No. 1
Dated: September 12, 2014

Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355

Attachment No. S1.3-3
to Addendum No. 1
Dated: September 12, 2014

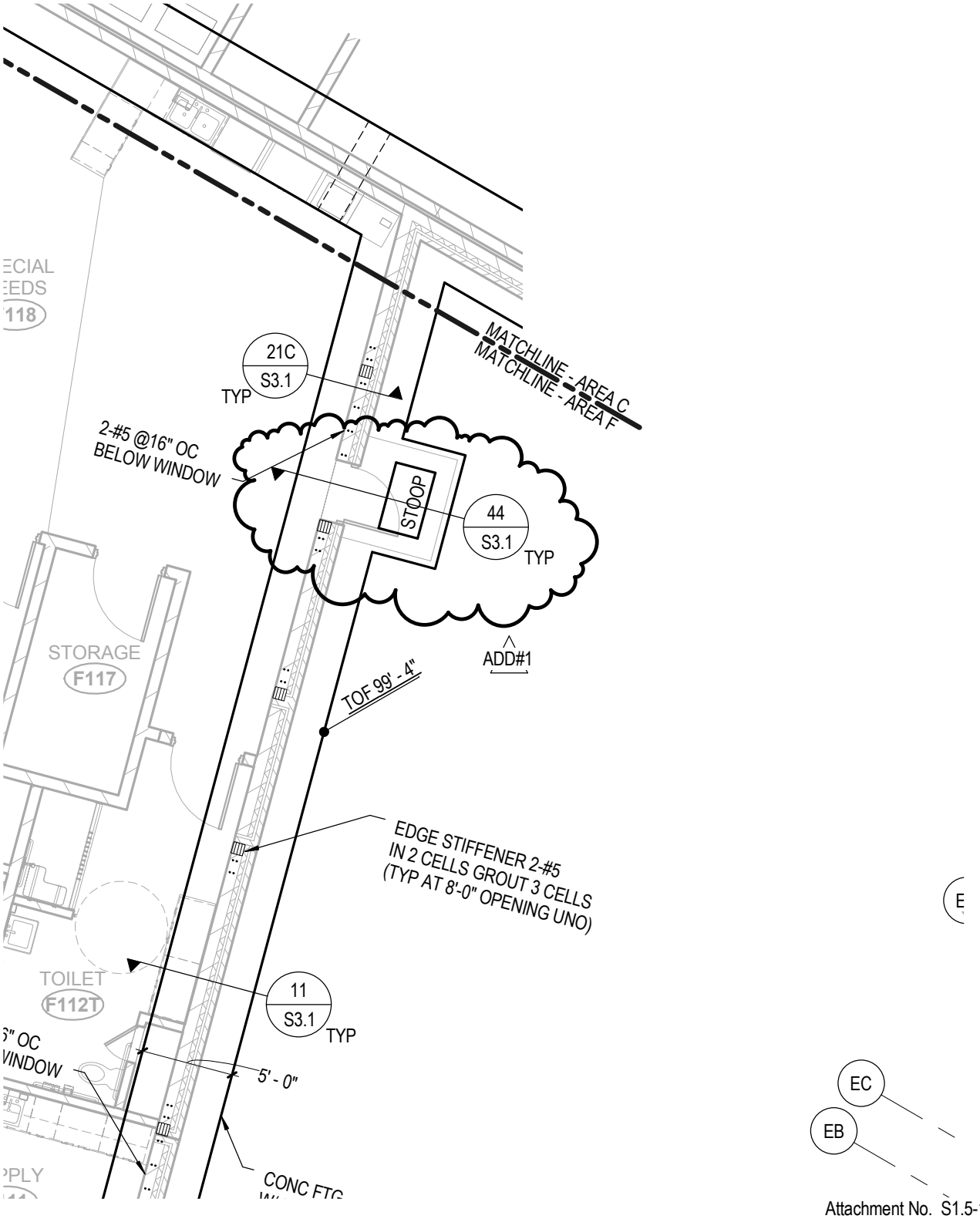
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.5-1
to Addendum No. 1
Dated: September 12, 2014

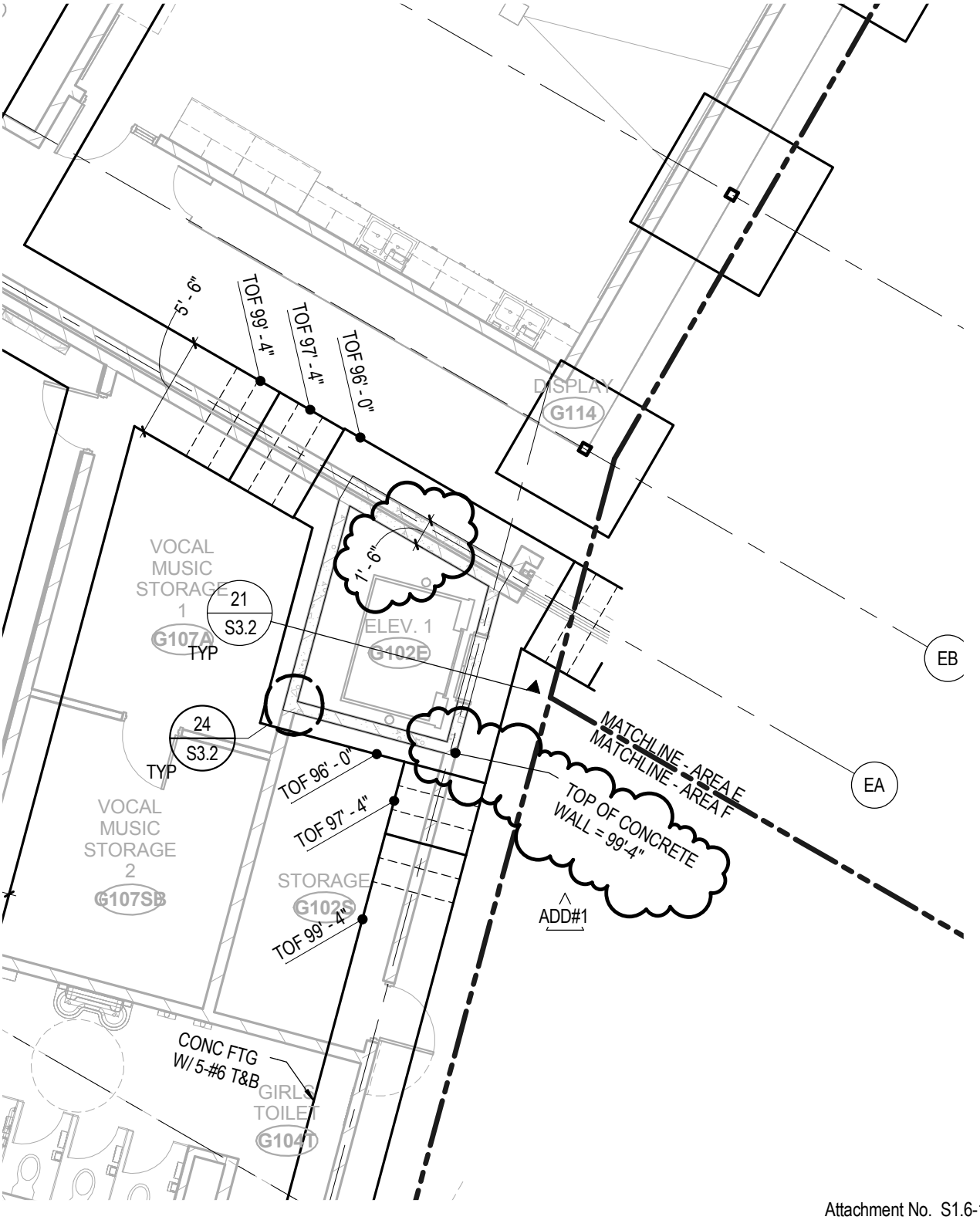
Wilkins Hinrichs Stober Architects, L.L.C.

DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355



Attachment No. S1.6-1
to Addendum No. 1
Dated: September 12, 2014

Wilkins Hinrichs Stober Architects, L.L.C.

