Sampson Construction Co., Inc.
3730 South 14th Street
Lincoln, NE 68502
Phone: (402) 434-5450
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Bid Bulletin #01

PROJECT: Skinner Early Childhood Center
Bid Package #2
Omaha, NE

DATE: September 10, 2014

This Bid Bulletin includes item 1-1 through 1-4. Each item shall be fully incorporated into the Bidding/Contract Documents and have the same force and effect as though originally included. Bidders shall acknowledge receipt of this Bid Bulletin on their bid.

Item 1-1 Anticipated start dates for the scopes of work bidding in bid package #2 are as follows:

- Foundations- 10/31/14
- CMU Masonry- 11/28/14
- Steel Erection- 12/5/14
- Building slabs- 1/14/15

Item 1-2 Bidders shall include winter protection/winter conditions as necessary for installation of their specific scope of work. Concrete additives, blankets, and heat for initial curing shall be included by the concrete bidders. Tenting and heat for masonry shall be included by the masonry bidders. If frost is present in soils, Sampson will provide ground thaw units for removal of frost prior to footing installation. Building enclosure and temporary building heat for completion of rough in and slab activities will be provided by Sampson.

Item 1-3 Concrete & reinforcing steel bidders shall provide pricing for Alternate #3 as noted in Addendum B.

Item 1-4 Attached is Addendum B from RDG Planning & Design dated 9/9/14. Please note Addendum A is applicable to previous bid package and does not apply to Bid Package #2.

END OF BID BULLETIN #01

September 9, 2014

ADDENDUM B

TO DRAWINGS AND SPECIFICATIONS FOR:

Structural Package
Skinner Early Childhood Center
Omaha, Nebraska
RDG Project No. 2014.128.00, File 35.1

RDG Planning & Design
900 Farnam on the Mall, Suite 100
Omaha, NE 68102-5089
(402) 392-0133

NOTICE TO BIDDERS

This Addendum modifies the original Contract Documents and Project Manual dated August 28, 2014. Bidders must acknowledge receipt of the Addendum on the Bid Form.

The following separate packages have been/will be issued:

Site Grading and Preparation Package	July 25, 2104
Structural Package	August 28, 2014
Final Construction Documents	To be Determined

Corresponding addenda:

Addendum A	August 6, 2014	Site Grading and Preparation Package
Addendum B	September 9, 2014	Structural Package

Unless otherwise indicated, the work described herein shall comply with, and be equal in all respect to, the original Contract Specifications and Drawings. Include all incidental work required to properly complete the work whether stated herein or not.

ADDENDA TO THE PROJECT MANUAL

Civil Specification Items

No items this addendum.

Architectural Specification Items

BGS-1 Section 01 23 00 – Alternates, Page 01 23 00-2, Paragraph 3.1: Add new Subparagraph C as follows:

C. Alternate No. 3 – Interior Concrete Footings:

1. Base Bid: Provide and construct interior concrete footings as defined within the Structural Drawings and Division 03 Section "Cast-in-Place Concrete."

2. Add Alternate: Under this alternate, Contractor has the option to increase the interior footing depths from 1'-4" to 3'-4" with a top layer of reinforcing matching the bottom layer of reinforcing to allow for cold weather construction.

BGS-2 Section 04 20 00 – Unit Masonry, Page 04 20 00-4, Paragraph 2.2, Subparagraph D, Item 3: Revise to read:

3. Size: 4"D x 8"H x 16"L nominal. Masonry units shall be manufactured to dimensions 3/8-inch less than nominal dimensions.

Structural Specification Items

BSS-1 Section 03 30 00 – Cast-In-Place Concrete, Page 03 30 00-1, Paragraph 1.4:

A. Renumber existing Subparagraph D to ***"Subparagraph E,"*** existing Subparagraph E to ***"Subparagraph F,"*** and do so for all remaining subparagraphs under Paragraph 1.4.

B. Insert new Paragraph D as follows for control and construction joint layout submittals as shown:

D. Control and Construction Joint Layouts: Placing drawings that detail layouts of slab-on-grade control and construction joints based upon requirements indicated on the construction drawings.

ADDENDA TO THE DRAWINGS

Civil Drawing Items

No items this addendum.

Architectural Drawing Items

No items this addendum.

Structural Drawing Items

BSD-1 Sheet S01.01: Add Plan Note #7 as follows:

7. AT CONTRACTOR'S OPTION, INTERIOR FOOTING DEPTHS MAY INCREASE FROM 1'-4" TO 3'-4" WITH A TOP LAYER OF REINFORCING MATCHING THE BOTTOM LAYER OF REINFORCING TO ALLOW FOR COLD WEATHER CONSTRUCTION.

BSD-2 Sheet S03.00:

A. Revise General Structural Note under Design Loads to read as follows:

LIVE LOADS = 25 PSF.

B. Revise General Structural Note C.5 as follows:

5. MECHANICALLY VIBRATE ALL CONCRETE INCLUDING SLABS ON GRADE.

C. Revise General Structural Note H.5 as follows:

5. WELD STEEL DECK TO STRUCTURAL STEEL MEMBERS WITH MINIMUM 5/8" DIAMETER PUDDLE WELDS AT EVERY DECK RIB AT DECK ENDS AND LAPS; FASTEN AT EVERY OTHER DECK RIB AT INTERMEDIATE LOCATIONS. #12 TEK SCREWS MAY BE USED IN LIEU OF WELDING AT THE CONTRACTOR'S OPTION. FASTEN STEEL DECK SIDELAPS WITH #10 TEK SCREWS, BUTTON PUNCH OR 1 1/2" SEAM WELD AT 24" ON CENTER.

BSD-3 Sheet S03.01:

A. Section 3 – PERIMETER WALL FOOTING SECTION: Revise track-to-stem wall connection as follows:

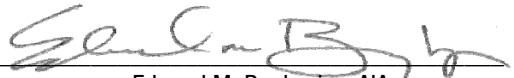
1 1/2" DEEP CONT. TRACK MATCHING STUD WIDTH AND GAUGE WITH TWO 1/4" TAPCON ANCHORS WITH 1 1/4" EMBEDMENT AT 32" O.C. AND 4" GAGE.

B. Section 8 – COOLING TOWER ENCLOSURE FOOTING SECTION: Revise track-to-stem wall connection as follows.

1 1/2" DEEP CONT. TRACK MATCHING STUD WIDTH AND GAUGE WITH TWO 1/4" TAPCON ANCHORS WITH 1 1/4" EMBEDMENT AT 32" O.C. AND 4" GAGE.

ALL OTHER REQUIREMENTS OF THE PLANS AND SPECIFICATIONS REMAIN IN EFFECT. THIS ADDENDUM SHALL BE ATTACHED AND MADE A PART OF THE PLANS AND SPECIFICATIONS.

RDG PLANNING & DESIGN

By 

Edward M. Buglewicz, AIA
for RDG Schutte Wilscam Birge, Inc.