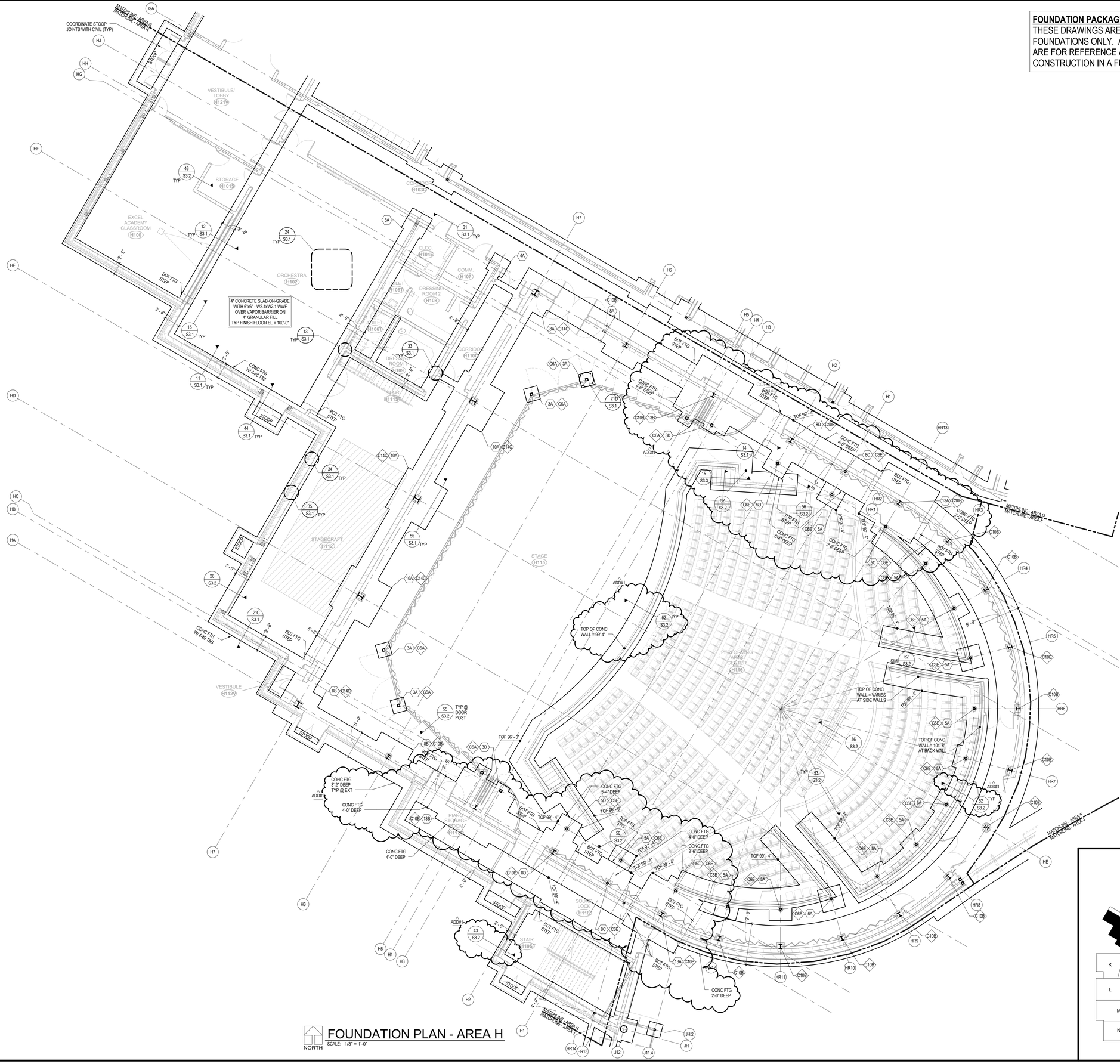
DLR Group
Architecture Engineering Planning Interiors

DLR Project # 10-13113-00

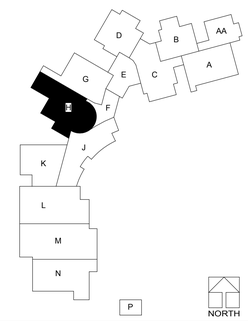
Kearney Public Schools
New Kearney High School -
Foundation Package

Project Number: 1355

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.



FOUNDATION PLAN - AREA H
SCALE: 1/8" = 1'-0"



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

JAMES A. MCCONNELL E-14279 9-25-14 STATE OF NEBRASKA

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.7-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

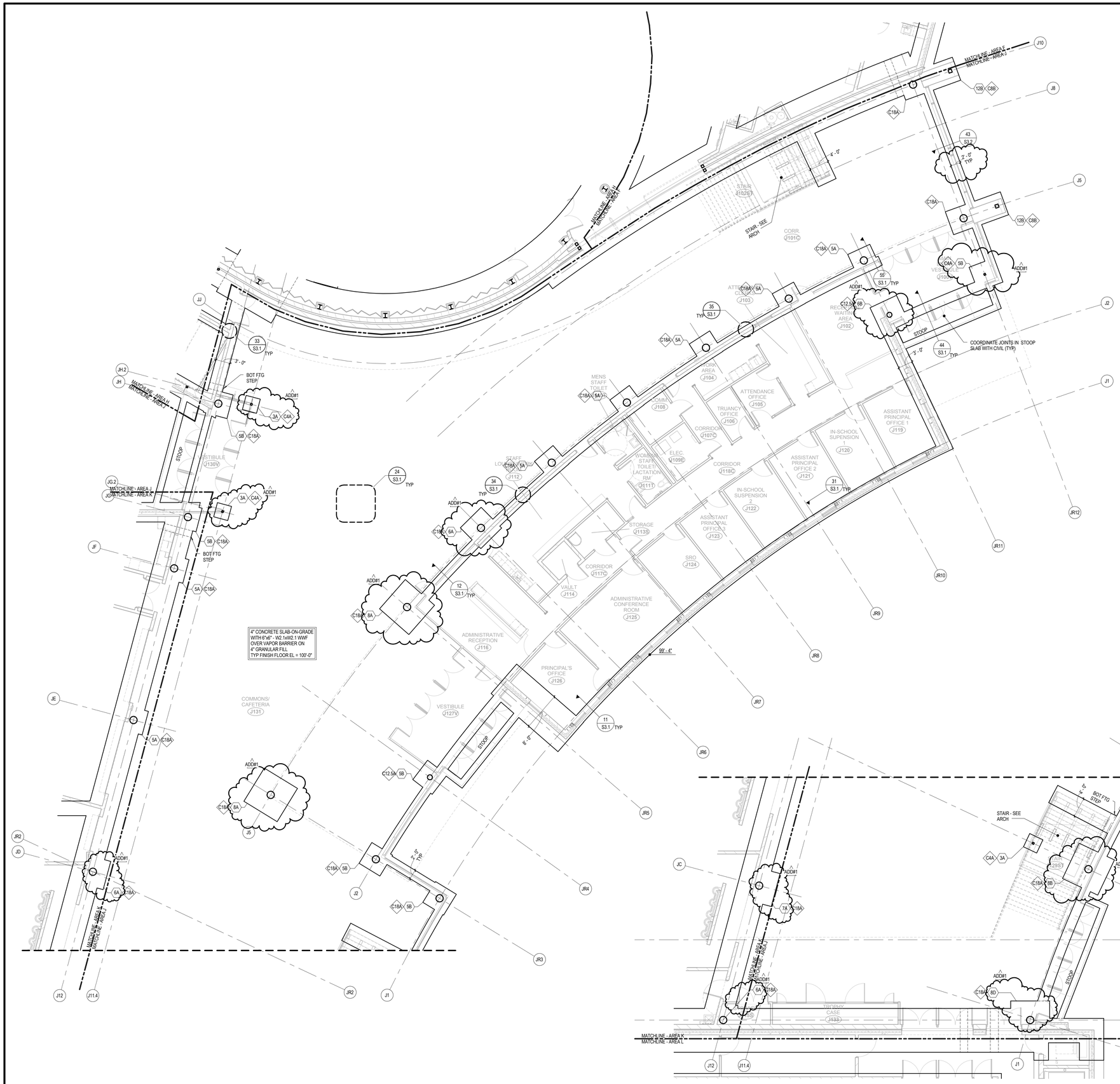
Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREA H

ProjectNumber: 1355
Date: September 2, 2014

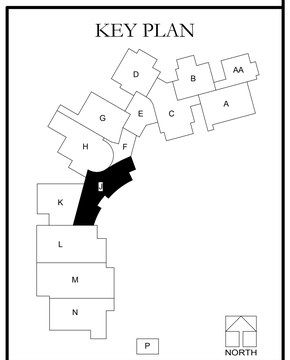
Sheet Number:
S1.7

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.



FOUNDATION PLAN - AREA J
SCALE: 1/8" = 1'-0"

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.8-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

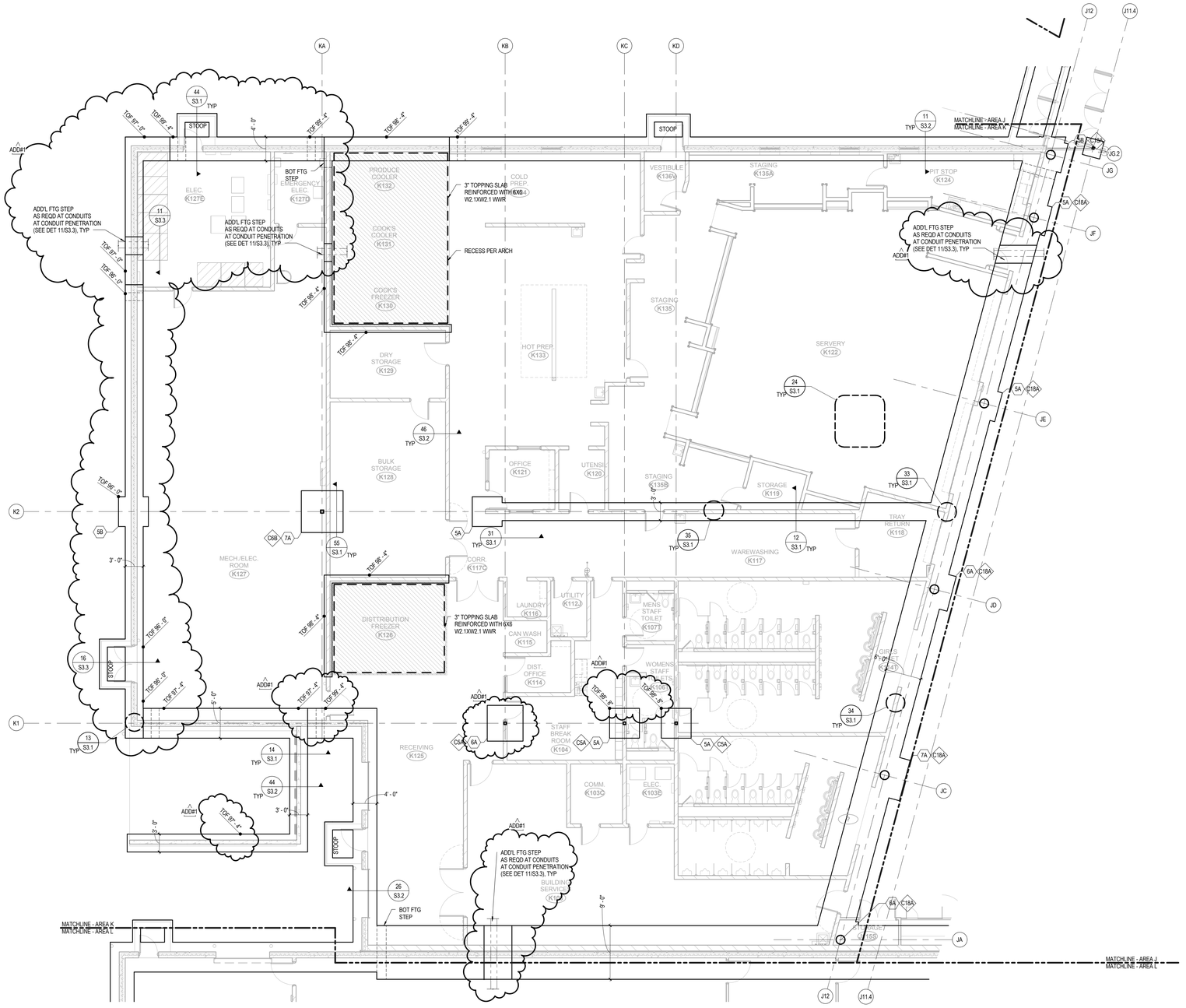
Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREA J

ProjectNumber: 1355
Date: September 2, 2014

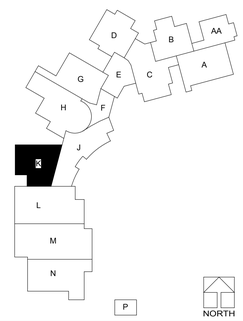
Sheet Number:
S1.8

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.



FOUNDATION PLAN - AREA K
SCALE: 1/8" = 1'-0"

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

PROFESSIONAL STRUCTURAL ENGINEER
RICHARD J. MCDONNELL
E-14275
9-12-14
STATE OF NEBRASKA

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.9-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

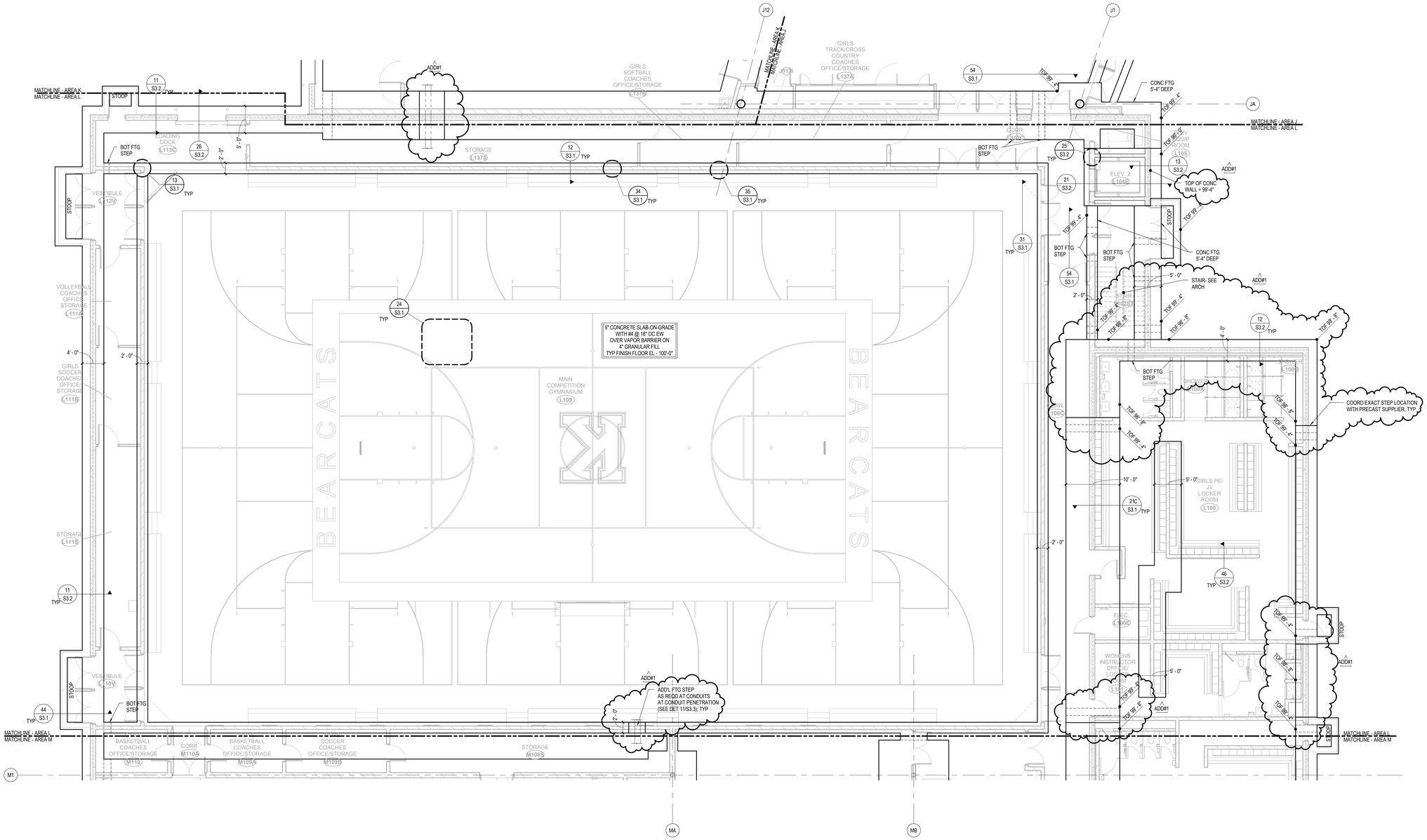
Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREA K

ProjectNumber: 1355
Date: September 2, 2014

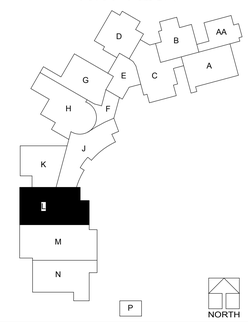
Sheet Number:
S1.9

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.