END OF ADDENDUM B

EMB/jm

Enclosure: Revised Sheets S01.01, S03.00, and S03.01

COLUMN SCHEDULE	
MARK	SIZE
C1	HSS5x5x5/16

FOOTING SCHEDULE		
MARK	SIZE (LxWxD)	REINFORCING
F1	6'-0"x6'-0"x1'-4"	SEVEN #5 E.W. BOT.
F2	5'-0"x5'-0"x3'-4"	SIX #5 E.W. T&B
F3	5'-0"x5'-0"x1'-4"	SIX #5 E.W. BOT.
F4	4'-0"x4'-0"x3'-4"	FIVE #5 E.W. T&B
F5	4'-0"x4'-0"x1'-4"	FIVE #5 E.W. BOT.
WF1	CONT.x2'-0"x1'-4"	THREE #5 CONT. BOT.
WF2	CONT.x1'-4"x3'-4"	TWO #5 CONT. T&B WITH #4 TRANS. TIES AT 24" O.C.

NOTES:
1. BOT. DENOTES BOTTOM, CONT. DENOTES CONTINUOUS, E.W. DENOTES EACH WAY, O.C. DENOTES ON CENTER, T&B DENOTES TOP AND BOTTOM AND TRANS. DENOTES TRANSVERSE.

2. PROVIDE VERTICAL DOWELS FOR WALLS PER SECTIONS ON SHEET S03.01.

DRILLED PIER SCHEDULE		
MARK	SIZE (DIAxD)	REINFORCING
DP1	1'-0"x7'-4"	FOUR #4 VERT. AND #4 HORIZ. TIES AT 12" O.C.

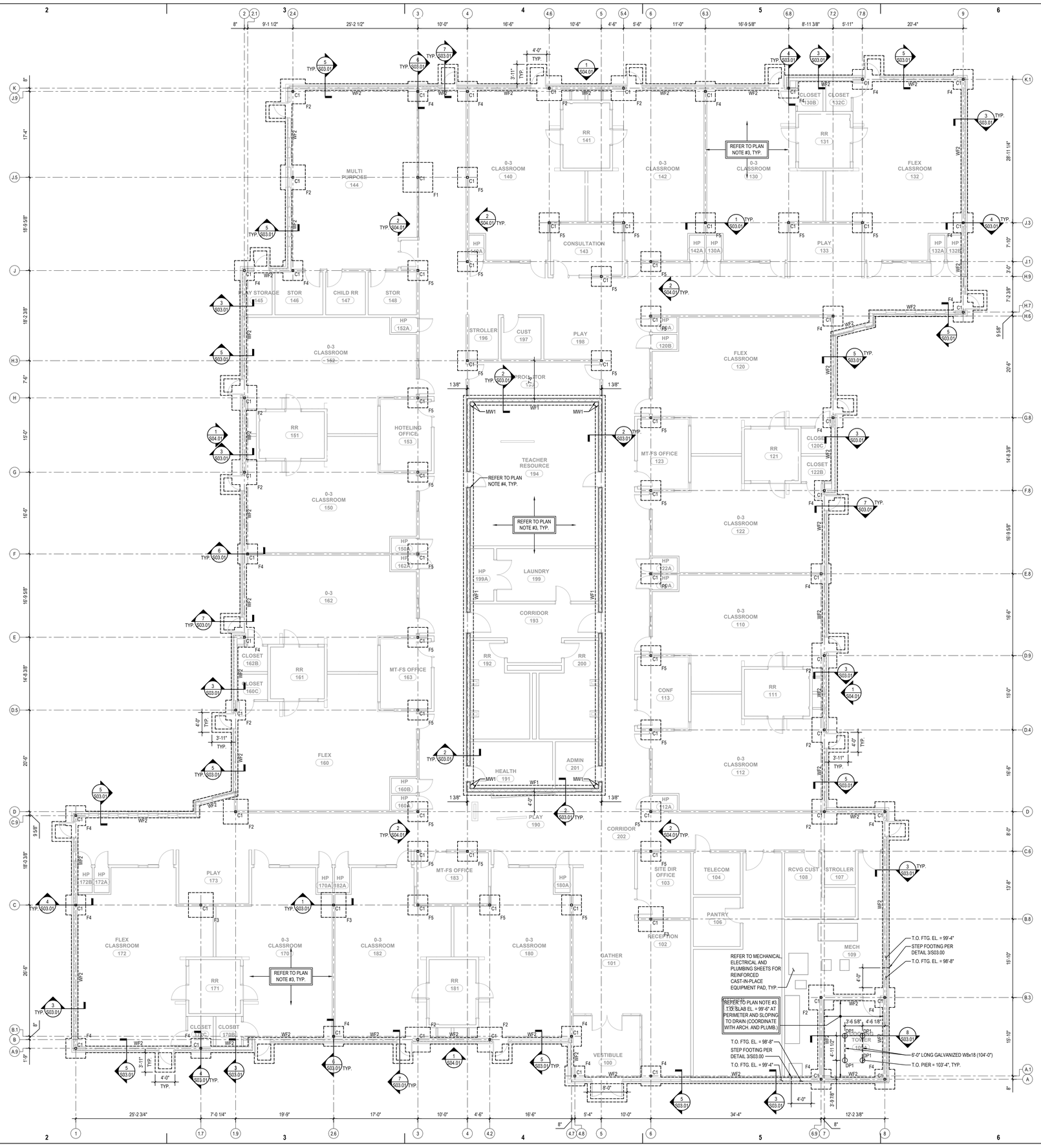
NOTES:
1. HORIZ. DENOTES HORIZONTAL, O.C. DENOTES ON CENTER AND VERT. DENOTES VERTICAL.

2. PROVIDE THREE SETS OF #4 HORIZ. TIES AT 3" O.C. WITHIN THE TOP 8" OF EACH PIER.

CMU WALL SCHEDULE		
MARK	CMU SIZE	REINFORCING
MW1	8"	#4 VERT. AT 8" O.C.

NOTES:
1. O.C. DENOTES ON CENTER AND VERT. DENOTES VERTICAL.

2. REFER TO DETAIL 4/S03.00 FOR REINFORCING DEVELOPMENT LENGTHS, Ssplice LENGTHS AND POSITION REQUIREMENTS.



1 FOUNDATION PLAN

1/8" = 1'-0"

PLAN NOTES:
1. MATCH ARCHITECTURAL FINISHED FLOOR ELEVATION OF 100'-0" TO CORRESPONDING CIVIL ELEVATION OF 1069.26. VERIFY WITH ARCHITECTURAL AND CIVIL SHEETS.

2. TOP OF FOOTING ELEVATION IS 99'-4", UNLESS NOTED OTHERWISE.

3. 4" THICK NORMAL WEIGHT CAST-IN-PLACE CONCRETE SLAB ON GRADE WITH 6x6-W1.4xW1.4 WWF REINFORCING ON 15 MIL VAPOR BARRIER AND 4" GRANULAR FILL. TOP OF SLAB ELEVATION IS 100'-0, UNLESS NOTED OTHERWISE. PROVIDE FLOOR SLAB CONTROL AND CONSTRUCTION JOINTS PER DETAIL 2/S03.00 AT SPACINGS LISTED ON THE GENERAL STRUCTURAL NOTES. COORDINATE FLOOR SLAB CONTROL AND CONSTRUCTION JOINTS AT EXPOSED CONCRETE AREAS WITH ARCHITECTURAL FLOOR FINISH PLAN.

4. PROVIDE LINTEL AND JAMB REINFORCING IN MASONRY WALL OPENINGS PER DETAIL 5/S03.00.

5. PROVIDE SLEEVES THROUGH CAST-IN-PLACE CONCRETE PERIMETER FOOTINGS AS REQUIRED PER MECHANICAL AND PLUMBING SHEETS. DO NOT ALLOW SLEEVES TO INTERFERE WITH REINFORCING AND PROVIDE MINIMUM CLEARANCE OF 3" TO ALL REINFORCING.

6. REFER TO SECTION 8/S03.01 FOR REINFORCED CAST-IN-PLACE CONCRETE FOOTINGS AND REINFORCED MASONRY WALLS AT DUMPSTER ENCLOSURE.

△ 7. AT CONTRACTOR'S OPTION, INTERIOR FOOTING DEPTHS MAY INCREASE FROM 1'-4" TO 3'-4" WITH A TOP LAYER OF REINFORCING MATCHING THE BOTTOM LAYER OF REINFORCING TO ALLOW FOR COLD WEATHER CONSTRUCTION.

PERFORMANCE Engineering
4940 N. 174th Street, Suite 200 - Omaha, NE 68164 | 7400 E. Orchard Rd, Suite 240 - Englewood CO 80111
(402) 330-2600 Fax (402) 330-2664 | (303) 721-1322 Fax (303) 721-1064
PE #: 14043A37

Structural Package

RDG:: PLANNING + DESIGN

ARCHITECT
RDG Planning & Design
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STRUCTURAL
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MECH/PLUMB/ELEC/TELE
Alvine Engineering
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FOOD SERVICE EQUIPMENT
Starn Consulting, Inc.
3030 Southwest Blvd Place
Tulsa, OK 74135
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LANDSCAPE ARCHITECT
RDG Planning & Design
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PROFESSIONAL STRUCTURAL ENGINEER — RYAN P. PELESHA — E-10430 — 9/26/14 — STATE OF NEBRASKA

SKINNER EARLY CHILDHOOD CENTER

4304 North 33rd Street
Omaha, NE 68107

KEY PLAN

REV	DATE	DESCRIPTION
1	9/5/2014	ADDENDUM #1

ISSUED: AUG 28, 2014
PROJECT NO: 2014.128.00

FOUNDATION PLAN

S01.01

GENERAL STRUCTURAL NOTES:

A. DESIGN DATA

DESIGN CODE:	2006 IBC
CONCRETE 28-DAY STRENGTH:	F'C = 4,000 PSI
STRUCTURAL STEEL (WIDE FLANGE SECTIONS)	ASTM A992
STRUCTURAL TUBES	ASTM A500 GRADE B
MISCELLANEOUS ROLLED SECTIONS (ANGLES, CHANNELS, PLATES, ETC.)	ASTM A36
HIGH STRENGTH BOLTS	ASTM A325
PLAIN BOLTS AND ANCHORS	ASTM F1554 GRADE 36
REINFORCING STEEL	ASTM A615 FY = 60,000 PSI
WELDED WIRE FABRIC	ASTM A185
CONCRETE MASONRY UNITS (GRADE N TYPE I)	F'M = 1,500 PSI
MORTAR (TYPE M OR S) AND GROUT 28-DAY STRENGTH	F'C = 2,000 PSI
ALLOWABLE SOIL BEARING CAPACITY	2,000 PSF

DESIGN LOADS

GRAVITY LOADS:

ROOF
- DEAD LOAD = 20 PSF
- STORM SHELTER DEAD LOAD = 15 PSF (COLLATERAL) + 12.5 PSF (PER 1" THICKNESS OF TOPPING) + PRECAST MEMBER SELF WEIGHTS
- ⚠1 LIVE LOAD = 25 PSF
- FLAT ROOF SNOW LOAD = 21 PSF*
- (BASED UPON GROUND SNOW LOAD OF 25 PSF, Ce=1.0, Ct=1.0, AND I=1.0)

*INCREASE FLAT ROOF SNOW LOAD FOR SNOW DRIFTING AS REQUIRED IN CONFORMANCE WITH THE AMERICAN SOCIETY OF CIVIL ENGINEERS ANSI/ASCE 7-05.

SLABS ON GRADE
- DEAD LOAD = 30 PSF (SUPERIMPOSED)
- LIVE LOAD = 100 PSF

LATERAL LOADS:

WIND LOADING CRITERIA
- BASE WIND SPEED (3 SECOND GUST) 'V' = 90 MPH
- BUILDING CATEGORY = II
- IMPORTANCE FACTOR 'Iw' = 1.0
- EXPOSURE CATEGORY = C
- MEAN ROOF HEIGHT = 12.5 FEET

SEISMIC LOADING CRITERIA
- SPECTRAL RESPONSE COEFFICIENT SDS = 0.114g
- SPECTRAL RESPONSE COEFFICIENT SD1 = 0.069g
- IMPORTANCE FACTOR 'Ie' = 1.0
- SEISMIC USE GROUP I
- SITE CLASS D
- SEISMIC DESIGN CATEGORY A
- SEISMIC FORCE RESISTING SYSTEMS = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES, ORDINARY REINFORCED CONCRETE MASONRY WALLS AND ORDINARY STEEL MOMENT FRAMES.

B. FOUNDATION WORK:

1. THE GEOTECHNICAL REPORT PREPARED BY TERRACON CONSULTANTS, INC. (PROJECT NO. 05145049), DATED JULY 11, 2014 IS AVAILABLE AND SHALL BE REVIEWED BY THE CONTRACTOR. SEE REPORT AND SPECIFICATIONS FOR OVEREXCAVATION AND RECOMPACTION REQUIREMENTS.
2. SUBSOILS SUPPORTING OR IN DIRECT CONTACT WITH FOOTINGS, SLABS-ON-GRADE, OR OTHER FOUNDATION ELEMENTS SHALL BE PROTECTED AGAINST FREEZING CONDITIONS THAT COULD CAUSE MOVEMENT OR OTHER DETRIMENTAL EFFECT TO THE STRUCTURE AS A WHOLE OR TO ANY OF ITS COMPONENT PARTS.
3. WHEN WORKING NEAR EXISTING AND/OR NEW CONSTRUCTION, THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION SO AS NOT TO UNDERMINE, DISTURB, DAMAGE OR, IN ANY WAY, CAUSE UNDESIRABLE MOVEMENT, CRACKING, AND/OR SETTLEMENT OF THE ADJACENT CONSTRUCTION.
4. ALL SLABS ON GRADE SHALL BEAR ON UNDISTURBED VIRGIN SOIL OR PROPERLY COMPACTED BACKFILL/GRANULAR FILL. ANY UNACCEPTABLE UNDISTURBED VIRGIN SOIL OR BACKFILL/GRANULAR FILL, AS DETERMINED BY THE OWNER'S GEOTECHNICAL ENGINEER, SHALL BE REMOVED AND REPLACED AS REQUIRED BY THE GEOTECHNICAL ENGINEER.
5. CONTRACTOR SHALL COORDINATE FOOTING ELEVATIONS WITH FINAL GRADING PLAN TO PROVIDE A MINIMUM OF 42" OF GRADE ABOVE THE BOTTOM OF ALL FOOTINGS FOR FROST PROTECTION.