FOUNDATION PLAN - AREA L
SCALE: 1/8" = 1'-0"
NORTH

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.10-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

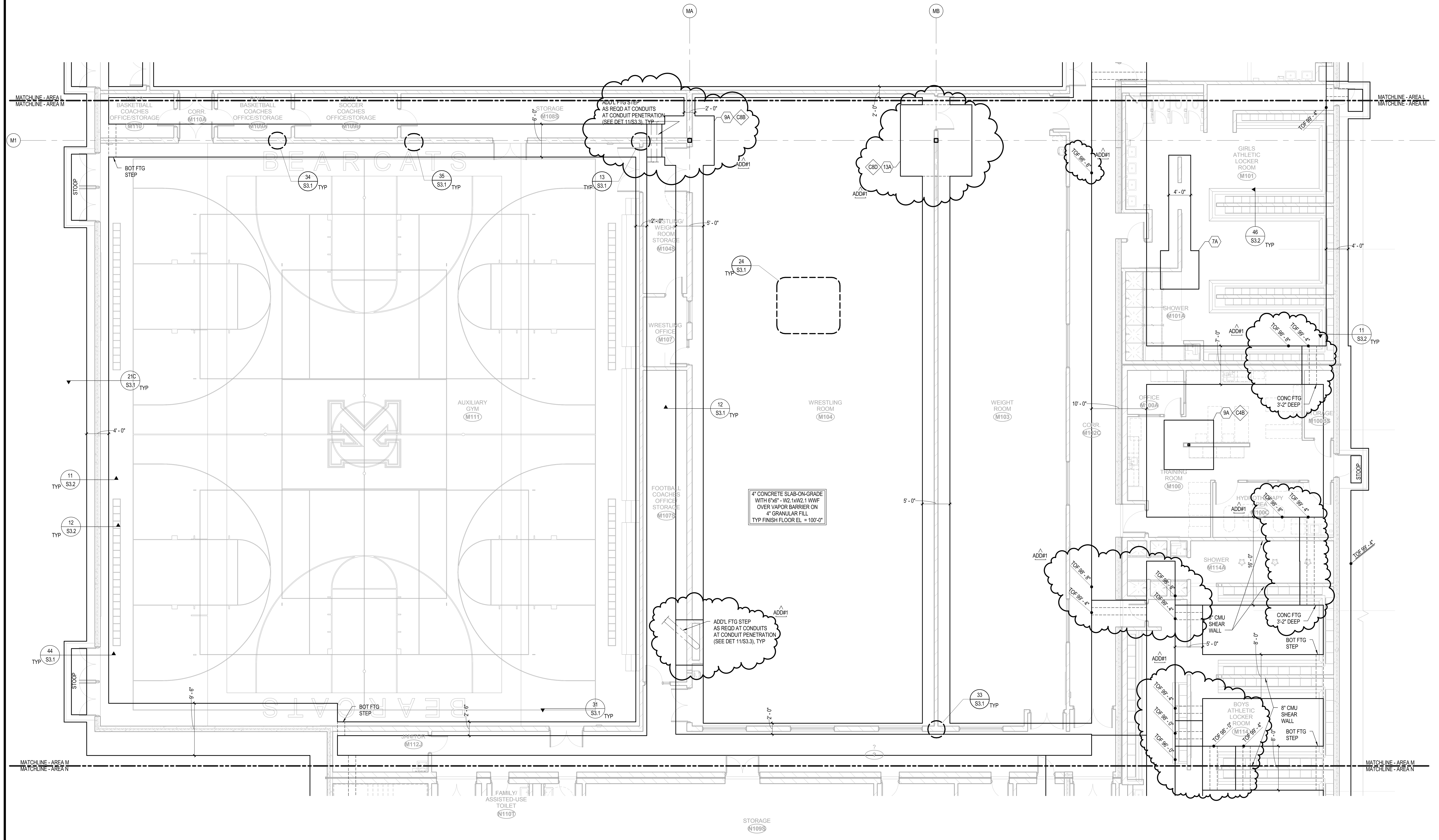
Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREA L

ProjectNumber: 1355
Date: September 2, 2014

Sheet Number:
S1.10

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.



FOUNDATION PLAN - AREA M
SCALE: 1/8" = 1'-0"

KEY PLAN

Wilkins Hinrichs Stober Architects L.L.C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5787 T
308-236-6929 F

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.11-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

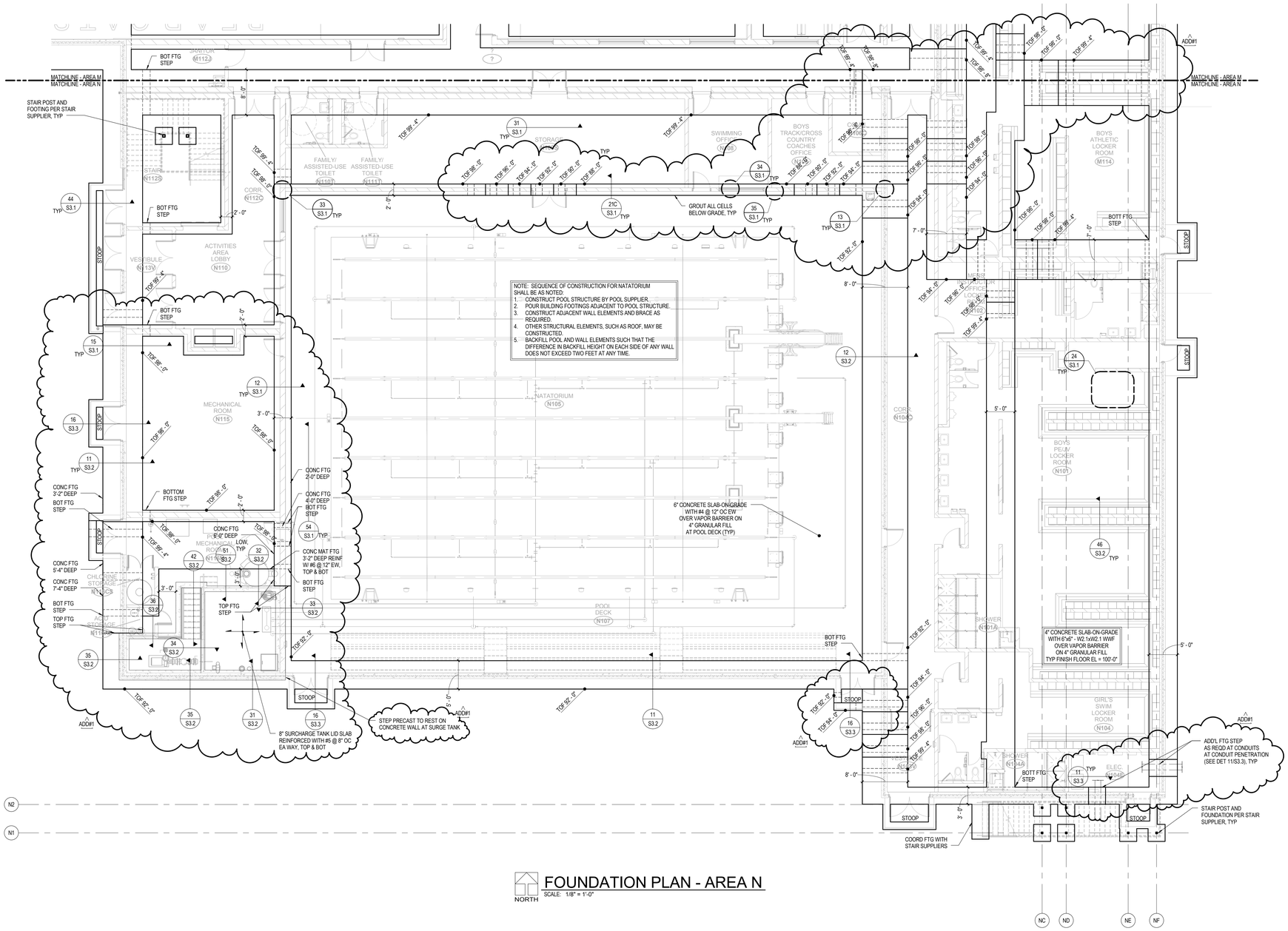
Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREA M