C. CONCRETE:

1. FOR REINFORCING DEVELOPMENT LENGTH AND SPLICE LENGTH SEE TYPICAL REINFORCING TABLE ON THIS SHEET.
2. PROVIDE CORNER BARS IN WALLS AND FOOTINGS THE SAME SIZE AND NUMBER AS THE CONTINUOUS REINFORCING.
3. REINFORCING IN FOOTINGS SHALL BE ACCURATELY PLACED BEFORE PLACING CONCRETE. DO NOT FLOAT REINFORCING INTO FOOTINGS.
4. CONCRETE SHALL BE REGULAR WEIGHT (144 PCF) WITH TYPE I CEMENT, POTABLE WATER, AND AGGREGATES CONFORMING TO REQUIREMENTS OF ASTM C33. CONCRETE SHALL CONFORM TO ACI 301-99.
5. MECHANICALLY VIBRATE ALL CONCRETE INCLUDING SLABS ON GRADE. ⚠
6. DO NOT PLACE PIPES, DUCTS, OR CHASES IN STRUCTURAL CONCRETE WITHOUT APPROVAL OF THE ARCHITECT/ENGINEER. SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATIONS.
7. CONSTRUCT FORMWORK SO CONCRETE MEMBERS AND STRUCTURES ARE OF SIZE, SHAPE, ALIGNMENT, ELEVATION, AND POSITION INDICATED, WITHIN TOLERANCE LIMITS OF ACI 117.
8. FINISH CONCRETE SUSPENDED SLABS AND SLABS-ON-GRADE PER THE FOLLOWING CRITERIA ACCORDING TO ASTM E 1155. COMPLY WITH ACI 302.1R RECOMMENDATIONS FOR SCREEDING, RESTRAIGHTENING, AND FINISHING OPERATIONS FOR CONCRETE SURFACES. DO NOT WET CONCRETE SURFACES.
   A. SLABS-ON-GRADE - SPECIFIED OVERALL VALUES OF FLATNESS, F(F) 35; AND OF LEVELNESS, F(L) 25; WITH MINIMUM LOCAL VALUES OF FLATNESS, F(F) 24; AND OF LEVELNESS, F(L) 17
   B. SUSPENDED SLABS - SPECIFIED OVERALL VALUES OF FLATNESS, F(F) 30; AND OF LEVELNESS, F(L) 20; WITH MINIMUM LOCAL VALUES OF FLATNESS, F(F) 24; AND OF LEVELNESS, F(L) 15
9. CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE PLACED AT COLUMN-LINE INTERSECTIONS AND AS NECESSARY TO NOT EXCEED A SPACING OF 36 TIMES THE SLAB THICKNESS. MAXIMUM ASPECT RATIO SHALL BE 1.5 TO 1.0 UNLESS NOTED OTHERWISE.
10. ALL CONSTRUCTION JOINTS IN CONCRETE WALLS SHALL HAVE A 2"x4" CONTINUOUS KEYWAY. ALL CONSTRUCTION JOINTS, EXCEPT THOSE DETAILED, SHALL HAVE ARCHITECT/ENGINEER APPROVAL. SEE SPECIFICATIONS FOR OTHER CONSTRUCTION JOINT REQUIREMENTS.
11. ALL REINFORCING STEEL SHALL BE DEFORMED NEW BILLETS BARS (A615, GRADE 60), BENT COLD, AND DETAILED, FABRICATED, AND HELD IN PLACE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI 315 - LATEST EDITION) EXCEPT AS OTHERWISE DETAILED OR SPECIFIED.
12. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING, UNLESS NOTED OTHERWISE:
    - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
    - CONCRETE EXPOSED TO EARTH OR WEATHER: 2"
    - CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      - SLABS AND WALLS: 1"
      - BEAMS AND COLUMNS: 2"
13. UNLESS NOTED OTHERWISE, SLABS ON GRADE SHALL BE 4" CONCRETE REINFORCED WITH 6x6-W1.4xW1.4 WELDED WIRE FABRIC WITH VAPOR BARRIER. AT THE CONTRACTOR'S OPTION, SLABS MAY BE REINFORCED WITH 1.5 POUNDS PER CUBIC YARD OF SYNTHETIC FIBERMESH REINFORCING.
14. ALL REINFORCING IN SLABS AND WALLS SHALL BE CONTINUOUS UNLESS DETAILED OTHERWISE AND LAP SPLICED ONLY IN REGIONS OF LOW STRESS. ALL BARS SHALL HAVE A STANDARD HOOK WHERE A HOOK IS SHOWN, UNLESS DETAILED OTHERWISE.

D. PRESTRESSED AND PRECAST CONCRETE:

1. DESIGN, FABRICATE, TRANSPORT, AND ERECT PER LATEST A.C.I. AND P.C.I. CODES AND HANDBOOK.
2. DESIGN FOR LOADS, IN ADDITION TO MEMBER WEIGHT, AS GIVEN UNDER (DESIGN LOADS) ABOVE.
3. SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS. FABRICATE AFTER THE ARCHITECT/ENGINEER REVIEW.
4. REINFORCE ENDS OF MEMBERS TO PREVENT CRACKING DUE TO VOLUME CHANGES.
5. CAMBER PER MANUFACTURER'S RECOMMENDATIONS, SUBJECT TO APPROVAL BY THE ARCHITECT / ENGINEER.
6. DETAILS SHOWING REINFORCING ARE FOR IN-PLACE CONDITIONS, THE CONTRACTOR IS RESPONSIBLE FOR PICK-UP POINT LOCATIONS AND INSERTS, SPECIAL PICK-UP REINFORCING AND STRONG BACKS, AND ALL PICK-UP AND PLACING OPERATIONS.
7. ALL PRECAST CONNECTIONS AND EMBEDMENTS SHALL BE DESIGNED AND DETAILED BY THE PRECAST MANUFACTURER. VERIFY LOCATION, SIZE, AND WEIGHT OF ALL EQUIPMENT WITH EQUIPMENT MANUFACTURER.

E. MASONRY:

1. FURNISH AND CONSTRUCT MASONRY IN ACCORDANCE WITH THE REQUIREMENTS OF THE SPECIFICATIONS FOR MASONRY CONSTRUCTION (ACI 530.1-02/ASCE 6-02/TMS 602-02).
2. LAY MASONRY UNITS IN RUNNING BOND, UNLESS NOTED OTHERWISE.
3. MAXIMUM GROUT LIFT WITHOUT CLEANOUTS SHALL BE 4'-0".
4. CMU WALLS PROVIDE CONTINUOUS FULL HEIGHT VERTICAL REINFORCING IN CENTER OF GROUT AT CENTER OF WALL. TYPICAL REINFORCING SHALL BE 1-#5 AT 48" ON CENTER AND 1-#5 AT CORNERS, INTERSECTIONS, WALL ENDS, DOOR AND WINDOW JAMBS, AND SIDE OF EXPANSION OR CONTROL JOINTS UNLESS NOTED OTHERWISE.
5. PROVIDE LADDER TYPE #9 JOINT REINFORCING AT 16" ON CENTER VERTICAL SPACING IN ALL CONCRETE MASONRY AND UNLESS NOTED OTHERWISE.
6. GROUT CELLS FULL AT ALL ADHESIVE AND EXPANSION ANCHOR LOCATIONS.
7. PROVIDE CONTINUOUS BOND BEAMS AT THE TOP OF ALL WALLS.
8. PROVIDE CONTINUOUS WIRE LATHE GROUT BARRIERS BELOW BOND BEAMS.
9. PROVIDE LINTELS OVER ALL OPENINGS AND RECESSES IN MASONRY WALLS.
10. FOR ALL OPENINGS NOT OTHERWISE DETAILED OR SCHEDULED, MINIMUM LINTELS SHALL BE FOR EACH 4 INCH OF MASONRY WIDTH ONE L3-1/2x3-1/2x1/4 FOR SPANS UP TO 4'-0", ONE L4x3-1/2x1/4 FOR SPANS UP TO 6'-0" AND ONE L5x3-1/2x1/4 FOR SPANS UP TO 8'-0". FOR SPANS LESS THAN 2'-0" PROVIDE A 5/16" PLATE.
11. ALL LINTELS SHALL HAVE A MINIMUM BEARING OF 8 INCHES EACH END.

F. STEEL:

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST AISC SPECIFICATIONS AND OSHA REGULATION 29 CFR PART 1926.
2. ALL STEEL BEAMS BEARING ON MASONRY SHALL HAVE A MINIMUM OF 8" OF BEARING. PROVIDE THE BEAMS WITH BEARING PLATES AND WALL ANCHORS UNLESS NOTED OTHERWISE. PROVIDE A MINIMUM OF 4 COURSES OF BRICK OR SOLID CONCRETE MASONRY FOR BEAM BEARING.
3. SHOP PAINT STRUCTURAL STEEL WITH FABRICATOR'S STANDARD LEAD- AND CHROMATE-FREE, NONASPHALTIC, RUST-INHIBITING PRIMER, U.N.O.
4. COMPLY WITH AMERICAN WELDING SOCIETY STANDARDS. ALL WELDERS SHALL HAVE VALID CERTIFICATES AND HAVE CURRENT EXPERIENCE IN TYPE OF WELD CALLED FOR.
5. WELDING ELECTRODES SHALL BE E70 FOR ALL STEEL, UNLESS OTHERWISE NOTED.
6. BOLTED CONNECTIONS SHALL BE SUPPLIED AND INSTALLED PER THE REQUIREMENTS OF RCSC'S "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS." SHOP AND FIELD BOLTED CONNECTIONS SHALL BE BEARING CONNECTIONS, U.N.O.
7. ANCHOR BOLTS FOR COLUMN BASE PLATES SHALL BE SECURED IN PLACE WITH A TEMPLATE AND SECURELY TIED TO REINFORCING BARS BEFORE PLACING OF CONCRETE.

G. STEEL JOISTS:

1. DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE SPECIFICATION BY A MEMBER OF THE SJI APPROVED FOR THE TYPE OF JOIST BEING USED. IN OF THE ABOVE REQUIREMENTS, THE FABRICATOR MAY PROVIDE AN ICBO RESEARCH RECOMMENDATION APPROVING THE TYPE OF BEING USED. STEEL JOISTS SHALL BE ERECTED IN ACCORDANCE WITH OSHA REGULATION 29 CFR PART 1926, EXCEPT FOR STEEL JOISTS THAT ARE PRE-ASSEMBLED INTO PANELS, CONNECTIONS OF INDIVIDUAL STEEL JOISTS TO STEEL STRUCTURES IN BAYS OF 40 FEET OR MORE SHALL BE FIELD BOLTED.
2. BRIDGING PER SJI SPECIFICATIONS AND DRAWINGS. WHERE BRIDGING INTERFERES WITH MECHANICAL OR OTHER INSTALLATION, REMOVE BRIDGING AFTER THE DECK IS IN PLACE AND REPLACE AS DIRECTED BY THE STRUCTURAL ENGINEER.
3. CONNECTIONS AND BEARING MATERIAL REQUIRED BY SJI SPECIFICATIONS, BUT NOT CALLED OUT, SHALL BE DESIGNED AND FURNISHED BY THE JOIST FABRICATOR.
4. ALL JOIST HEADERS AND ACCESSORIES SHALL BE DESIGNED AND FURNISHED BY THE JOIST MANUFACTURER.
5. BRIDGING FOR THE STANDARD JOISTS SHALL BE DESIGNED IN ACCORDANCE WITH SJI SPECIFICATIONS.
6. CONTRACTOR SHALL COORDINATE LOCATIONS AND MAGNITUDES OF LOADS WITH RESPECTIVE EQUIPMENT SUPPLIER OR AS INDICATED ON FRAMING PLANS. JOIST MANUFACTURER SHALL DESIGN JOIST FOR ADDITIONAL LOADS FROM EQUIPMENT.

H. STEEL DECK:

1. MATERIAL, DESIGN, MANUFACTURE AND INSTALLATION OF METAL DECKING SHALL BE FURNISHED BY A MEMBER OF THE STEEL DECK INSTITUTE.
2. PROVIDE ALL NECESSARY DETAILS SUCH AS RIDGE OR VALLEY SPLICE PLATES.
3. THE DECK MANUFACTURER SHALL FURNISH CLOSURE PLATES, SUMP PANS AND DRAIN PLATES, RIDGE AND VALLEY PLATES, WELDING WASHERS AND OTHER ACCESSORIES REQUIRED TO PROVIDE A FINISHED SURFACE FOR APPLICATION OF INSULATION AND ROOFING.
4. PROVIDE L3x3x1/4 ANGLE FRAMING AROUND ALL ROOF PENETRATIONS AND AS REQUIRED FOR SUPPORT OF ROOF CURBS TO STIFFEN METAL DECK EDGES.
5. WELD STEEL DECK TO STRUCTURAL STEEL MEMBERS WITH MINIMUM 5/8" DIAMETER PUDDLE WELDS AT EVERY DECK RIB AT DECK ENDS AND LAPS; FASTEN AT EVERY OTHER DECK RIB AT INTERMEDIATE LOCATIONS. #12 TEK SCREWS MAY BE USED IN LIEU OF WELDING AT THE CONTRACTOR'S OPTION. FASTEN STEEL DECK SIDELAPS WITH #10 TEK SCREWS, BUTTON PUNCH OR 1 1/2" SEAM WELD AT 24" ON CENTER.
6. ANCHOR CONNECTIONS TO THE UNDERSIDE OF STEEL FLOOR DECKS SHALL BE PLACED ALONG THE CENTER OF DECK RIBS.