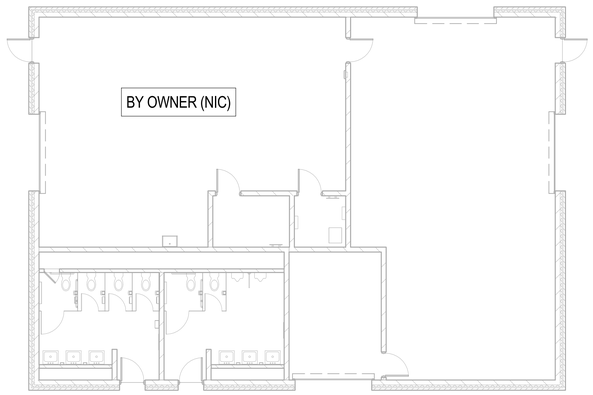
ProjectNumber: 1355
Date: September 2, 2014

Sheet Number:
S1.11

FOUNDATION PACKAGE
THESE DRAWINGS ARE ISSUED FOR FOUNDATIONS ONLY. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.

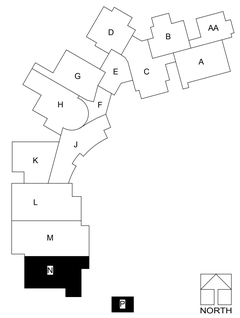


FOUNDATION PLAN - AREA N
NORTH
SCALE: 1/8" = 1'-0"



FOUNDATION PLAN - AREA P
NORTH
SCALE: 1/8" = 1'-0"

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S1.12-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

FOUNDATION PLAN - AREAS N & P

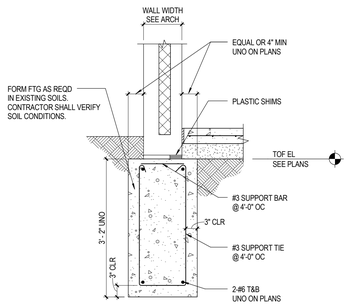
ProjectNumber: 1355
Date: September 2, 2014

Sheet Number:
S1.12

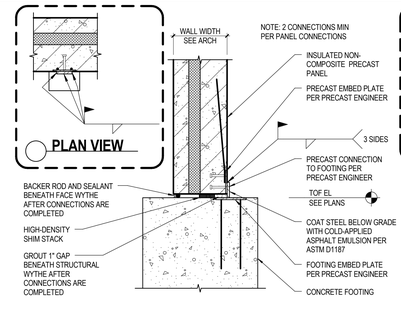
Structural Spread Footing Schedule

Mark	Footing Dimensions		Footing Thickness	Reinforcing
	Length	Width		
3A	3' - 0"	3' - 0"	2' - 0"	4-#6 EW BOTTOM
3B	3' - 0"	3' - 0"	3' - 2"	4-#6 EW BOTTOM
3D	3' - 0"	3' - 0"	4' - 0"	4-#6 EW BOTTOM
3E	3' - 6"	3' - 6"	3' - 2"	4-#6 EW BOTTOM
4A	4' - 0"	4' - 0"	2' - 0"	5-#6 EW BOTTOM
4B	4' - 0"	4' - 0"	3' - 2"	5-#6 EW BOTTOM
5A	5' - 0"	5' - 0"	2' - 0"	6-#6 EW BOTTOM
5B	5' - 0"	5' - 0"	3' - 2"	6-#6 EW TOP AND BOT
5C	5' - 0"	5' - 0"	2' - 6"	6-#7 EW BOTTOM
5D	5' - 0"	5' - 0"	5' - 4"	6-#7 EW TOP AND BOT
6A	6' - 0"	6' - 0"	2' - 0"	10-#8 EW BOTTOM
6B	6' - 0"	6' - 0"	3' - 2"	7-#7 EW TOP AND BOT
7A	7' - 0"	7' - 0"	2' - 0"	7-#7 EW BOTTOM
7B	7' - 0"	7' - 0"	3' - 2"	7-#7 EW TOP AND BOT
8A	8' - 0"	8' - 0"	2' - 0"	9-#8 EW BOTTOM
8B	8' - 0"	8' - 0"	3' - 2"	9-#8 EW TOP AND BOT
8C	8' - 0"	8' - 0"	4' - 0"	9-#8 EW TOP & BOT
8D	8' - 0"	8' - 0"	4' - 6"	9-#8 EW TOP AND BOT
8E	8' - 0"	8' - 0"	5' - 4"	9-#8 EW TOP & BOT
9A	9' - 0"	9' - 0"	2' - 0"	10-#8 EW BOTTOM
9B	9' - 0"	9' - 0"	3' - 2"	10-#8 EW TOP & BOT
10A	10' - 0"	10' - 0"	2' - 0"	12-#8 EW BOTTOM
11B	12' - 8"	11' - 4"	2' - 0"	12-#8 EW BOTTOM
12B	12' - 0"	5' - 0"	3' - 2"	13-#7 SW BOT, 6-#7 LW BOT
13A	13' - 0"	13' - 0"	2' - 6"	14-#8 EW BOTTOM
13B	13' - 0"	13' - 0"	4' - 0"	14-#8 EW TOP AND BOT

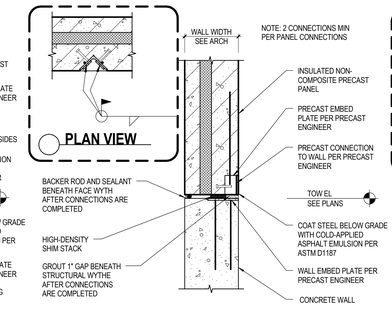
Attachment No. S3.1-1
to Addendum No. 1
Dated: September 12, 2014



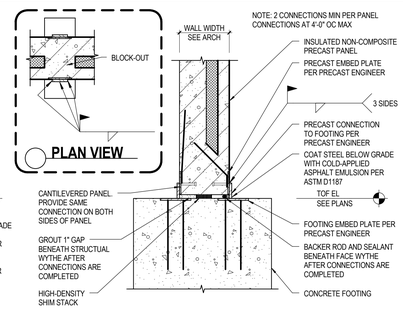
11 S3.2 TYP PRECAST WALL PANEL FTG
NO SCALE



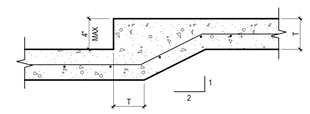
12 S3.2 TYP PRECAST WALL PANEL CONNECTION
NO SCALE (AT PRECAST PANEL)