J. LIGHT GAUGE METAL:

1. LIGHT GAUGE METAL FRAMING SHALL BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE LATEST EDITION OF "THE NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND "THE COLD FORMED STEEL FRAMING DESIGN GUIDE" OF THE AMERICAN IRON AND STEEL INSTITUTE.
2. PROVIDE TRACKS, BLOCKING, LINTELS, CLIP ANGLES, STRAP BRACING, SHOES, REINFORCEMENTS, FASTENERS, AND ACCESSORIES TO PROVIDE A COMPLETE METAL FRAME SYSTEM.
3. ISOLATE PARTITIONS FROM STRUCTURAL ELEMENTS WITH SLIP OR CUSHION-TYPE JOINTS BETWEEN STEEL FRAMING AND STRUCTURE AS RECOMMENDED BY STEEL FRAMING MANUFACTURER TO PREVENT TRANSFER OF STRUCTURAL LOADS OR MOVEMENTS TO PARTITIONS.
4. ATTACH SIMILAR MATERIALS BY WELDING. (IE TRACK TO STUD, STUD TO STRUCTURAL STEEL). ATTACH DISSIMILAR MATERIALS BY BOLTING OR SCREW FASTENERS.
5. INSTALL HORIZONTAL STIFFENERS IN STUD SYSTEM, SPACED (VERTICAL DISTANCE) AT NOT MORE THAN 3'-4" O.C. WELD AT EACH INTERSECTION.
6. FASTEN GYPBOARD AND PLYWOOD WITH 1/4 INCH TEK SCREWS AT 6 INCH AT ALL SUPPORTS AND EDGES.
7. BEARING STUDS MUST BE FABRICATED WITH FULL STUD END SEATED AGAINST TRACK WEB. DO NOT USE STUD THAT HAS BEEN CUT.
8. PROVIDE DOUBLE METAL STUD AROUND ALL OPENINGS IN STUD WALL SYSTEM AT JAMBS, HEADS, AND SILLS. WELD AT ALL INTERSECTIONS.

K. INSPECTIONS:

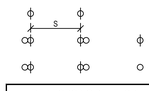
1. IN ACCORDANCE WITH 2006 IBC SECTION 1704, AS NOTED BELOW, TESTING AND INSPECTION SHALL BE BY AN INDEPENDENT TESTING/INSPECTION FIRM UNDER THE SUPERVISION OF A LICENSED ENGINEER EMPLOYED BY THAT FIRM. THIS ENGINEER SHALL BE DEEMED THE DESIGNATED ENGINEER OF RECORD FOR SPECIAL INSPECTIONS PERFORMED BY HIS FIRM OR HIS CONSULTANTS. INSPECTORS SHALL BE ICBO CERTIFIED AND APPROVED BY THE BUILDING OFFICIAL.
2. THE DESIGNATED ENGINEER OF RECORD FOR SPECIAL INSPECTIONS SHALL BE RESPONSIBLE FOR DEFINING THE ACTIVITIES OF THE INSPECTORS, FOR CERTIFYING THE QUALIFICATIONS OF THE INSPECTORS WITH THE BUILDING OFFICIAL AND TO ATTEND THE PRE-CONSTRUCTION MEETING TO DEFINE THEIR SCOPE OF SERVICES AND THE TESTING OR TEST PROCEDURES THAT ARE REQUIRED AS OUTLINED IN THE INTERNATIONAL BUILDING CODE.
3. SPECIAL INSPECTION IS TO BE PROVIDED IN ADDITION TO THE INSPECTIONS CONDUCTED BY THE LOCAL DEPARTMENT OF BUILDING SAFETY AND SHALL NOT BE CONSTRUED TO RELIEVE THE OWNER OR HIS AUTHORIZED AGENT FROM REQUESTING THE PERIODIC AND CALLED INSPECTIONS REQUIRED BY SECTION 104.4 OF THE INTERNATIONAL BUILDING CODE.
4. STEEL PER SECTION 1704.3.
5. CONCRETE PER TABLE 1704.4 AND SECTION 1704.4.
6. ANCHOR BOLTS INSTALLED IN CONCRETE PER TABLE 1704.4.
7. REINFORCING PER TABLE 1704.4 AND EXCEPTION FOR CONCRETE REQUIRING SPECIAL INSPECTION.
8. MASONRY PER TABLE 1704.5.1 AND SECTION 1704.5.
9. GRADING, EXCAVATION AND FILLING PER SECTION 1704.7. COORDINATE WITH GEOTECHNICAL REPORT AND SITE GRADING PLAN.
10. EXPANSION BOLT, SCREW ANCHOR AND ADHESIVE ANCHOR INSTALLATION TO VERIFY INSTALLATION IN ACCORDANCE WITH ICBO REPORTS NOTED PREVIOUSLY OR APPROVED EQUAL.

L. OTHER:

1. UNLESS NOTED OTHERWISE, EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT 3 EXPANSION ANCHORS OR APPROVED EQUAL. EPOXY ADHESIVE SHALL BE HILTI HIT-HY 150 MAX OR APPROVED EQUAL.
2. VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO STARTING WORK. NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR INCONSISTENCIES.
3. VERIFY IN FIELD ALL EXISTING CONDITIONS SHOWN ON DRAWINGS.
4. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR MECHANICAL, ELECTRICAL, AND PLUMBING WITH APPROPRIATE TRADES. PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING, OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION.
5. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE STAMP (AND SIGNATURE) OF AN ENGINEER REGISTERED IN NEBRASKA.

TYPICAL REINFORCING NOTES:

1. REINFORCING BAR DEVELOPMENT AND LAP SPLICE LENGTH SHALL BE AS SHOWN IN THIS TABLE UNLESS OTHERWISE NOTED ON THE DRAWINGS.
2. THE LENGTHS SHOWN IN THE TABLE ARE BASED ON THE FOLLOWING CONCRETE COVERAGE AND REINFORCING C-C SPACING:
   - BEAMS OR COLUMNS:
     - COVER (EQUAL OR MORE) 1.0bd (BAR DIAMETER)
     - CENTER TO CENTER (C-C) SPACING (EQUAL OR MORE) 2.0bd
   - ALL OTHERS:
     - COVER (EQUAL OR MORE) 1.0bd
     - CENTER TO CENTER SPACING (EQUAL OR MORE) 3.0bd.
3. TOP BARS ARE DEFINED AS HORIZONTAL REINFORCEMENT SUCH THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.
4. DEVELOPMENT AND SPLICE LENGTH SHOWN SHALL NOT APPLY IF ANY OF THE FOLLOWING CONDITIONS OCCUR:
   - A) f'c < 4000 PSI
   - B) fy > 60,000 PSI
   - C) THE COVER OR C-C BAR SPACING IS NOT AS LISTED ABOVE
   - D) THE REINFORCING STEEL IS EPOXY COATED
   - E) LIGHT WEIGHT CONCRETE IS USED.
5. CENTER ON CENTER SPACING (S) IS DEFINED AS BELOW:

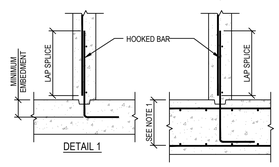


REINFORCING DEVELOPMENT AND SPLICES
f'c = 4,000 PSI

BAR SIZE	DEVELOPMENT LENGTH		SPLICE LENGTH	
	OTHER	TOP	OTHER	TOP
#3	1'-3"	1'-7"	1'-3"	2'-0"
#4	1'-7"	2'-1"	2'-1"	2'-8"
#5	2'-0"	2'-7"	2'-7"	3'-4"
#6	2'-5"	3'-1"	3'-1"	4'-0"
#7	3'-6"	4'-6"	4'-6"	5'-10"
#8	4'-0"	5'-2"	5'-2"	6'-8"
#9	4'-6"	5'-10"	5'-10"	7'-7"
#10	5'-1"	6'-7"	6'-7"	8'-6"
#11	5'-7"	7'-3"	7'-3"	9'-5"

DEVELOPMENT LENGTH NOTES:

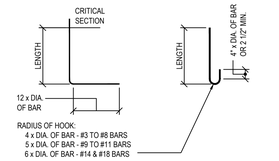
1. WHERE DRAWINGS ARE DETAILED SIMILAR TO DETAIL 2, EXTEND THE EMBEDMENT LENGTH SUCH THAT THE HOOKED BAR CONTACTS THE LAYER OF MAIN REINFORCING SHOWN.
2. EMBEDMENT LENGTHS IN CHART ARE TYPICAL EXCEPT AS NOTED IN DETAIL 2, OR AS INDICATED ON DRAWINGS.



DETAIL 1 DETAIL 2

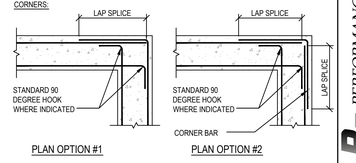
DEVELOPMENT LENGTHS FOR HOOKED BARS (f'c = 4,000 PSI)

BAR SIZE	LENGTH OR MIN. EMBEDMENT
#3	7"
#4	10"
#5	1'-0"
#6	1'-3"
#7	1'-5"
#8	1'-7"
#9	1'-10"
#10	2'-0"
#11	2'-3"

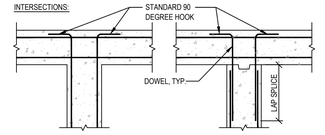


HORIZONTAL WALL AND FOOTING REINFORCING DETAILS:

CORNERS:



INTERSECTIONS:



NOTES:
1. UNLESS OTHERWISE INDICATED, THE CONTRACTOR HAS THE OPTION OF REINFORCING CORNERS IN ACCORDANCE WITH OPTION #1 OR OPTION #2.
2. UNLESS OTHERWISE INDICATED, THE CONTRACTOR HAS THE OPTION OF CONSTRUCTING INTERSECTIONS WITH OR WITHOUT CONSTRUCTION JOINTS. REINFORCE PER APPLICABLE DETAIL.
3. VERTICAL REINFORCING NOT SHOWN FOR CLARITY.
4. FOR LOCATIONS WITH ONLY ONE RUN OF HORIZONTAL REINFORCING, PROVIDE LAP SPLICES AS SHOWN FOR OUTSIDE LAYER OF REINFORCING SHOWN ABOVE.
5. FOR LOCATIONS WITH COLUMN SPREAD FOOTINGS, NO HOOKED BARS OR DOWELS ARE REQUIRED AS LONG AS HORIZONTAL WALL AND FOOTING REINFORCING HAS ADEQUATE DEVELOPMENT OR SPLICE LENGTH INTO SPREAD FOOTING.

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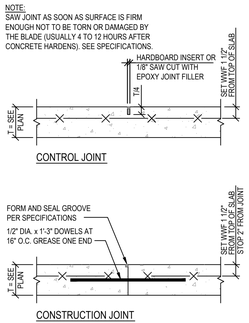
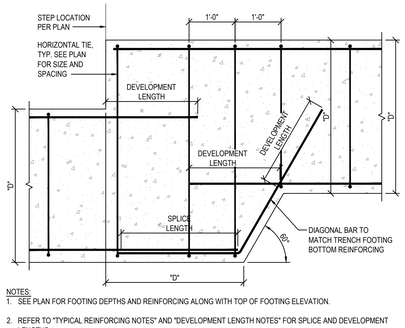
ANCHOR BOLT SCHEDULE		
DIA.	A	B
1 1/4"	3"	1'-6"
1"	3"	1'-2"
3/4"	2"	9"
5/8"	2"	8"
1/2"	2"	7"

COLUMN SIZE	BASE PLATE SIZE (U.N.O.)	ANCHOR BOLTS	LAYOUT (U.N.O.)
HSS5x5	3/4"x11x0'-11"	FOUR 3/4" DIA.	1 1/2" TYP. 1 1/2" TYP.

1 S03.00 BASE PLATE AND ANCHOR BOLT SCHEDULE
SCALE: 1 1/2" = 1'-0"

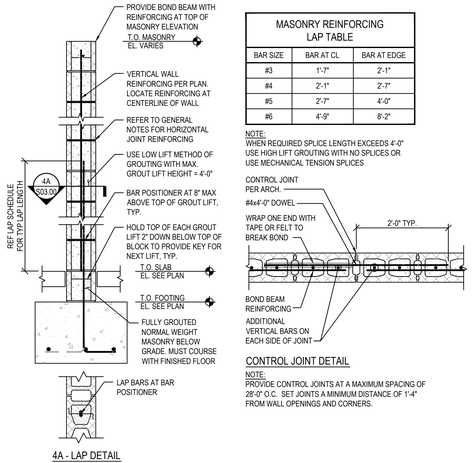


2 S03.00 FLOOR SLAB CONTROL AND CONSTRUCTION JOINT DETAILS
SCALE: 1 1/2" = 1'-0"



NOTES:
1. SEE PLAN FOR FOOTING DEPTHS AND REINFORCING ALONG WITH TOP OF FOOTING ELEVATION.
2. REFER TO "TYPICAL REINFORCING NOTES" AND "DEVELOPMENT LENGTH NOTES" FOR SPLICE AND DEVELOPMENT LENGTHS.
3. WALL ATOP FOOTING NOT SHOWN FOR CLARITY.

3 S03.00 FOOTING STEP DETAIL
SCALE: 3/4" = 1'-0"



4A - LAP DETAIL

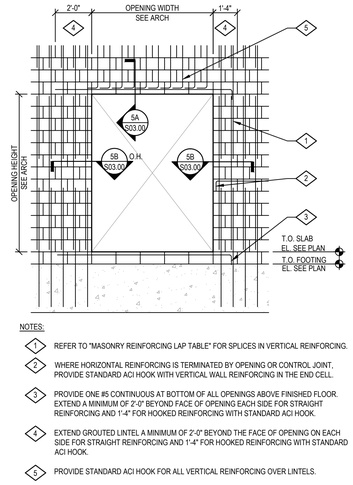
MASONRY REINFORCING LAP TABLE		
BAR SIZE	BAR AT CL	BAR AT EDGE
#3	1'-7"	2'-1"
#4	2'-1"	2'-7"
#5	2'-7"	4'-0"
#6	4'-9"	8'-2"

NOTE:
WHEN REQUIRED SPLICE LENGTH EXCEEDS 4'-0" USE HIGH LIFT GROUTING WITH NO SPLICES OR USE MECHANICAL TENSION SPLICES

CONTROL JOINT DETAIL

NOTE:
PROVIDE CONTROL JOINTS AT A MAXIMUM SPACING OF 28'-0" O.C. SET JOINTS A MINIMUM DISTANCE OF 1'-4" FROM WALL OPENINGS AND CORNERS.

4 S03.00 MASONRY WALL REINFORCING DIAGRAM
SCALE: NO SCALE


2'-0"
OPENING WIDTH SEE ARCH
1'-4"
OPENING HEIGHT SEE ARC

SHEET NOTES:
1. FINISH MATERIALS NOT NOTED AND SOME NOT SHOWN FOR CLARITY. REFER TO ARCHITECTURAL SHEETS FOR INFORMATION ON FINISH MATERIALS.
2. VERIFY ALL DIMENSIONS SHOWN WITH ARCHITECTURAL SHEETS.