13 S3.2 TYP PRECAST WALL PANEL CONNECTION
NO SCALE (AT PRECAST PANEL)

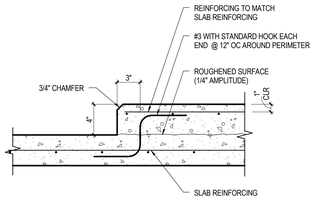


14 S3.2 TYP PRECAST WALL PANEL CONNECTION
NO SCALE (AT CANTILEVERED PANEL)



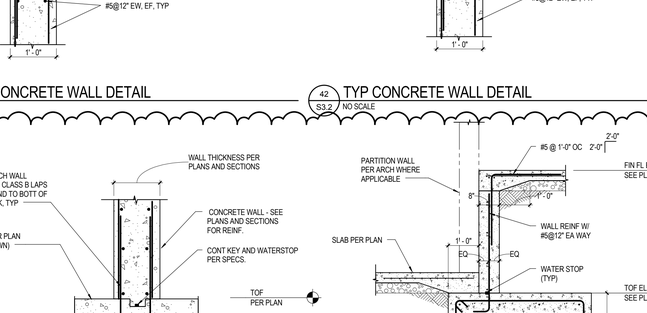
NOTE:
SEE ARCHITECTURAL PLANS FOR LOCATION OF RECESSED SLABS

15 S3.2 TYP FLOOR SLAB RECESS
NO SCALE

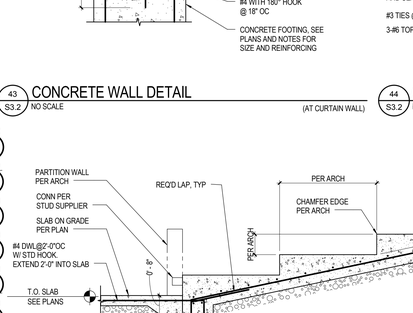


NOTE:
VERIFY SIZE AND LOCATION WITH MECHANICAL OR ELECTRICAL CONTRACTOR

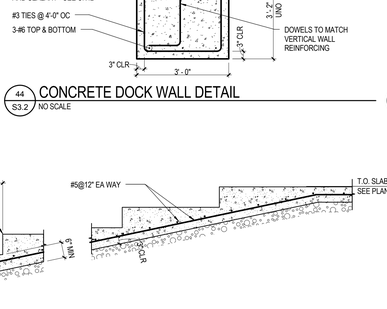
16 S3.2 TYP INTEGRAL EQUIPMENT PAD DETAIL
NO SCALE (AT SLAB-ON-GRADE OR STRUCTURAL FLOOR)



21 S3.2 ELEVATOR PIT DETAIL
SCALE: 1/2" = 1'-0"

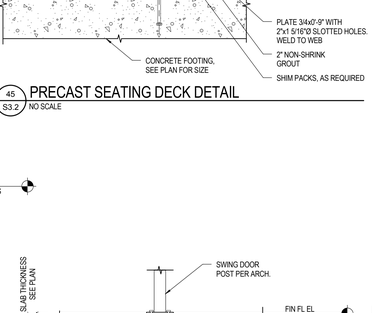


23 S3.2 TYP CONCRETE WALL REINFORCING DETAIL
NO SCALE (AT CONCRETE WALLS)



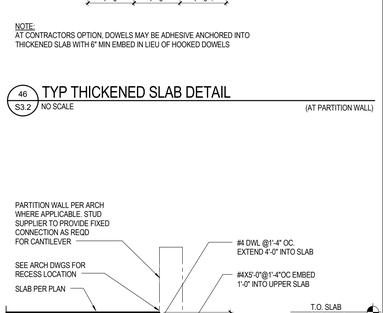
NOTE:
CORNER BARS TO BE SAME SIZE AND SPACING AS THE HORIZONTAL BARS.

24 S3.2 TYP CORNER REINFORCING DETAIL
NO SCALE (AT CONCRETE WALLS)



NOTE:
INTERSECTION BARS TO BE SAME SIZE AND SPACING AS THE HORIZONTAL BARS.

25 S3.2 TYP INTERSECTION REINFORCING DETAIL
NO SCALE (AT CONCRETE WALLS)



26 S3.2 TYP SLAB DETAIL
NO SCALE (AT OVERHEAD DOORS)

31 S3.2 TYP SURGE TANK LID DETAIL
NO SCALE

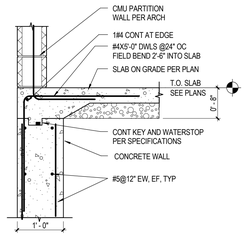
32 S3.2 TYP SURGE TANK LID DETAIL
NO SCALE

33 S3.2 TYP SURGE TANK LID DETAIL
NO SCALE (AT PRECAST PANEL)

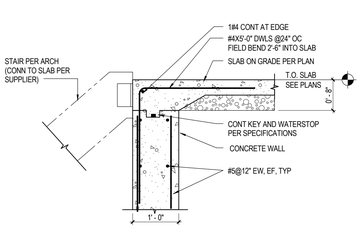
34 S3.2 TYP SURGE TANK LID DETAIL
NO SCALE

35 S3.2 TYP POOL MECH ROOM WALL
NO SCALE (AT PRECAST PANEL)

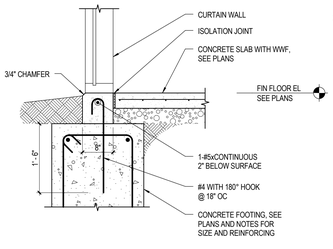
36 S3.2 TYP CONCRETE WALL DETAIL
NO SCALE



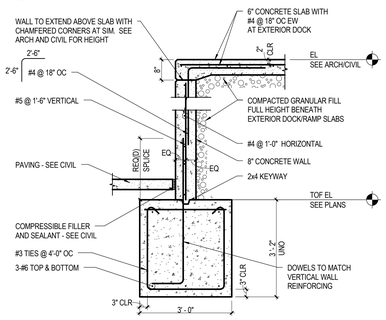
41 S3.2 TYP CONCRETE WALL DETAIL
NO SCALE



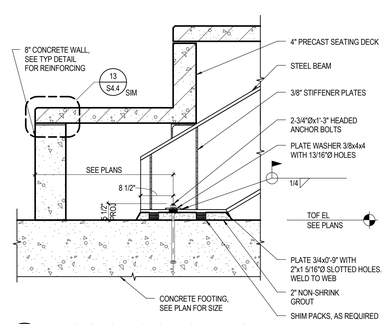
42 S3.2 TYP CONCRETE WALL DETAIL
NO SCALE



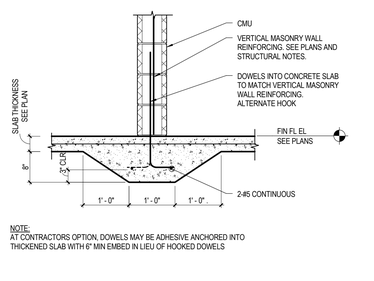
43 S3.2 CONCRETE WALL DETAIL
NO SCALE (AT CURTAIN WALL)



44 S3.2 CONCRETE DOCK WALL DETAIL
NO SCALE



45 S3.2 PRECAST SEATING DECK DETAIL
NO SCALE



NOTE:
AT CONTRACTORS OPTION, DOWELS MAY BE ADHESIVE ANCHORED INTO THICKENED SLAB WITH 6" MIN EMBED IN LIEU OF HOOKED DOWELS

46 S3.2 TYP THICKENED SLAB DETAIL
NO SCALE (AT PARTITION WALL)

51 S3.2 TYP WALL TO FTG DETAIL
NO SCALE

52 S3.2 TYP SEATING WALL DETAIL
NO SCALE

53 S3.2 TYP SEATING WALL DETAIL
NO SCALE

55 S3.2 TYP SLAB AT SWING DOOR POST
NO SCALE

56 S3.2 TYP SLAB STEP DETAIL
NO SCALE

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

FOUNDATION PACKAGE
THE DETAILS PERTAINING TO FOUNDATIONS ARE ISSUED FOR PERMIT AND CONSTRUCTION. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S3.2-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

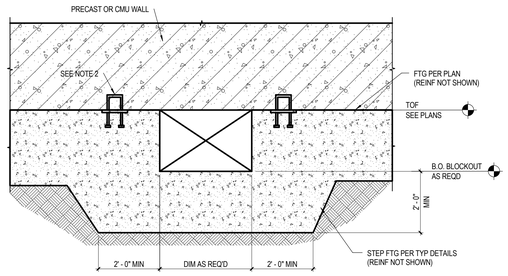
STRUCTURAL DETAILS

ProjectNumber: 1355
Date: September 2, 2014

Sheet Number:
S3.2

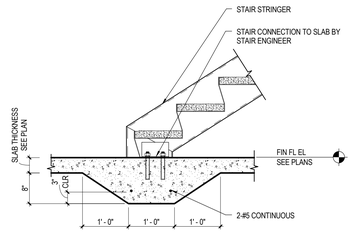
NOTES:

1. SEE ELECTRICAL DWGS FOR CONDUIT EXTENTS AND COORDINATE FOOTING STEPS AS REQUIRED.
2. AT PRECAST WALLS, PRECAST SUPPLIER SHALL DESIGN WALL TO SPAN OVER PENETRATION AND LOCATE ALL PRECAST CONNECTIONS AND EMBEDDED ITEMS OUTSIDE PENETRATION EXTENTS.
3. AT CMU WALLS, PROVIDE A LINTEL TO SPAN BLOCKOUT PER DET14/SS.2.
4. THIS DETAIL IS REQUIRED AT ALL LOCATIONS WHERE MULTIPLE CONDUITS MUST PENETRATE THROUGH FOOTING. APPROXIMATE LOCATIONS MAY BE SHOWN ON PLANS, HOWEVER CONTRACTOR IS REPONSIBLE FOR VERIFYING AND COORDINATING ALL LOCATIONS WHERE THIS IS NECESSARY, REGARDLESS OF WHETHER SHOWN ON STRUCTURAL FOUNDATION PLANS, PRIOR TO CONSTRUCTING FOUNDATIONS.

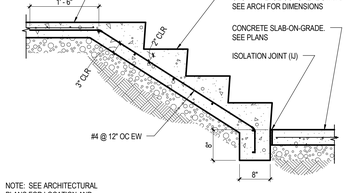


ADD#1

11 S3.3 TYP LARGE CONDUIT PENETRATION
NO SCALE

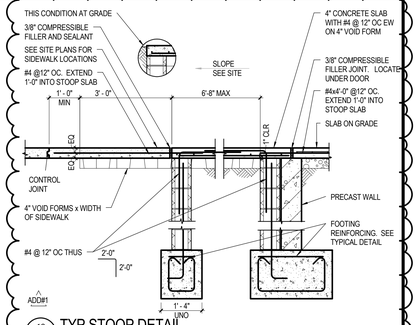


14 S3.3 TYP THICKENED SLAB DETAIL
NO SCALE (AT STAIR)



NOTE: SEE ARCHITECTURAL PLANS FOR LOCATION AND DIMENSIONS OF CONCRETE STEPS.

15 S3.3 TYP CONCRETE SLAB-ON-GRADE STEPS
NO SCALE



ADD#1

16 S3.3 TYP STOOP DETAIL
NO SCALE (AT PRECAST WALL)

FOUNDATION PACKAGE
THE HIGHLIGHTED DETAILS ARE ISSUED FOR PERMIT AND CONSTRUCTION. ALL OTHER ITEMS SHOWN ARE FOR REFERENCE AND WILL BE ISSUED FOR CONSTRUCTION IN A FUTURE BID PACKAGE.

Wilkins Hinrichs Stober Architects L. L. C.
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

PROFESSIONAL STRUCTURAL ENGINEER
JASON MCGINNELL
C-14275
9-25-14
STATE OF NEBRASKA

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Attachment No. S3.3-1
to Addendum No. 1
Dated: September 12, 2014

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD#1	ADDENDUM #1	9/12/2014

STRUCTURAL DETAILS

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:
S3.3

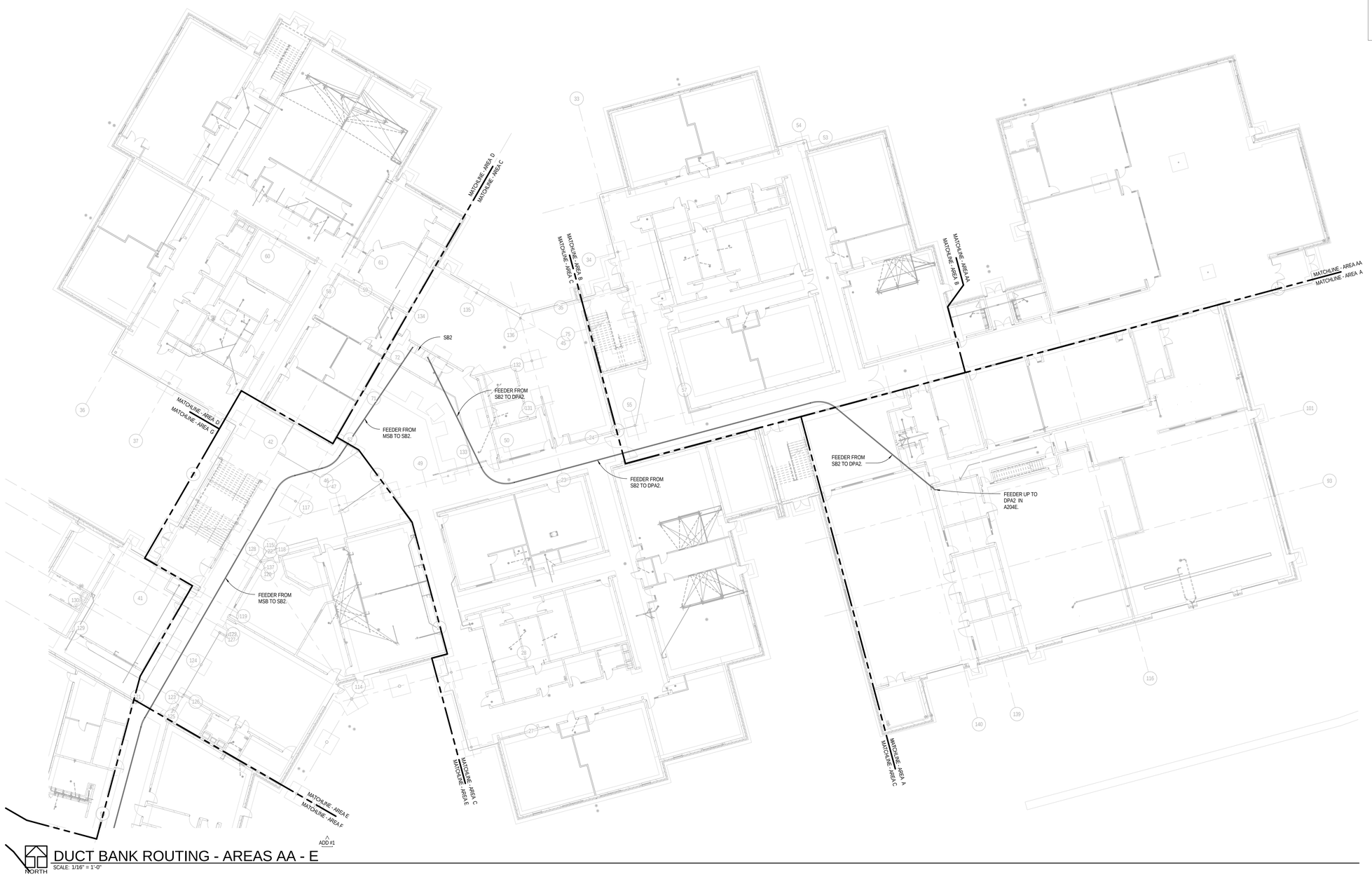
GENERAL NOTES

A. SEE SHEET E0.1 FOR ADDITIONAL GENERAL NOTES.

B. PLAN SHOWS PATHWAYS OF LARGER DUCTBANKS OF ELECTRICAL FEEDERS AND SERVICE ENTRANCE CONDUCTORS FOR THE PURPOSE OF COORDINATING STRUCTURAL FOOTINGS.

C. SEE POWER ONE-LINE DRAWINGS AND SCHEDULES ON SHEETS E5.1 THRU E5.3 FOR CONDUIT SIZES AND QUANTITIES.

D. NOT ALL UNDERGROUND CONDUITS ARE SHOWN. ELECTRICAL CONTRACTOR TO COORDINATE ALL LOCATIONS OF UNDERGROUND CONDUITS WITH FOOTINGS AND FOUNDATIONS CONTRACTOR. INSTALL SLEEVES PER SECTION 260500 IN FOOTINGS AND FOUNDATIONAL WALLS.