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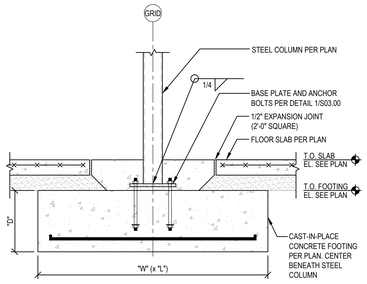
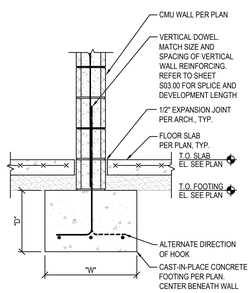
PE #: 14A437

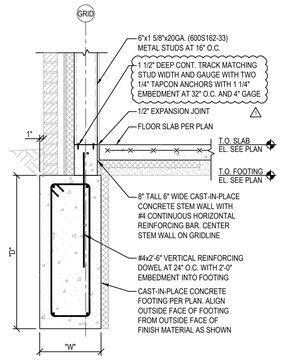


NOTE:
INTERNAL DEMISING METAL STUD WALL NOT SHOWN FOR CLARITY.

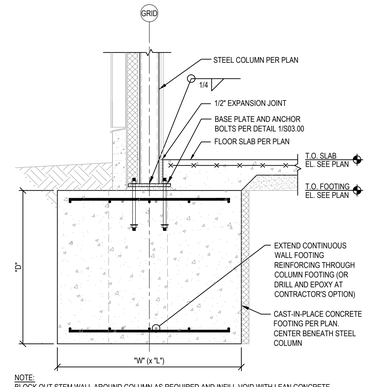
1 S03.01 **INTERIOR COLUMN FOOTING SECTION**
SCALE: 3/4" = 1'-0"



2 S03.01 **INTERIOR WALL FOOTING SECTION**
SCALE: 3/4" = 1'-0"

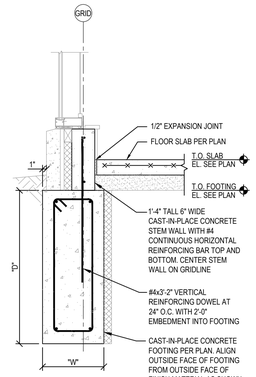


3 S03.01 **PERIMETER WALL FOOTING SECTION**
SCALE: 3/4" = 1'-0"

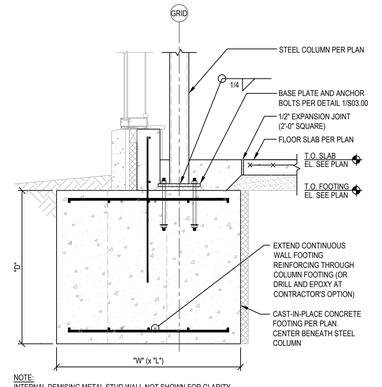


NOTE:
BLOCK OUT STEM WALL AROUND COLUMN AS REQUIRED AND INFILL VOID WITH LEAN CONCRETE.

4 S03.01 **PERIMETER COLUMN FOOTING SECTION**
SCALE: 3/4" = 1'-0"



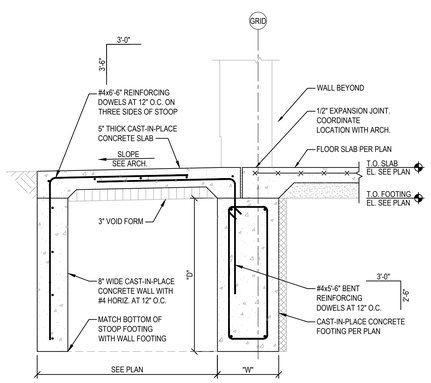
5 S03.01 **PERIMETER WALL FOOTING SECTION AT WINDOW**
SCALE: 3/4" = 1'-0"



NOTE:
INTERNAL DEMISING METAL STUD WALL NOT SHOWN FOR CLARITY.

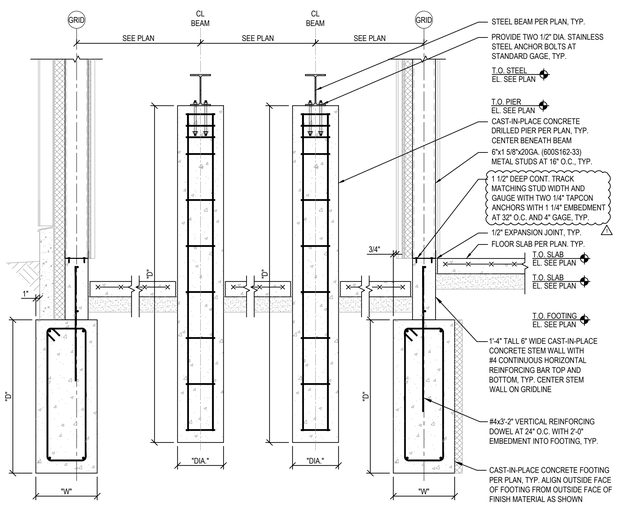
6 S03.01 **PERIMETER COLUMN FOOTING SECTION AT WINDOW**
SCALE: 3/4" = 1'-0"

NOTES:
1. PROVIDE CORNER HORIZONTAL REINFORCING PER SHEET S03.00 AT EDGES OF STOOPS.
2. COORDINATE EXACT PLACEMENT OF STOOP WITH ARCHITECTURAL AND CIVIL SHEETS.

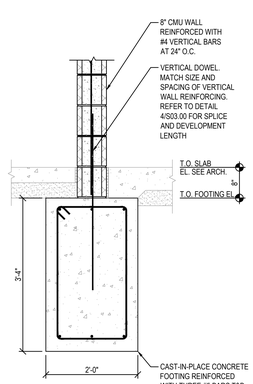


7 S03.01 **STOOP SECTION**
SCALE: 3/4" = 1'-0"

NOTES:
1. COORDINATE EXACT PLACEMENT OF COOLING TOWER SUPPORTS WITH EQUIPMENT MANUFACTURER.
2. PROVIDE 7/8" DIA. HOLES AT STANDARD GAGE OF 1 3/8" AT OUTER EDGE OF TOP FLANGES OF BEAMS (FOR A TOTAL GAGE OF 3'-3 3/8") AT A DISTANCE OF 3 1/4" FROM BEAM ENDS (S-5 1/2" SPACING). PROVIDE FOUR 3/4" DIA. STAINLESS STEEL BOLTS FOR CONNECTION OF COOLING TOWER CURB.



8 S03.01 **COOLING TOWER ENCLOSURE FOOTING SECTION**
SCALE: 3/4" = 1'-0"



9 S03.01 **TRASH ENCLOSURE WALL FOOTING SECTION**
SCALE: 3/4" = 1'-0"

Structural Package

RDG:: PLANNING + DESIGN

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SKINNER EARLY CHILDHOOD CENTER

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KEY PLAN

REV	DATE	DESCRIPTION
1	9/5/2014	ADDENDUM #1

ISSUED: AUG 28, 2014
PROJECT NO: 2014.128.00

FOUNDATION SECTIONS

S03.01