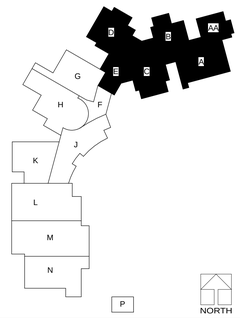


NORTH

DUCT BANK ROUTING - AREAS AA - E

SCALE: 1/16" = 1'-0"

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

PRELIMINARY DRAWINGS NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD #1	Addendum No. 1	9/12/2014

DUCTBANK ROUTING PLAN - AREAS AA - E

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

E2.29

GENERAL NOTES

A. SEE SHEET E0.1 FOR ADDITIONAL GENERAL NOTES.

B. PLAN SHOWS PATHWAYS OF LARGER DUCTBANKS OF ELECTRICAL FEEDERS AND SERVICE ENTRANCE CONDUCTORS FOR THE PURPOSE OF COORDINATING STRUCTURAL FOOTINGS.

C. SEE POWER ONE-LINE DRAWINGS AND SCHEDULES ON SHEETS E5.1 THRU E5.3 FOR CONDUIT SIZES AND QUANTITIES.

D. NOT ALL UNDERGROUND CONDUITS ARE SHOWN. ELECTRICAL CONTRACTOR TO COORDINATE ALL LOCATIONS OF UNDERGROUND CONDUITS WITH FOOTINGS AND FOUNDATIONS CONTRACTOR. INSTALL SLEEVES PER SECTION 260500 IN FOOTINGS AND FOUNDATIONAL WALLS.

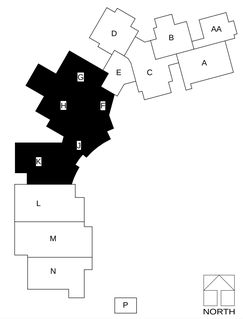
DUCT BANK ROUTING - AREAS F- K

ADD #1

SCALE: 1/16" = 1'-0"

NORTH

KEY PLAN



Wilkins Hinrichs Stober Architects L. L. C.

2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

PRELIMINARY DRAWINGS NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD #1	Addendum No. 1	9/12/2014

DUCTBANK ROUTING PLAN - AREAS F - K

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

E2.30

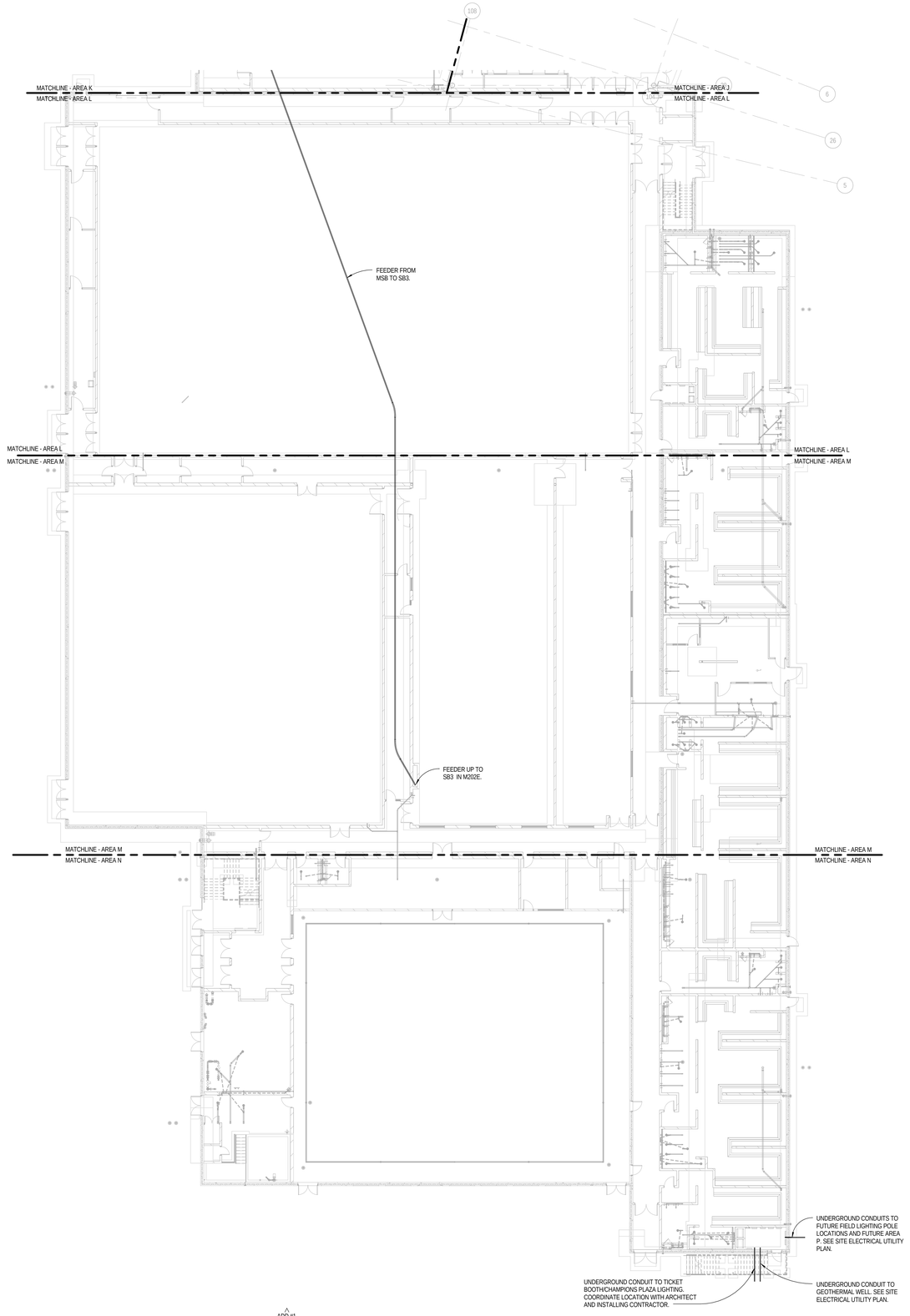
GENERAL NOTES

A. SEE SHEET E0.1 FOR ADDITIONAL GENERAL NOTES.

B. PLAN SHOWS PATHWAYS OF LARGER DUCTBANKS OF ELECTRICAL FEEDERS AND SERVICE ENTRANCE CONDUCTORS FOR THE PURPOSE OF COORDINATING STRUCTURAL FOOTINGS.

C. SEE POWER ONE-LINE DRAWINGS AND SCHEDULES ON SHEETS E5.1 THRU E5.3 FOR CONDUIT SIZES AND QUANTITIES.

D. NOT ALL UNDERGROUND CONDUITS ARE SHOWN. ELECTRICAL CONTRACTOR TO COORDINATE ALL LOCATIONS OF UNDERGROUND CONDUITS WITH FOOTINGS AND FOUNDATIONS CONTRACTOR. INSTALL SLEEVES PER SECTION 260500 IN FOOTINGS AND FOUNDATIONAL WALLS.



NORTH

DUCT BANK ROUTING - AREAS L - N ADD #1

SCALE: 1/16" = 1'-0"

Wilkins Hinrichs Stober Architects L. L. C.

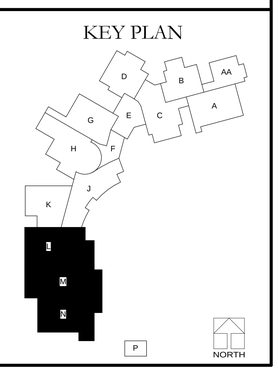
2908 West 39th Street
Suite A
Kearney, NE 68845
308-237-5767 T
308-236-6929 F

PRELIMINARY DRAWINGS NOT FOR CONSTRUCTION

DLR Group
Architecture
Engineering
Planning
Interiors

DLR Group Project No: 10-13113-00

KEY PLAN



Kearney Public Schools
New Kearney High School - Foundation Package
Kearney, Nebraska

Revision/Issue		Date
ADD #1	Addendum No. 1	9/12/2014

DUCTBANK ROUTING PLAN - AREAS L - N

ProjectNumber: **1355**
Date: **September 2, 2014**

Sheet Number:

E2.31